

March 1, 2021

Mr. Paul Gaudette Chairman  
Sandisfield Conservation Commission  
66 Sandisfield Road  
P.O. Box 663  
Sandisfield, MA 01255

RE: Notice of Intent Submission  
SAMA Productions, LLC  
Abby Road and Town Hill Road  
Sandisfield, MA

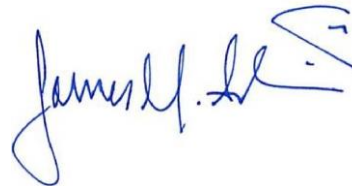
Dear Mr. Gaudette;

Enclosed please find eight (8) copies of a Notice of Intent prepared for SAMA Productions, LLC and property located at Lot 8 Abby Road (Portion of Map 408-Block 0-Lot 31) in Sandisfield, MA. As required, a copy has been forwarded to the Western Regional Office of the Massachusetts Department of Environmental Protection in Springfield, MA. The project does not fall within NHESP jurisdiction.

The proposed project includes the construction of a 100,000± square-foot of commercial greenhouses and a 5,000 sf one story metal building with related construction work, such as tree clearing and utilities. A portion of the work falls within buffer zone of a bordering vegetated wetlands. A detailed project narrative and plans have been provided in support of this NOI submission.

If you should have any questions or concerns, or require additional information, please do not hesitate to contact the office.

Sincerely,  
SK DESIGN GROUP, INC.



Jim Scalise PE

Enclosures

Cc: MADEP – Western Region  
SAMA Productions, LLC  
File

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Notice of Intent  
SAMA Productions LLC  
Abby Road,  
Sandisfield, Massachusetts

### **WPA Form 3 – Notice of Intent Form**

### **WPA Form 3 – Wetland Fee Transmittal Form**

- Check Affidavit
  
- ❖ Narrative
- ❖ List of Abutters
- ❖ Abutter Notification
- ❖ Copy of Certificates of Mailing

### **Figures**

1. USGS Locus Map
2. NHESP GIS Map of Estimated Habitats of Rare Wildlife and Vernal Pools
3. FEMA Floodplain Map
4. Assessor's Map

### **Attachments**

- A. Deed of Property
- B. Survey Map
- C. Wetland Report
- D. Stormwater Report
- E. Plans to Accompany Notice of Intent Prepared for SAMA Productions LLC



Massachusetts Department of Environmental Protection

# eDEP Transaction Copy

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Here is the file you requested for your records.

To retain a copy of this file you must save and/or print.

Username: **SKDESIGN**

Transaction ID: **1259789**

Document: **WPA Form 3 - NOI**

Size of File: **250.28K**

Status of Transaction: **In Process**

Date and Time Created: **3/3/2021:1:09:36 PM**

**Note:** This file only includes forms that were part of your transaction as of the date and time indicated above. If you need a more current copy of your transaction, return to eDEP and select to "Download a Copy" from the Current Submittals page.

**Massachusetts Department of Environmental Protection**  
Bureau of Resource Protection - Wetlands  
**WPA Form 3 - Notice of Intent**  
Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP:  
MassDEP File #:  
eDEP Transaction #:1259789  
City/Town:SANDISFIELD

**A.General Information**

1. Project Location:

a. Street Address	LOT 8 ABBY ROAD	c. Zip Code	01255
b. City/Town	SANDISFIELD	e. Longitude	73.14030W
d. Latitude	42.14593N	g.Parcel/Lot #	31
f. Map/Plat #	408		

2. Applicant:

Individual  Organization

a. First Name	JOHN	b.Last Name	HECK
c. Organization	SAMA PRODUCTIONS, LLC		
d. Mailing Address	28 PRISCILLA AVENUE		
e. City/Town	YONKERS	f. State	NY
g. Zip Code	10710		
h. Phone Number	914-263-6627	i. Fax	
j. Email	generalpushing@gmail.com		

3.Property Owner:

more than one owner

a. First Name	JAMES	b. Last Name	MIECZKOWSKI
c. Organization	EXECUTOR OF ESTATE OF OF PAUL BOBRYK		
d. Mailing Address	535 FISH ROCK ROAD		
e. City/Town	SOUTHBURY	f.State	CT
g. Zip Code	06488-2131		
h. Phone Number	413-443-3537	i. Fax	
j.Email	jscalise@sk-designgroup.com		

4.Representative:

a. First Name	JAMES	b. Last Name	SCALISE
c. Organization	SK DESIGN GROUP, INC		
d. Mailing Address	2 FEDERICO DRIVE, SUITE 2		
e. City/Town	PITTSFIELD	f. State	MA
g. Zip Code	01201		
h.Phone Number	413-443-3537	i.Fax	413-445-5376
j.Email	jscalise@sk-designgroup.com		

5.Total WPA Fee Paid (Automatically inserted from NOI Wetland Fee Transmittal Form):

a.Total Fee Paid	1,050.00	b.State Fee Paid	512.50	c.City/Town Fee Paid	537.50
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6.General Project Description:

CONSTRUCTION OF COMMERCIAL GREENHOUSES. GRADING AND STORMWATER BMPS. THE BMPS ARE IN THE BUFFER ZONE.

7a.Project Type:

- |   |  |
|---|--|
| 1. <input type="checkbox"/> Single Family Home                | 2. <input type="checkbox"/> Residential Subdivision                  |
| 3. <input type="checkbox"/> Limited Project Driveway Crossing | 4. <input checked="" type="checkbox"/> Commercial/Industrial         |
| 5. <input type="checkbox"/> Dock/Pier                         | 6. <input type="checkbox"/> Utilities                                |
| 7. <input type="checkbox"/> Coastal Engineering Structure     | 8. <input type="checkbox"/> Agriculture (eg., cranberries, forestry) |
| 9. <input type="checkbox"/> Transportation                    | 10. <input type="checkbox"/> Other                                   |

7b.Is any portion of the proposed activity eligible to be treated as a limited project subject to 310 CMR 10.24 (coastal) or 310

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CMR 10.53 (inland)?

1.  Yes  No If yes, describe which limited project applies to this project:  
 2. Limited Project

8. Property recorded at the Registry of Deeds for:

<b>a.County:</b>	<b>b.Certificate:</b>	<b>c.Book:</b>	<b>d.Page:</b>
SOUTHERN BERKSHIRE		390	398

**B. Buffer Zone & Resource Area Impacts (temporary & permanent)**

1. Buffer Zone & Resource Area Impacts (temporary & permanent):

This is a Buffer Zone only project - Check if the project is located only in the Buffer Zone of a Bordering Vegetated Wetland, Inland Bank, or Coastal Resource Area.

2. Inland Resource Areas: (See 310 CMR 10.54 - 10.58, if not applicable, go to Section B.3. Coastal Resource Areas)

Resource Area	Size of Proposed Alteration	Proposed Replacement (if any)
---------------	-----------------------------	-------------------------------

a. <input type="checkbox"/> Bank	1. linear feet	2. linear feet
b. <input type="checkbox"/> Bordering Vegetated Wetland	1. square feet	2. square feet
c. <input type="checkbox"/> Land under Waterbodies and Waterways	1. Square feet	2. square feet
	3. cubic yards dredged	
d. <input type="checkbox"/> Bordering Land Subject to Flooding	1. square feet	2. square feet
	3. cubic feet of flood storage lost	4. cubic feet replaced
e. <input type="checkbox"/> Isolated Land Subject to Flooding	1. square feet	
	2. cubic feet of flood storage lost	3. cubic feet replaced

f.  Riverfront Area

1. Name of Waterway (if any)
2. Width of Riverfront Area (check one)
- 25 ft. - Designated Densely Developed Areas only
- 100 ft. - New agricultural projects only
- 200 ft. - All other projects
3. Total area of Riverfront Area on the site of the proposed project
- square feet

4. Proposed Alteration of the Riverfront Area:

- a. total square feet      b. square feet within 100 ft.      c. square feet between 100 ft. and 200 ft.

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5. Has an alternatives analysis been done and is it attached to this NOI?  Yes  No  
 6. Was the lot where the activity is proposed created prior to August 1, 1996?  Yes  No

3.Coastal Resource Areas: (See 310 CMR 10.25 - 10.35)

Resource Area Size of Proposed Alteration Proposed Replacement (if any)

a. <input type="checkbox"/> Designated Port Areas	Indicate size under	Land under the ocean below,
b. <input type="checkbox"/> Land Under the Ocean	1. square feet	
	2. cubic yards dredged	
c. <input type="checkbox"/> Barrier Beaches	Indicate size under Coastal Beaches and/or Coastal Dunes, below	
d. <input type="checkbox"/> Coastal Beaches	1. square feet	2. cubic yards beach nourishment
e. <input type="checkbox"/> Coastal Dunes	1. square feet	2. cubic yards dune nourishment
f. <input type="checkbox"/> Coastal Banks	1. linear feet	
g. <input type="checkbox"/> Rocky Intertidal Shores	1. square feet	
h. <input type="checkbox"/> Salt Marshes	1. square feet	2. sq ft restoration, rehab, crea.
i. <input type="checkbox"/> Land Under Salt Ponds	1. square feet	
	2. cubic yards dredged	
j. <input type="checkbox"/> Land Containing Shellfish	1. square feet	
k. <input type="checkbox"/> Fish Runs	Indicate size under Coastal Banks, Inland Bank, Land Under the Ocean, and/or inland Land Under Waterbodies and Waterways, above	
	1. cubic yards dredged	
l. <input type="checkbox"/> Land Subject to Coastal Storm Flowage	1. square feet	

4.Restoration/Enhancement

Restoration/Replacement

If the project is for the purpose of restoring or enhancing a wetland resource area in addition to the square footage that has been entered in Section B.2.b or B.3.h above, please entered the additional amount here.

a. square feet of BVW b. square feet of Salt Marsh

5.Projects Involves Stream Crossings

Project Involves Streams Crossings

□ **Massachusetts Department of Environmental Protection**  
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Provided by MassDEP:  
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City/Town:SANDISFIELD

If the project involves Stream Crossings, please enter the number of new stream crossings/number of replacement stream crossings.

a. number of new stream crossings

b. number of replacement stream crossings

**C. Other Applicable Standards and Requirements**

**Streamlined Massachusetts Endangered Species Act/Wetlands Protection Act Review**

1. Is any portion of the proposed project located in **Estimated Habitat of Rare Wildlife** as indicated on the most recent Estimated Habitat Map of State-Listed Rare Wetland Wildlife published by the Natural Heritage of Endangered Species program (NHESP)?

a.  Yes  No

If yes, include proof of mailing or hand delivery of NOI to:  
Natural Heritage and Endangered Species  
Program  
Division of Fisheries and Wildlife  
1 Rabbit Hill Road  
Westborough, MA 01581

b. Date of map:FROM MAP VIEWER

If yes, the project is also subject to Massachusetts Endangered Species Act (MESA) review (321 CMR 10.18)...

c. Submit Supplemental Information for Endangered Species Review \* (Check boxes as they apply)

1.  Percentage/acreage of property to be altered:

(a) within Wetland Resource Area

percentage/acreage

(b) outside Resource Area

percentage/acreage

2.  Assessor's Map or right-of-way plan of site

3.  Project plans for entire project site, including wetland resource areas and areas outside of wetland jurisdiction, showing existing and proposed conditions, existing and proposed tree/vegetation clearing line, and clearly demarcated limits of work \*\*

a.  Project description (including description of impacts outside of wetland resource area & buffer zone)

b.  Photographs representative of the site

c.  MESA filing fee (fee information available at: <http://www.mass.gov/eea/agencies/dfg/dfw/natural-heritage/regulatory-review/mass-endangered-species-act-mesa/mesa-fee-schedule.html>)

Make check payable to "Natural Heritage & Endangered Species Fund" and **mail to NHESP** at above address

*Projects altering 10 or more acres of land, also submit:*

d.  Vegetation cover type map of site

e.  Project plans showing Priority & Estimated Habitat boundaries

d. OR Check One of the following

1.  Project is exempt from MESA review. Attach applicant letter indicating which MESA exemption applies. (See 321 CMR 10.14, <http://www.mass.gov/eea/agencies/dfg/dfw/laws-regulations/cmr/321-cmr-1000-massachusetts-endangered-species-act.html#10.14>; the NOI must still be sent to NHESP if the project is within estimated habitat pursuant to 310 CMR 10.37 and 10.59.)

2.  Separate MESA review ongoing.

a. NHESP Tracking Number

b. Date submitted to NHESP

□ **Massachusetts Department of Environmental Protection**

Bureau of Resource Protection - Wetlands

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Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP:  
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City/Town:SANDISFIELD

3.  Separate MESA review completed.

Include copy of NHESP "no Take" determination or valid Conservation & Management Permit with approved plan.

\* Some projects **not** in Estimated Habitat may be located in Priority Habitat, and require NHESP review...

2. For coastal projects only, is any portion of the proposed project located below the mean high waterline or in a fish run?

a.  Not applicable - project is in inland resource area only

b.  Yes  No

If yes, include proof of mailing or hand delivery of NOI to either:

South Shore - Cohasset to Rhode Island, and the Cape & Islands:

North Shore - Hull to New Hampshire:

Division of Marine Fisheries -  
Southeast Marine Fisheries Station  
Attn: Environmental Reviewer  
836 S. Rodney French Blvd  
New Bedford, MA 02744

Division of Marine Fisheries -  
North Shore Office  
Attn: Environmental Reviewer  
30 Emerson Avenue  
Gloucester, MA 01930

If yes, it may require a Chapter 91 license. For coastal towns in the Northeast Region, please contact MassDEP's Boston Office.

For coastal towns in the Southeast Region, please contact MassDEP's Southeast Regional office.

3. Is any portion of the proposed project within an Area of Critical Environmental Concern (ACEC)?

a.  Yes  No

If yes, provide name of ACEC (see instructions to WPA Form 3 or DEP Website for ACEC locations). **Note:** electronic filers click on Website.

b. ACEC Name

4. Is any portion of the proposed project within an area designated as an Outstanding Resource Water (ORW) as designated in the Massachusetts Surface Water Quality Standards, 314 CMR 4.00?

a.  Yes  No

5. Is any portion of the site subject to a Wetlands Restriction Order under the Inland Wetlands Restriction Act (M.G.L.c. 131, § 40A) or the Coastal Wetlands Restriction Act (M.G.L.c. 130, § 105)?

a.  Yes  No

6. Is this project subject to provisions of the MassDEP Stormwater Management Standards?

a.  Yes, Attach a copy of the Stormwater Report as required by the Stormwater Management Standards per 310 CMR 10.05(6)(k)-(q) and check if:

1.  Applying for Low Impact Development (LID) site design credits (as described in Stormwater Management Handbook Vol.2, Chapter 3)

2.  A portion of the site constitutes redevelopment

3.  Proprietary BMPs are included in the Stormwater Management System

b.  No, Explain why the project is exempt:

1.  Single Family Home



Massachusetts Department of Environmental Protection

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- 2. Emergency Road Repair
3. Small Residential Subdivision (less than or equal to 4 single-family houses or less than or equal to 4 units in multi-family housing project) with no discharge to Critical Areas.

D. Additional Information

Applicants must include the following with this Notice of Intent (NOI). See instructions for details.

Online Users: Attach the document transaction number (provided on your receipt page) for any of the following information you submit to the Department by regular mail delivery.

- 1. USGS or other map of the area (along with a narrative description, if necessary) containing sufficient information for the Conservation Commission and the Department to locate the site.
2. Plans identifying the location of proposed activities (including activities proposed to serve as a Bordering Vegetated Wetland [BVW] replication area or other mitigating measure) relative to the boundaries of each affected resource area.
3. Identify the method for BVW and other resource area boundary delineations (MassDEP BVW Field Data Form(s).
4. List the titles and dates for all plans and other materials submitted with this NOI.

a. Plan Title: b. Plan Prepared By: c. Plan Signed/Stamped By: c. Revised Final Date: e. Scale:

PLANS TO ACCOMPANY NOTICE OF INTENT PREPARED FOR SAMA PRODUCTIONS, LLC TOWN HILL ROAD, SANDISFIELD, MA

SK DESIGN GROUP, INC

JAMES SCALISE

March 2, 2021

VARIES

- 5. If there is more than one property owner, please attach a list of these property owners not listed on this form.
6. Attach proof of mailing for Natural Heritage and Endangered Species Program, if needed.
7. Attach proof of mailing for Massachusetts Division of Marine Fisheries, if needed.
8. Attach NOI Wetland Fee Transmittal Form.
9. Attach Stormwater Report, if needed.

□ **Massachusetts Department of Environmental Protection**  
Bureau of Resource Protection - Wetlands  
**WPA Form 3 - Notice of Intent**  
Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP:  
MassDEP File #:  
eDEP Transaction #:1259789  
City/Town:SANDISFIELD

**E. Fees**

1. Fee Exempt: No filing fee shall be assessed for projects of any city, town, county, or district of the Commonwealth, federally recognized Indian tribe housing authority, municipal housing authority, or the Massachusetts Bay Transportation Authority.

Applicants must submit the following information (in addition to pages 1 and 2 of the NOI Wetland Fee Transmittal Form) to confirm fee payment:

120  
2. Municipal Check Number  
122  
4. State Check Number  
John  
6. Payer name on check: First Name

3/2/21  
3. Check date  
3/2/21  
5. Check date  
Heck  
7. Payer name on check: Last Name

**F. Signatures and Submittal Requirements**

I hereby certify under the penalties of perjury that the foregoing Notice of Intent and accompanying plans, documents, and supporting data are true and complete to the best of my knowledge. I understand that the Conservation Commission will place notification of this Notice in a local newspaper at the expense of the applicant in accordance with the wetlands regulations, 310 CMR 10.05(5)(a).

I further certify under penalties of perjury that all abutters were notified of this application, pursuant to the requirements of M.G.L. c. 131, § 40. Notice must be made by Certificate of Mailing or in writing by hand delivery or certified mail (return receipt requested) to all abutters within 100 feet of the property line of the project location.

\_\_\_\_\_  
1. Signature of Applicant

\_\_\_\_\_  
2. Date

\_\_\_\_\_  
3. Signature of Property Owner(if different)

\_\_\_\_\_  
4. Date

\_\_\_\_\_  
5. Signature of Representative (if any)

\_\_\_\_\_  
6. Date

**For Conservation Commission:**

Two copies of the completed Notice of Intent (Form 3), including supporting plans and documents, two copies of the NOI Wetland Fee Transmittal Form, and the city/town fee payment, to the Conservation Commission by certified mail or hand delivery.

**For MassDEP:**

One copy of the completed Notice of Intent (Form 3), including supporting plans and documents, one copy of the NOI Wetland Fee Transmittal Form, and a copy of the state fee payment to the MassDEP Regional Office (see Instructions) by certified mail or hand delivery.

**Other:**

If the applicant has checked the "yes" box in Section C, Items 1-3, above, refer to that section and the Instructions for additional submittal requirements.

The original and copies must be sent simultaneously. Failure by the applicant to send copies in a timely manner may result in dismissal of the Notice of Intent.

**Massachusetts Department of Environmental Protection**  
 Bureau of Resource Protection - Wetlands  
**WPA Form 3 - Notice of Wetland Fee Transmittal**  
**Form**  
 Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP:  
 MassDEP File #:  
 eDEP Transaction #:1259789  
 City/Town:SANDISFIELD

**A. Applicant Information**

1. Applicant:

a. First Name	JOHN	b. Last Name	HECK		
c. Organization	SAMA PRODUCTIONS, LLC				
d. Mailing Address	28 PRISCILLA AVENUE				
e. City/Town	YONKERS	f. State	NY	g. Zip Code	10710
h. Phone Number	9142636627	i. Fax		j. Email	generalpushing@gmail.com

2. Property Owner:(if different)

a. First Name	JAMES	b. Last Name	MIECZKOWSKI		
c. Organization	EXECUTOR OF ESTATE OF OF PAUL BOBRYK				
d. Mailing Address	535 FISH ROCK ROAD				
e. City/Town	SOUTHBURY	f. State	CT	g. Zip Code	06488-2131
h. Phone Number	4134433537	i. Fax		j. Email	jscalise@sk-designgroup.com

3. Project Location:

a. Street Address	LOT 8 ABBY ROAD	b. City/Town	SANDISFIELD
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Are you exempted from Fee?

**Note:** Fee will be exempted if you are one of the following:

- City/Town/County/District
- Municipal Housing Authority
- Indian Tribe Housing Authority
- MBTA

State agencies are only exempt if the fee is less than \$100

**B. Fees**

Activity Type	Activity Number	Activity Fee	RF Multiplier	Sub Total
B.)STORMWATER BMPS IN THE BUFFER ZONE	1	1050.00		1050.00
		City/Town share of filling fee	State share of filing fee	Total Project Fee
		\$537.50	\$512.50	\$1,050.00

**Massachusetts Department of Environmental Protection**

Bureau of Resource Protection - Wetlands

**WPA Form 3 - Notice of Intent**

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122  
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John  
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3. Check date  
3/2/21  
5. Check date  
Heck  
7. Payer name on check: Last Name

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I hereby certify under the penalties of perjury that the foregoing Notice of Intent and accompanying plans, documents, and supporting data are true and complete to the best of my knowledge. I understand that the Conservation Commission will place notification of this Notice in a local newspaper at the expense of the applicant in accordance with the wetlands regulations, 310 CMR 10.05(5)(a).

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1. Signature of Applicant  
3. Signature of Property Owner (if different)  
5. Signature of Representative (if any)

2. Date 3/3/21  
4. Date 3/3/21  
6. Date

**For Conservation Commission:**

Two copies of the completed Notice of Intent (Form 3), including supporting plans and documents, two copies of the NOI Wetland Fee Transmittal Form, and the city/town fee payment, to the Conservation Commission by certified mail or hand delivery.

**For MassDEP:**

One copy of the completed Notice of Intent (Form 3), including supporting plans and documents, one copy of the NOI Wetland Fee Transmittal Form, and a copy of the state fee payment to the MassDEP Regional Office (see Instructions) by certified mail or hand delivery.

**Other:**

If the applicant has checked the "yes" box in Section C, Items 1-3, above, refer to that section and the Instructions for additional submittal requirements.

The original and copies must be sent simultaneously. Failure by the applicant to send copies in a timely manner may result in dismissal of the Notice of Intent.

**Affidavit of Payment**

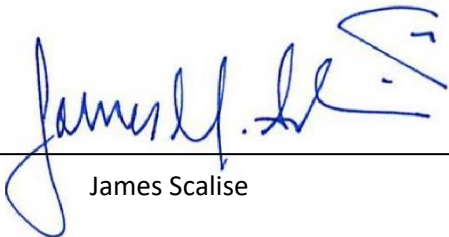
**Notice of Intent Application  
SAMA Productions LLC  
Lot 8 Abby Road  
Sandisfield, MA**

Submitted with this Notice of Intent Application were the required filing fees for the above referenced project. These filing fees are as follows:

Town of Sandisfield  
\$537.50 check per NOI Activity Fee

Commonwealth of Massachusetts  
\$512.50 check per NOI Activity Fee

The checks were issued by SAMA Productions LLC on or about March 2, 2021.

  
James Scalise

## Notice of Intent Narrative

## NOTICE OF INTENT NARRATIVE

*SAMA Productions LLC  
East Hubbard Road (aka Abby Road) and Town Hill Road,  
Sandisfield, Massachusetts  
(Portion of Map 408, Lot 31)*

### 1.0 EXISTING CONDITIONS

The property is a 46.7 Acre lot located on the south side of Abby Road, 1000 feet, more or less, west of the intersection of East Hubbard Road and Town Hill Road (see Figure 1). The lot is undeveloped forest and includes Town Hill a topographic feature. The property is bordered by state land to the west and remaining land of Lot 31 to the south and east. North of the lot is bordered by Abby Road and undeveloped residential property. The parcel is defined as Lot 8 on the attached survey prepared for “Franklin Woods Investments” A copy of the survey is included in Attachment B. A bordering vegetated wetland is located along the westerly property line along the length of the proposed development. The proposed work is located in the buffer zone and provides a 50 foot setback from the BVW boundary.

The property is not located within a habitat boundary established by the Natural Heritage and Endangered Species Program (NHESP)(see Figure 2).

### 2.0 PROJECT DESCRIPTION & SEQUENCE

The proposed project includes: the construction of a 105,000 S.F. of buildings including 23 greenhouse structures and a single 5,000 sf pre-engineered metal building. The site improvements include a fenced in yard surrounding the greenhouses, entrance drive, parking, septic system, well for drinking water and stormwater BMPs. The project may include an irrigation well in the development area for watering plants.

The design proposes grading and stormwater BMPs in the buffer zone. This was unavoidable due to the lot configuration and site topography. Shifting the development area south or east required significant removal of soil to depths over 15 feet as the work area infringes upon the hillside. As a result of the site constraints the project infringes in the outer buffer zone.

The general sequence for construction would consist of the following:

#### **Proposed Project Sequence**

1. Install MADEP file number sign.
2. File Order in the Registry of Deeds
3. Contact surveyor to locate clearing limits.
4. Install erosion controls along resource areas and limit of work as shown on project plans.
5. Contact Conservation Commission for site inspection and notify commencement of work.
6. Clear trees as shown on the project plan.
7. Start construction of building and related site work.
8. Maintain erosion controls regularly.
9. Construct temporary stormwater controls.
10. Complete earthwork and infrastructure including lighting, drainage and fencing.
11. Construct buildings.
12. Backfill and install topsoil, planting beds, seed, and straw mulch.

13. Restore temporary construction access and dewatering measures.
14. Loam, seed and mulch all remaining disturbed areas.
15. Contact Conservation Commission for a site visit.
16. Remove erosion controls.

**3.0 BUFFER ZONE (310 CMR 10.02)**

Buffer zone is associated with the onsite Bordering Vegetated Wetland. A portion of the proposed work falls within the Buffer Zone.

**4.0 BANK (310 CMR 10.54)**

There is a small channel in the BVW. This boundary was estimated, and field located by SK Design Group. None of the proposed work falls within the Bank resource.

**5.0 BORDERING VEGETATED WETLAND (310 CMR 10.55)**

There is no alteration of bordering vegetated wetlands associated with the project. The boundary was inspected and delineated in accordance with procedures under the Wetland Protection Act. Specifics are found in Attachment C.

**6.0 LAND UNDER WATER BODIES AND WATERWAYS (310 CMR 10.56)**

No Land under water bodies and waterways is associated with this property.

**7.0 LAND SUBJECT TO FLOODING (310 CMR 10.57)  
BORDERING**

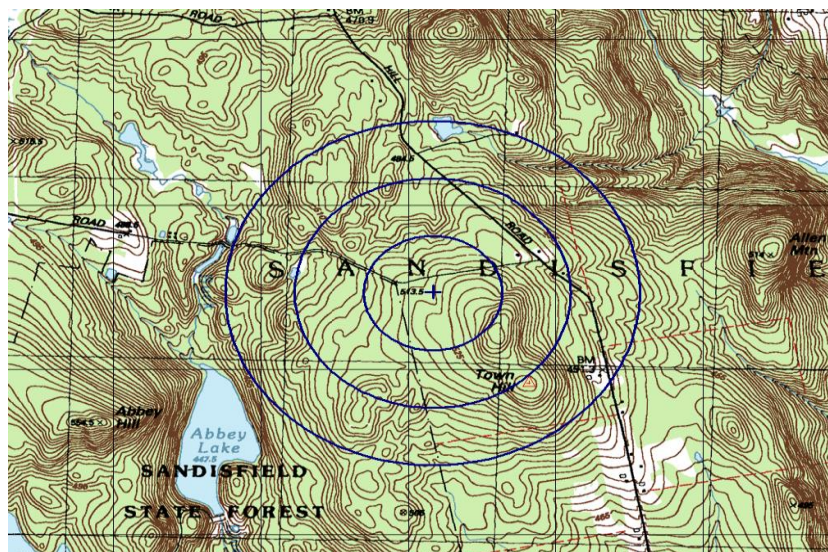
There is bordering no land subject to flooding (100-year floodplain) associated with this property. A FEMA map is included as Figure 3.

**ISOLATED**

There is no known isolated land subject to flooding associated with the project site.

**8.0 RIVERFRONT (310 CMR 10.58)**

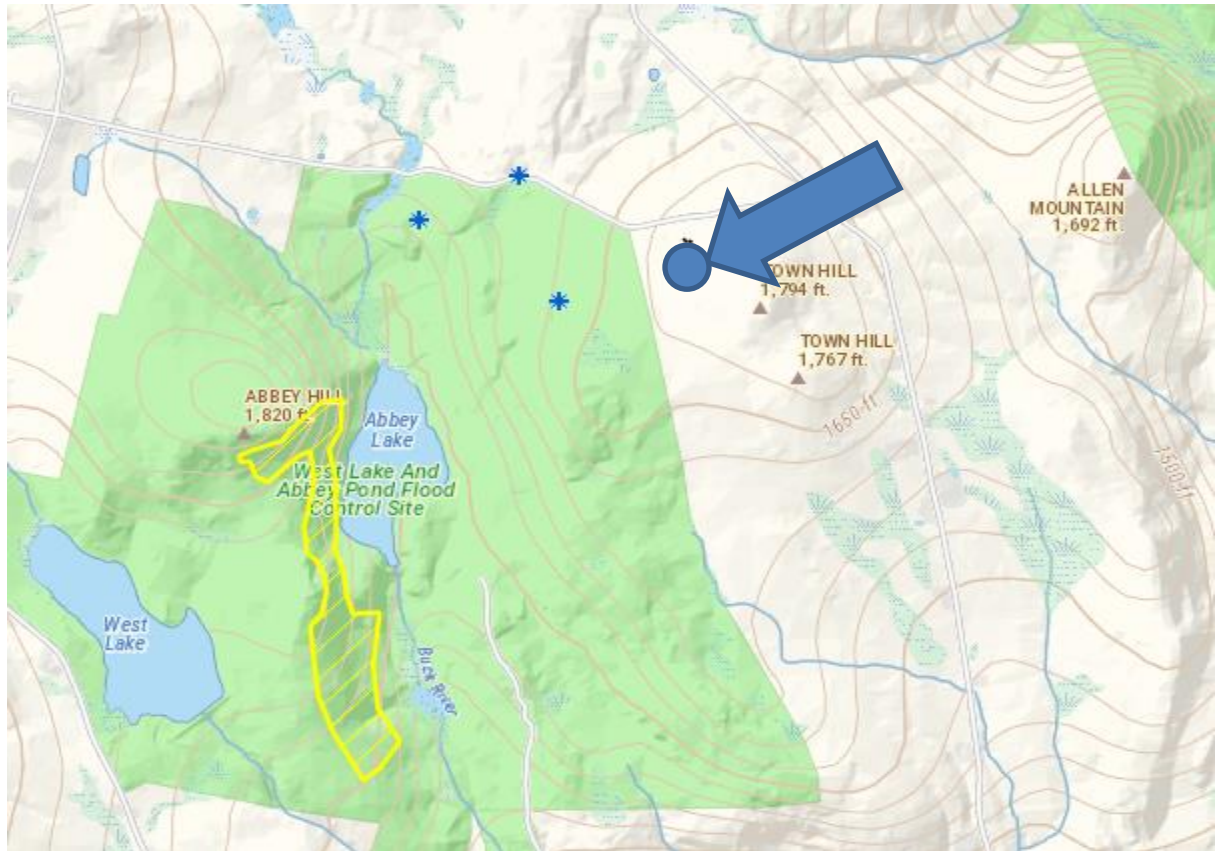
Rivers are solid blue lines depicted on the USGS map. The figure copied below depicts no such line. Thus, no such rivers exist on or near the property.





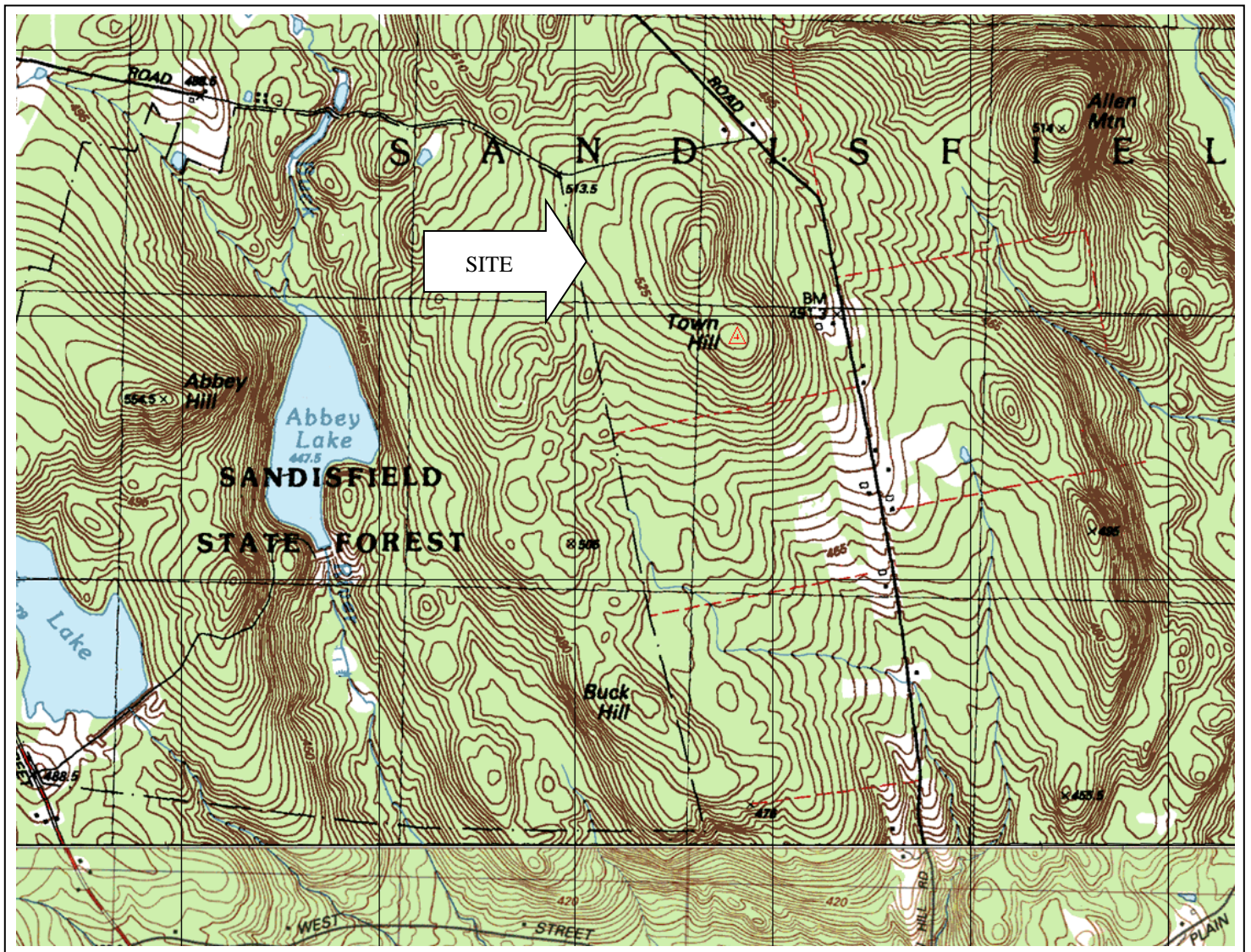
**9.0 ESTIMATED HABITATS OF RARE WILDLIFE (310 CMR 10.59)**

Per the Mass. NHESP website interactive map (14th Edition Natural Heritage Atlas, August 1, 2017) the property does not fall within a polygon established by the Natural Heritage and Endangered Species Program.



**10.0 STREAM CROSSING STANDARDS**

There are no stream crossings proposed as a part of this application.



Source: U.S.G.S. Map

**FIGURE #1**

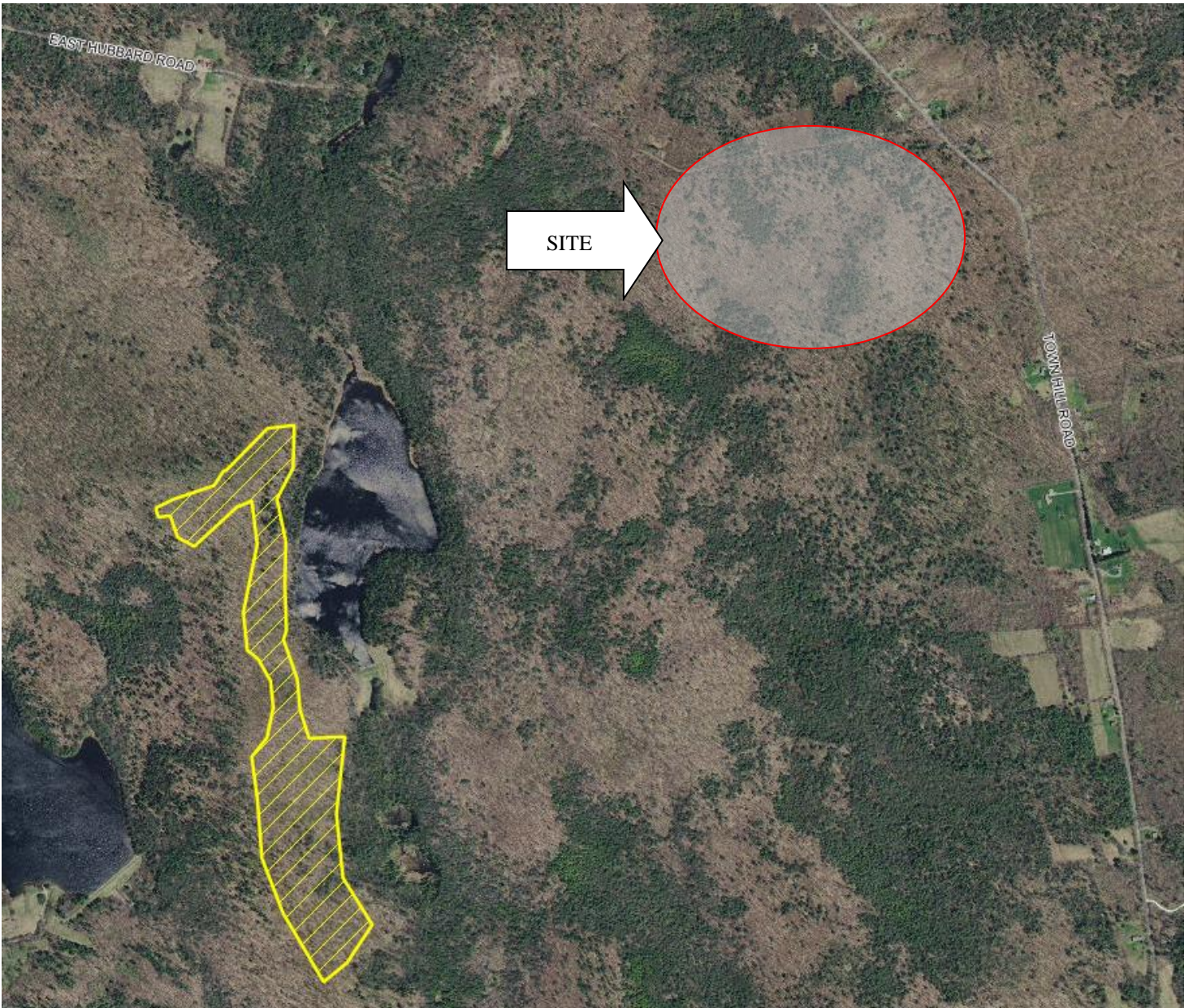
**U.S.G.S. Map**  
 Lot8 Abby Road  
 Sandisfield, MA



**DESIGN GROUP, INC.**

CIVIL ENGINEERS - SURVEYORS - CONSULTANTS

2 FEDERICO DR., PITTSFIELD, MA 01201 (413) 443-3537



**PRIORITY & ESTIMATED HABITAT**

**FIGURE #2**

**N.H.E.S.P. MAP**

Lot-8 Abby Road  
Sandisfield, MA



**DESIGN GROUP, INC.**

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Source: [www.msc.fema.gov](http://www.msc.fema.gov)

**FIGURE #3**

**FEMA Floodplain Map**

Lot-8 Abby Road  
Sandisfield, MA



**DESIGN GROUP, INC.**

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Source: Sandisfield GIS Map

**FIGURE #4**

**Town of Sandisfield Town Map**

Abby Road  
Sandisfield, MA  
Portion of Lot 31



**DESIGN GROUP, INC.**

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2 FEDERICO DR., PITTSFIELD, MA 01201 (413) 443-3537



# 100 foot Abutters List Report

Sandisfield, MA  
February 24, 2021

## Subject Property:

Parcel Number: 408-0-31  
CAMA Number: 408-0-31  
Property Address: TOWN HILL RD

Mailing Address: BOBRYK PAUL ESTATE OF  
MIECZKOWSKI JAMES EXECUTOR  
535 FISH ROCK RD  
SOUTHBURY, CT 06488-2131

---

## Abutters:

Parcel Number: 405-0-20  
CAMA Number: 405-0-20  
Property Address: 102 TOWN HILL RD

Mailing Address: ARICO THOMAS  
94 GREAT HILL POND ROAD  
PORTLAND, CT 06480

Parcel Number: 405-0-22  
CAMA Number: 405-0-22  
Property Address: ABBY ROAD

Mailing Address: REIDEMAN JON FALK MADELILNE  
PO BOX 172  
NORFOLK, CT 06058

Parcel Number: 405-0-26.1  
CAMA Number: 405-0-26.1  
Property Address: 103 TOWN HILL RD

Mailing Address: TRUE CYNTHIA M  
PO BOX 502  
SANDISFIELD, MA 01255-0502

Parcel Number: 408-0-1  
CAMA Number: 408-0-1  
Property Address: TOWN HILL RD

Mailing Address: MACK BRENNIA  
69 NORFOLK RD  
WINSTED, CT 06089

Parcel Number: 408-0-28  
CAMA Number: 408-0-28  
Property Address: 75 TOWN HILL RD

Mailing Address: HOLT HENRY G  
PO BOX 89  
VINAL HAVEN, ME 04863-0089

Parcel Number: 408-0-3  
CAMA Number: 408-0-3  
Property Address: TOWN HILL RD

Mailing Address: HULL FORESTLANDS, LP  
101 HAMPTON ROAD  
POMFRET CENTER, CT 06259

Parcel Number: 408-0-4  
CAMA Number: 408-0-4  
Property Address: TOWN HILL RD

Mailing Address: COSTIGAN JAMES V COSTIGAN MARY  
K  
7 RODERICK LANE  
GARDEN CITY, NY 11530

Parcel Number: 408-0-5  
CAMA Number: 408-0-5  
Property Address: TOWN HILL RD

Mailing Address: HOLT HENRY G  
PO BOX 89  
VINAL HAVEN, ME 04863-0089



www.cai-tech.com

Data shown on this report is provided for planning and informational purposes only. The municipality and CAI Technologies are not responsible for any use for other purposes or misuse or misrepresentation of this report.

Abutters List, Notification and  
Certificate of Mailings

**NOTIFICATION TO ABUTTERS UNDER THE  
MASSACHUSETTS WETLANDS PROTECTION ACT**

In accordance with the second paragraph of Massachusetts General Laws Chapter 131, Section 40, you are hereby notified of the following:

The name of the applicant is **SAMA Productions LLC.**

The applicant has filed a Notice of Intent with the Sandisfield Conservation Commission seeking permission to remove, fill, dredge or alter an Area Subject to Protection under the Wetlands Protection Act (General Laws Chapter 131, Section 40).

The street address of the property where the activity is proposed is: **Abby Road** and the Town's Assessor's Map/Lot is: **408/31.**

A brief description of the proposed work is **the construction commercial greenhouses along Abby Road on a 46 acre lot.**

Copies of the Notice of Intent may be **examined** at the Sandisfield Conservation Commission office, Town Hall-66sandisfield road, Sandisfield. For more information, please call the Commission office at 413-258-4711Ext #.

Copies of the Notice of Intent may be **obtained** from either (check one) the applicant \_\_\_ or the applicant's representative **X**, by calling this telephone number 413-443-3537 between the hours of 9:00 am and 4:00 pm, Monday through Friday. *NOTE: Provider may charge for the cost of reproduction.*

Information regarding the date, time and place of the public hearing regarding this application may be obtained from the Sandisfield Conservation Commission by calling 413-258-4711 Ext #. Meetings are usually held on the 3<sup>rd</sup> Tuesday of each month.

Notice of the public hearing, including its date, time and place will be published at least five (5) days in advance in the **Berkshire Eagle** under the legal notice section.

Notice of the public hearing, including its date, time and place will be posted in City Hall at least forty-eight (48) hours in advance.

The date that this notification was sent to abutters is **March 9, 2021.**  
Month/Day/Year

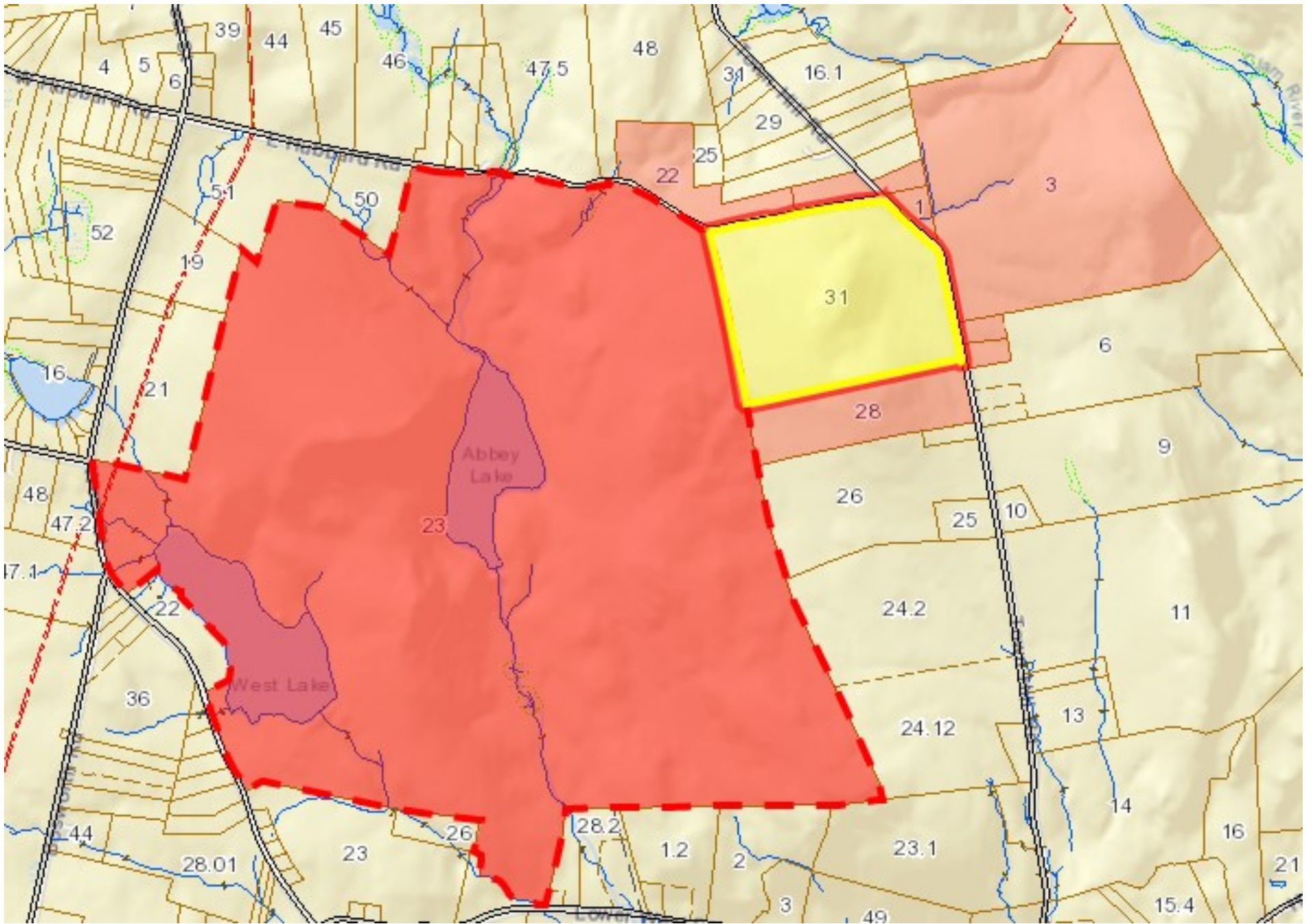
A completed copy of this form and a list of the abutters to whom it was sent shall be submitted by the applicant to the Sandisfield Conservation Commission with the Notice of Intent.

You may also contact the nearest Department of Environmental Protection Regional Office at 413-784-1100 for more information about this application or the Wetlands Protection Act.

Western Region: 413-784-1100  
Northeast Region: 617-935-2160

Central Region: 508-792-7650  
Southeast Region: 508-946-2800





Map of notified abutters

**Attachment A**  
Survey of Property

We, WOLODYMYR DUTKA and STEPHANIA DUTKA, husband and wife, both of 211 State Road in Kerhonkson, in the County of Ulster and State of New York, for consideration paid in the amount of ONE HUNDRED THIRTEEN THOUSAND AND NO/100 (\$113,000.00) DOLLARS, GRANT to FRANK J. BOBRYK and PAUL P. BOBRYK, both of whom have a residence and post office address at 35 Boswell Street, in Stratford, in the County of Fairfield, and State of Connecticut, to be held by them as joint tenants and not as tenants in common, with WARRANTY COVENANTS, the land with the buildings thereon situated in the Town of Sandisfield, Berkshire County, Massachusetts, bounded and described as follows:

*For  
Lia Penders  
See  
Bk 607 Pg 74*

*For  
Estate  
Affidavit  
See  
Bk 1515  
Pg 107*

Beginning at a point on the westerly line of Town Hill Road which marks the northeast corner of land now or formerly of Anthony and Nora Parilli, which point is the southeast corner of the parcel being conveyed; thence westerly along said Parilli's north line 2850 feet more or less to a concrete bound set in the ground on the easterly line of land of the Commonwealth of Massachusetts required by taking dated June 30, 1965 and recorded in the Southern Berkshire Registry of Deeds in Book 347, Page 535; thence northerly along said easterly line of the said Commonwealth of Massachusetts the following two courses: North 2 degrees 05 minutes 25 seconds East 1,863.74 feet to a concrete bound set in the ground; and North 00 degrees 42 minutes 50 seconds East 1,176.96 feet more or less to a concrete bound set in the southerly line of Hubbard Road; thence easterly and southeasterly along said southerly line of Hubbard Road to the intersection of that road with the westerly line of Town Hill Road; thence southerly along the said westerly line of said Town Hill Road to the point or place of beginning.

Containing 200 acres, more or less.

Being all and the same premises as were conveyed to Wolodymyr Dutka and Stephania Dutka by Joseph V. Haber and Amalia H. Yalch Haber by Warranty Deed dated September 26, 1966, and recorded in said Registry of Deeds in Book 354, Page 69, however the same may be therein described, except for the parcel of land consisting of 8 acres, more or less, which was conveyed by the said Dutka et ux to Joseph V. Haber et ux by deed dated September 26, 1966, and recorded in the said Registry of Deeds in Book 354, Page 71.

Being also all and the same premises described in Parcel One in a deed from Jane L. Stratton and Michael Waters to

ROBERT J. DELANEY



1973 00390398

Joseph Frankel and Samuel Neidorf dated November 12, 1918 and recorded in the said Registry of Deeds in Book 221, Page 401, however the same may be otherwise described.

For a sketch of the premises herein conveyed, reference is made to the map accompanying the taking by the Commonwealth of Massachusetts, Water Resources Commission, dated June 30, 1965, and recorded in the said Registry of Deeds in Map File No. 43, being Sheet 5 of 6 on said map.

Taxes for the year 1973 having been apportioned among the parties, the grantees herein assume and agree to pay the same in full.

WITNESS our hands and seals this 7<sup>th</sup> day of September, 1973.

*Wolodymyr Dutka*

Wolodymyr Dutka

BERKSHIRE SOUTH  
COMMONWEALTH OF MASSACHUSETTS  
DEEDS & ENCISE  
*Cancelled*  
SEP 13 1973  
19000

BERKSHIRE SOUTH  
COMMONWEALTH OF MASSACHUSETTS  
DEEDS & ENCISE  
*Cancelled*  
SEP 13 1973  
3000

*Stephanie Dutka*

Stephania Dutka

BERKSHIRE SOUTH  
COMMONWEALTH OF MASSACHUSETTS  
DEEDS & ENCISE  
*Cancelled*  
SEP 13 1973  
17765

COMMONWEALTH OF MASSACHUSETTS

Berkshire, ss.

*September 7*, 1973.

Then personally appeared the above-named Wolodymyr Dutka and Stephanina Dutka and acknowledged the foregoing instrument to be their free act and deed, before me,

Received Southern Berkshire Registry of Deeds  
September 7, 1973 at 1:10 p.m.

*Clair K. Height*

*Robert J. Donelan*

Robert J. Donelan  
Notary Public

Sr. Clerk. My commission expires: September 21, 1974.  
Chap. 36, Sec. 8

**Attachment B**  
Survey Map



**Attachment C**  
Wetland Report

February 3, 2021

Via email: [generalpushing@gmail.com](mailto:generalpushing@gmail.com)  
[Gwheck21@gmail.com](mailto:Gwheck21@gmail.com)

John Heck  
SAMA Productions LLC

RE: Resource Area Delineation  
Town Hill Road / Abbey Road  
Map 408, Lot 31  
Sandifield, MA  
SKDG Project No. 200181

Dear Mr. Heck;

On November 13, 2020, SK Design Group, Inc. (SKDG) conducted a resource area delineation on the above referenced site. The site consists of an undeveloped, forested parcel of land at the southwest corner of the intersection of Town Hill Road and Abbey Road in Sandisfield, Massachusetts. Town Hill Road abuts the east property line of the property while Abbey Hill is located along the north property line. The property slopes from upward from both Town Hill and Abbey Roads to the top of Town Hill. Two (2) areas of Bordering Vegetated Wetland (BVW) were identified on the property, one (1) on the far western portion and one (1) at the intersection of Abbey and Town Hill Roads.

The area of BVW identified on the western portion of the property (BVW "A") is classified as a Forested Wetland system. This BVW area was directly influenced by groundwater elevation and topography of the surrounding area. Multiple water carrying channels (Bank) are located within this BVW area. There were several indicators of hydrology present in this BVW areas, including, areas of inundation, hydric soils with a sapric O-horizon, hydrophytic plant species, saturation and standing water within the top 12 inches of soil, and mottling and gleying in the soils. The BVW area was dominated by Sphagnum (*Sphagnum spp.*), Yellow Birch (*Betula alleghensis*), and Eastern Hemlock (*Tsuga canadensis*). The soils observed in the BVW area revealed a sapric O-horizon, a low chroma color matrix with mottling and concretions in the A and B horizons, saturation within 12-inches of the surface and gleying in the B-Horizon. These characteristics are typical of hydric soil conditions (See field data forms for soil profiles). Soil profiles were recorded along the BVW and upland boundaries on the subject property and are represented in the Appendix G Forms attached hereto. Other soil borings were taken throughout the wetland area to verify soil conditions but were not recorded. The BVW area identified on November 13, 2020 was marked in the field utilizing blue flagging numbered sequentially and field surveyed.

The area of BVW identified on the eastern portion of the property (BVW "B") is also classified as a Forested Wetland system. This BVW area was directly influenced by groundwater elevation, hydrology of an unnamed intermittent stream, and topography of the surrounding area.



There were several indicators of hydrology present in this BVW areas, including, areas of inundation, hydric soils with a sapric O-horizon, hydrophytic plant species, saturation and standing water within the top 12 inches of soil, and mottling and gleying in the soils. The BVW area was dominated by Sphagnum (*Sphagnum spp.*), Sensitive Fern (*Onoclea sensibilis*), Yellow Birch (*Betula alleghensis*), and Eastern Hemlock (*Tsuga canadensis*). The soils observed in the BVW area revealed a sapric O-horizon, a low chroma color matrix with mottling and concretions in the A and B horizons, saturation within 12-inches of the surface and gleying in the B-Horizon. These characteristics are typical of hydric soil conditions (See field data forms for soil profiles). Soil profiles were recorded along the BVW and upland boundaries on the subject property and are represented in the Appendix G Forms attached hereto. Other soil borings were taken throughout the wetland area to verify soil conditions but were not recorded. The BVW area identified on November 13, 2020 was marked in the field utilizing blue flagging numbered sequentially and field surveyed.

Forested upland areas were observed surrounding both BVW areas on the property. The uplands were dominated by Evergreen Woodfern (*Dryopteris intermedia*), Christmas Fern (*Polystichum acrostichoides*), Mountain Laurel (*Kalmia latifolia*), American Beech (*Fagus grandifolia*), Northern Red Oak (*Quercus rubra*), Black Cherry (*Prunus serotina*), and Eastern White Pine (*Pinus strobus*). The upland soils revealed a high chroma color matrix with no redoximorphic features, typical of non-hydric soil conditions. Upland soil profiles were recorded on the Appendix G field data sheets attached hereto. Other soil profiles were collected throughout the upland areas to verify soil conditions but were not recorded.

If you should have any questions concerning this matter, please contact our office.

Sincerely,  
S-K Design Group, Inc.



Brett W. Kamienski,  
Wetland Scientist



**Section II. Indicators of Hydrology    A-15 (BVW)**

**Hydric Soil Interpretation**

**1. Soil Survey**

Is there a published soil survey for this site?  yes  no  
 title/date: Soil Survey of Berkshire County, 1988  
 map number: 48  
 soil type mapped: PmC - Peru-Marlow Association  
 hydric soil inclusions: No

Are field observations consistent with soil survey? yes  no   
 Remarks:

BVW located in drainageway and depression area.

**2. Soil Description**

Horizon	Depth	Matrix Color	Mottles Color
A	0-6"	10 YR 3/1	
Bw1	6-12"	10 YR 4/2	10YR 4/6~40%

Remarks:

Redox @ 6"+

**3. Other:**

Conclusion: Is soil hydric?  yes  no

**Other Indicators of Hydrology: (check all that apply & describe)**

- Site Inundated: \_\_\_\_\_
- Depth to free water in observation hole: \_\_\_\_\_
- Depth to soil saturation in observation hole: <12"
- Water marks: \_\_\_\_\_
- Drift lines: \_\_\_\_\_
- Sediment Deposits: \_\_\_\_\_
- Drainage patterns in BVW: \_\_\_\_\_
- Oxidized rhizospheres: \_\_\_\_\_
- Water-stained leaves: Evidence of standing water
- Recorded Data (streams, lake, or tidal gauge; aerial photo; other):  
 \_\_\_\_\_  
 \_\_\_\_\_
- Other: \_\_\_\_\_

**Vegetation and Hydrology Conclusion**

	Yes	No
Number of wetland indicator plants ≥ # of non-wetland indicator plants	<u>X</u>	_____
<b>Wetland hydrology present:</b>		
Hydric soil present	<u>X</u>	_____
Other indicators of hydrology present	<u>X</u>	_____
<b>Sample location is in a BVW</b>	<u>X</u>	_____

*Submit this form with the Request for Determination of Applicability or Notice of Intent.*



**Section II. Indicators of Hydrology    A-15 (UP)**

**Hydric Soil Interpretation**

**1. Soil Survey**

Is there a published soil survey for this site?  yes  no  
 title/date: Soil Survey of Berkshire County, 1988  
 map number: 48  
 soil type mapped: PmC – Peru-Marlow Association  
 hydric soil inclusions: No

Are field observations consistent with soil survey?  yes  no  
 Remarks:

Well-drained soil on glacial till uplands

**2. Soil Description**

Horizon	Depth	Matrix Color	Mottles Color
A	0-7"	10 YR 3/2	
Bw1	7-14"	10 YR 3/4	

Remarks:

No redox to 14", stony soils

**3. Other:**

Conclusion: Is soil hydric? yes  no

**Other Indicators of Hydrology: (check all that apply & describe)**

- Site Inundated: \_\_\_\_\_
- Depth to free water in observation hole: \_\_\_\_\_
- Depth to soil saturation in observation hole: \_\_\_\_\_
- Water marks: \_\_\_\_\_
- Drift lines: \_\_\_\_\_
- Sediment Deposits: \_\_\_\_\_
- Drainage patterns in BVW: \_\_\_\_\_
- Oxidized rhizospheres: \_\_\_\_\_
- Water-stained leaves: \_\_\_\_\_
- Recorded Data (streams, lake, or tidal gauge; aerial photo; other):  
 \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

**Vegetation and Hydrology Conclusion**

	Yes	No
Number of wetland indicator plants ≥ # of non-wetland indicator plants	_____	<u>  X  </u>
<b>Wetland hydrology present:</b>		
Hydric soil present	_____	<u>  X  </u>
Other indicators of hydrology present	_____	<u>  X  </u>
<b>Sample location is in a BVW</b>	_____	<u>  X  </u>

*Submit this form with the Request for Determination of Applicability or Notice of Intent.*



**Section II. Indicators of Hydrology A-5 (BVW)**

**Hydric Soil Interpretation**

**1. Soil Survey**

Is there a published soil survey for this site? **yes** no  
 title/date: Soil Survey of Berkshire County, 1988  
 map number: 48  
 soil type mapped: PmC – Peru-Marlow Association  
 hydric soil inclusions: No

Are field observations consistent with soil survey? yes **no**  
 Remarks:

BVW located in low lying drainageway

**2. Soil Description**

Horizon	Depth	Matrix Color	Mottles Color
A	0-5"	10 YR 3/1	
Bw1	5-12"	10 YR 5/2	7.5YR 4/6~20% 10YR 2/1<5%

Remarks:

Redox @ 5"+

Bw1 – silt & clay, tight & stony

**3. Other:**

Conclusion: Is soil hydric? **yes** no

**Other Indicators of Hydrology: (check all that apply & describe)**

- Site Inundated: Portions of BVW
- Depth to free water in observation hole: <12"
- Depth to soil saturation in observation hole: <12"
- Water marks: \_\_\_\_\_
- Drift lines: \_\_\_\_\_
- Sediment Deposits: \_\_\_\_\_
- Drainage patterns in BVW: \_\_\_\_\_
- Oxidized rhizospheres: \_\_\_\_\_
- Water-stained leaves: Evidence of standing water
- Recorded Data (streams, lake, or tidal gauge; aerial photo; other):  
\_\_\_\_\_
- Other: Pockets of sapric O horizon

**Vegetation and Hydrology Conclusion**

	Yes	No
Number of wetland indicator plants ≥ # of non-wetland indicator plants	<u>X</u>	_____
<b>Wetland hydrology present:</b>		
Hydric soil present	<u>X</u>	_____
Other indicators of hydrology present	<u>X</u>	_____
<b>Sample location is in a BVW</b>	<u>X</u>	_____

Submit this form with the Request for Determination of Applicability or Notice of Intent.

# MassDEP Bordering Vegetated Wetland (310 CMR 10.55) Delineation Field Data Form

Applicant: Fulcrum Prepared by: SK Design Group, Inc. Project location: Town Hill Rd (408-31) DEP File #: \_\_\_\_\_  
Sandisfield

Check all that apply:

- Vegetation alone presumed adequate to delineate BVW boundary: fill out Section I only
- Vegetation and other indicators of hydrology used to delineate BVW boundary: fill out Sections I and II
- Method other than dominance test used (attach additional information)

## Section I.

Vegetation	Observation Plot Number: <b>A-5 (UP)</b>		Transect Number: 5	Date of Delineation: 11/13/2020
A. Sample Layer & Plant Species (by common/scientific name)	B. Percent Cover (or basal Area)	C. Percent Dominance	D. Dominant Plant (yes or no)	E. Wetland Indicator Category*

(Midpoint)

### Groundcover:

Evergreen Woodfern ( <i>Dryopteris intermedia</i> )	10.5	22%	Yes	FACU
Christmas Fern ( <i>Polystichum acrostichoides</i> )	38.0	78%	Yes	FACU

### Shrubs/Sapling:

American Beech ( <i>Fagus grandifolia</i> )	3.0	50%	Yes	FACU
*Eastern Hemlock ( <i>Tsuga canadensis</i> )	3.0	50%	Yes	FACW

### Trees:

American Beech ( <i>Fagus grandifolia</i> )	38.0	33%	Yes	FACU
Black Cherry ( <i>Prunus serotina</i> )	63.0	55%	Yes	FACU
*Eastern Hemlock ( <i>Tsuga canadensis</i> )	10.5	9%	No	FACW
*Yellow Birch ( <i>Betula alleghensis</i> )	3.0	3%	No	FAC

\* Use an asterisk to mark wetland indicator plants: plant species listed in the Wetlands Protection Act (MGL c.131, s.40); plants in the genus *Sphagnum*; plants listed as FAC, FAC+, FACW-, FACW, FACW+, or OBL; or plants with physiological or morphological adaptations. If any plants are identified as wetland indicator plants due to physiological or morphological adaptations, describe the adaptation next to the asterisk.

## Vegetation conclusion:

Number of dominant wetland indicator plants: 1                      Number of dominant non-wetland indicator plants: 5

Is the number of dominant wetland plants equal to or greater than the number of dominant non-wetland plants? yes **no**

If vegetation alone is presumed adequate to delineate the BVW boundary, submit this form with the Request for Determination of Applicability or Notice of Intent



**Section II. Indicators of Hydrology A-5 (UP)**

**Hydric Soil Interpretation**

**1. Soil Survey**

Is there a published soil survey for this site?  yes  no  
 title/date: Soil Survey of Berkshire County, 1988  
 map number: 48  
 soil type mapped: PmC – Peru-Marlow Association  
 hydric soil inclusions: No

Are field observations consistent with soil survey?  yes  no  
 Remarks:

Well drained stony soil on glacial till uplands.

**2. Soil Description**

Horizon	Depth	Matrix Color	Mottles Color
A	0-7"	10 YR 3/2	
Bw1	7-14"	10 YR 3/4	

Remarks:

**3. Other:**

Conclusion: Is soil hydric? yes  no

**Other Indicators of Hydrology: (check all that apply & describe)**

- Site Inundated: \_\_\_\_\_
- Depth to free water in observation hole: \_\_\_\_\_
- Depth to soil saturation in observation hole: \_\_\_\_\_
- Water marks: \_\_\_\_\_
- Drift lines: \_\_\_\_\_
- Sediment Deposits: \_\_\_\_\_
- Drainage patterns in BVW: \_\_\_\_\_
- Oxidized rhizospheres: \_\_\_\_\_
- Water-stained leaves: \_\_\_\_\_
- Recorded Data (streams, lake, or tidal gauge; aerial photo; other):  
 \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

**Vegetation and Hydrology Conclusion**

	Yes	No
Number of wetland indicator plants ≥ # of non-wetland indicator plants	_____	<u>  X  </u>
<b>Wetland hydrology present:</b>		
Hydric soil present	_____	<u>  X  </u>
Other indicators of hydrology present	_____	<u>  X  </u>
<b>Sample location is in a BVW</b>	_____	<u>  X  </u>

*Submit this form with the Request for Determination of Applicability or Notice of Intent.*



**Section II. Indicators of Hydrology B-20 (BVW)**

**Hydric Soil Interpretation**

**1. Soil Survey**

Is there a published soil survey for this site?  yes  no  
 title/date: Soil Survey of Berkshire County, 1988  
 map number: 48  
 soil type mapped: PoB – Pillsbury loam  
 hydric soil inclusions: Yes

Are field observations consistent with soil survey?  yes  no  
 Remarks:

BVW located in drainageway

**2. Soil Description**

Horizon	Depth	Matrix Color	Mottles Color
Oa	3-0"	10 YR 2/1	
A	0-4"	10 YR 3/1	
Bw1	4-12"	10 YR 5/2	7.5YR 4/6~40% 10YR 2/1<5%

Remarks:

Sapric O horizon

**3. Other:**

Conclusion: Is soil hydric?  yes  no

**Other Indicators of Hydrology: (check all that apply & describe)**

- Site Inundated: Portions of BVW
- Depth to free water in observation hole: <12"
- Depth to soil saturation in observation hole: <12"
- Water marks: \_\_\_\_\_
- Drift lines: \_\_\_\_\_
- Sediment Deposits: \_\_\_\_\_
- Drainage patterns in BVW: \_\_\_\_\_
- Oxidized rhizospheres: \_\_\_\_\_
- Water-stained leaves: Evidence of standing water
- Recorded Data (streams, lake, or tidal gauge; aerial photo; other):  
\_\_\_\_\_
- Other: Pockets of sapric O horizon

**Vegetation and Hydrology Conclusion**

	Yes	No
Number of wetland indicator plants ≥ # of non-wetland indicator plants	<u>X</u>	_____
<b>Wetland hydrology present:</b>		
Hydric soil present	<u>X</u>	_____
Other indicators of hydrology present	<u>X</u>	_____
<b>Sample location is in a BVW</b>	<u>X</u>	_____

Submit this form with the Request for Determination of Applicability or Notice of Intent.



**Section II. Indicators of Hydrology B-20 (UP)**

**Hydric Soil Interpretation**

**1. Soil Survey**

Is there a published soil survey for this site?  yes  no  
 title/date: Soil Survey of Berkshire County, 1988  
 map number: 48  
 soil type mapped: PmC – Peru-Marlow Association  
 hydric soil inclusions: No

Are field observations consistent with soil survey?  yes  no  
 Remarks:

Well-drained soil on glacial till uplands

**2. Soil Description**

Horizon	Depth	Matrix Color	Mottles Color
A	0-7"	10 YR 3/2	
Bw1	7-14"	10 YR 3/4	

Remarks:

No redox to 14", stony soils

**3. Other:**

Conclusion: Is soil hydric? yes  no

**Other Indicators of Hydrology: (check all that apply & describe)**

- Site Inundated: \_\_\_\_\_
- Depth to free water in observation hole: \_\_\_\_\_
- Depth to soil saturation in observation hole: \_\_\_\_\_
- Water marks: \_\_\_\_\_
- Drift lines: \_\_\_\_\_
- Sediment Deposits: \_\_\_\_\_
- Drainage patterns in BVW: \_\_\_\_\_
- Oxidized rhizospheres: \_\_\_\_\_
- Water-stained leaves: \_\_\_\_\_
- Recorded Data (streams, lake, or tidal gauge; aerial photo; other):  
 \_\_\_\_\_  
 \_\_\_\_\_

<b>Vegetation and Hydrology Conclusion</b>		
	Yes	No
Number of wetland indicator plants ≥ # of non-wetland indicator plants	_____	<u>X</u>
<b>Wetland hydrology present:</b>		
Hydric soil present	_____	<u>X</u>
Other indicators of hydrology present	_____	<u>X</u>
<b>Sample location is in a BVW</b>	_____	<u>X</u>

*Submit this form with the Request for Determination of Applicability or Notice of Intent.*

**Attachment D**  
Stormwater Report



# Checklist for Stormwater Report

## A. Introduction

**Important:** When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the [Massachusetts Stormwater Handbook](#). The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.<sup>1</sup> This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8<sup>2</sup>
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

<sup>1</sup> The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

<sup>2</sup> For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



# Checklist for Stormwater Report

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## B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

*Note:* Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

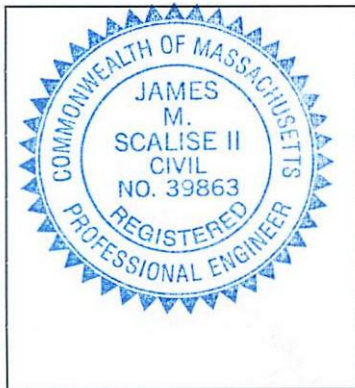
A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

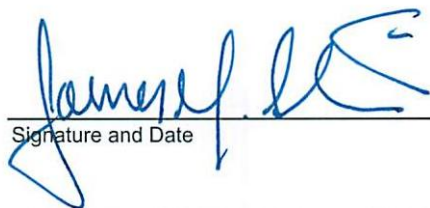
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### Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



 3/1/21  
Signature and Date

---

### Checklist

**Project Type:** Is the application for new development, redevelopment, or a mix of new and redevelopment?

- New development
- Redevelopment
- Mix of New Development and Redevelopment





# Checklist for Stormwater Report

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## Checklist (continued)

**LID Measures:** Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- No disturbance to any Wetland Resource Areas
- Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- Reduced Impervious Area (Redevelopment Only)
- Minimizing disturbance to existing trees and shrubs
- LID Site Design Credit Requested:
  - Credit 1
  - Credit 2
  - Credit 3
- Use of “country drainage” versus curb and gutter conveyance and pipe
- Bioretention Cells (includes Rain Gardens)
  - Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- Treebox Filter
- Water Quality Swale
- Grass Channel
- Green Roof
- Other (describe): \_\_\_\_\_

### Standard 1: No New Untreated Discharges

- No new untreated discharges
- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



# Checklist for Stormwater Report

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## Checklist (continued)

### Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

### Standard 3: Recharge

- Soil Analysis provided.
- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.
  - Static
  - Simple Dynamic
  - Dynamic Field<sup>1</sup>
- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
  - Site is comprised solely of C and D soils and/or bedrock at the land surface
  - M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
  - Solid Waste Landfill pursuant to 310 CMR 19.000
  - Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

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<sup>1</sup> 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



# Checklist for Stormwater Report

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## Checklist (continued)

### Standard 3: Recharge (continued)

- The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

### Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
  - Provisions for storing materials and waste products inside or under cover;
  - Vehicle washing controls;
  - Requirements for routine inspections and maintenance of stormwater BMPs;
  - Spill prevention and response plans;
  - Provisions for maintenance of lawns, gardens, and other landscaped areas;
  - Requirements for storage and use of fertilizers, herbicides, and pesticides;
  - Pet waste management provisions;
  - Provisions for operation and management of septic systems;
  - Provisions for solid waste management;
  - Snow disposal and plowing plans relative to Wetland Resource Areas;
  - Winter Road Salt and/or Sand Use and Storage restrictions;
  - Street sweeping schedules;
  - Provisions for prevention of illicit discharges to the stormwater management system;
  - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
  - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
  - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
  - Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
    - is within the Zone II or Interim Wellhead Protection Area
    - is near or to other critical areas
    - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
    - involves runoff from land uses with higher potential pollutant loads.
  - The Required Water Quality Volume is reduced through use of the LID site Design Credits.
  - Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



# Checklist for Stormwater Report

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## Checklist (continued)

### Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
  - The  $\frac{1}{2}$ " or 1" Water Quality Volume or
  - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

### Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does **not** cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has **not** been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

### Standard 6: Critical Areas

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.



# Checklist for Stormwater Report

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## Checklist (continued)

### Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
  - Limited Project
  - Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
  - Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
  - Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
  - Bike Path and/or Foot Path
  - Redevelopment Project
  - Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
  - Construction Period Operation and Maintenance Plan;
  - Names of Persons or Entity Responsible for Plan Compliance;
  - Construction Period Pollution Prevention Measures;
  - Erosion and Sedimentation Control Plan Drawings;
  - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
  - Vegetation Planning;
  - Site Development Plan;
  - Construction Sequencing Plan;
  - Sequencing of Erosion and Sedimentation Controls;
  - Operation and Maintenance of Erosion and Sedimentation Controls;
  - Inspection Schedule;
  - Maintenance Schedule;
  - Inspection and Maintenance Log Form.
- A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



# Checklist for Stormwater Report

## Checklist (continued)

### Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- The project is **not** covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

### Standard 9: Operation and Maintenance Plan

- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
  - Name of the stormwater management system owners;
  - Party responsible for operation and maintenance;
  - Schedule for implementation of routine and non-routine maintenance tasks;
  - Plan showing the location of all stormwater BMPs maintenance access areas;
  - Description and delineation of public safety features;
  - Estimated operation and maintenance budget; and
  - Operation and Maintenance Log Form.
- The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
  - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
  - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

### Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.

# STORMWATER MANAGEMENT NARRATIVE

To Accompany Special Permit Application

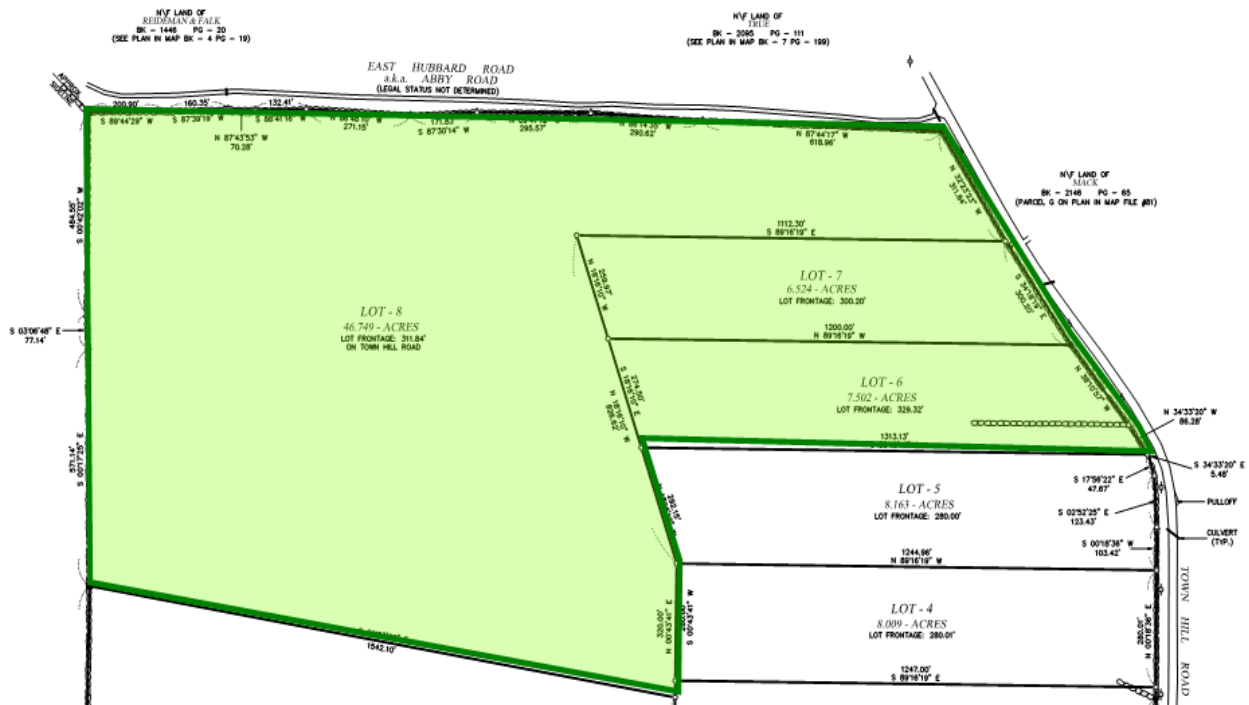
SAMA Productions LLC

Abby Road

Sandisfield, MA

## Introduction

The project site is located along Abby road near the intersection of Abby Road and Town Hill Road in Sandisfield, MA.



*Portion of property survey (Prepared By Kelly Granger and Parsons Assoc.)*

The property contains 60 acres of land, more or less, accessed Abby Road traveling west along Abby road. The existing site is known as Town Hill and is a forested hillside. The entire development is located on lot 8 which includes 46 acres of land.

The proposed project includes the construction of (23) green houses and a 5,000-sf pre-engineered processing building. The project also includes the construction of gravel driveways, with parking areas, grading, utilities, and stormwater infrastructure.

The method to be used to analyze both the existing and proposed drainage flow is outlined in the Soil Conservation Services Technical Release 20 (TR-20) and Soil Conservation Services Technical Release 55 (TR-55) "Urban Hydrology for Small Watersheds", 2<sup>nd</sup> Edition, 1986. The SCS methods are utilized by HydroCAD stormwater modeling software, which provides methods to calculate stormwater runoff volumes, peak rates of runoff and hydrographs for small watersheds. The input data includes site use, published hydrologic soil type and statistical rainfall data. The analysis will show no increase in runoff for a 24-hour, type III storm during a 2-year, 10-year, and 100-year storm event.

The time of concentration calculations are based upon the travel time from the hydraulically most distant point within the watershed to the final design point. The times were calculated utilizing the *Soil Conservation Services Technical Note Hydrology No. N4* dated July 1986.

The proposed drain pipes will be high density polyethylene (HDPE), more specifically we have specified Advance Drainage System (ADS N-12). This type of pipe is light weight, durable, and provides the capacity of other types of pipe (i.e. metal, concrete, etc.)

The stormwater design mimics existing drainage patterns and is design following the Mass Department of Environmental Protection Stormwater Management Standards.

*Standard #1-No new stormwater conveyances (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.*

The project as proposed includes the collection, treatment and mitigation of stormwater runoff from all developed areas and by-passes upgradient stormwater to insure hydrology is not interrupted to downgradient areas. The onsite drainage channel is designed at less than 5% slope. The conveyance is designed to have sufficient capacity to convey runoff safely during the 10-year storm. The maximum calculated velocity is 4.4 fps. The drainage channel will be planted with tall fescue grass mix and a grass legume mixture. This ground cover will prevent erosion. Should erosive conditions persist, check dams shall be installed. Check dams shall be installed at 50' intervals 18 inches high and well graded 6-inch minus stone.

*Standard #2-Stormwater management systems shall be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates. This Standard may be waived for discharges to land subject to coastal storm flowage as defined in 310 CMR 10.04.*



Peak flows are managed according to the standards as outlined below:

	<b>Westerly DP</b>	<b>Westerly DP</b>
<b>Event</b>	<b>Existing Outflow (cfs)</b>	<b>Proposed Outflow (cfs)</b>
2 year	5.64	5.55
10 year	13.12	12.09
100 year	33.83	31.15
	<b>Easterly DP</b>	<b>Easterly DP</b>
<b>Event</b>	<b>Existing Outflow (cfs)</b>	<b>Proposed Outflow (cfs)</b>
2 year	4.94	4.87
10 year	11.48	11.44
100 year	29.58	28.72

*Standard #3 Loss of annual recharge to groundwater shall be eliminated or minimized through the use of infiltration measures.*

Infiltration volume is provided using the static method. All infiltration BMPs are proposed in fill areas onsite. Other infiltration measures are proposed to collect stormwater onsite but are not included in the calculations for compliance under this standard. Construction of the BMPs in fill insures the BMPS meet the separation to groundwater requirements under this section. The below summary indicates the standard is met.

### Groundwater recharge

Target Depth Factor	0.25	inch
	0.021	feet
impervious area	1.45	acres (West)
sum	1.45	
Area	62,998	square feet
<b>Recharge volume required</b>	<b>1,312</b>	<b>Cubic feet</b>

### Infiltration trenches proposed

Greenhouse group 1	0	Cubic feet
Greenhouse group 2	989	Cubic feet
Greenhouse group 3	897	Cubic feet
<b>Volume Provided</b>	<b>1,886</b>	<b>Cubic feet</b>

The infiltration trenches installed above groundwater by 2 feet minimum will include 1830 LF of trench. The design includes a trench 18 inches deep by 12 inches wide with a 4-inch perforated pipe. This provides 1,886 cubic feet of storage meeting the standard.

Onsite soils were investigated by advancing 12 test excavations. Onsite soils were characterized as fine sandy loam to depths of greater than 10 feet in all tested areas. Groundwater depth varied from 18"-24" below existing grade based upon >5% soil mottling. Soil permeability is 0.6 in/hr. for design purposes. Infiltration BMPs require a minimum of 0.17 in/hr. therefore this criterion is met.

The westerly subarea is subject to jurisdiction under the Wetlands Protection Act. The westerly subarea includes 1.446 acres of impervious area with a total area of 14.4 acres. Approximately 10,000 sf of area in the gravel yard will not be recharged. This amounts to  $10,000 \text{ SF} / (1.45 \times 43560) = 15\%$ .

Solution:

$$R_v = F \times \text{Impervious Area therefore } R_v = (0.25/12)(1.45)(43560)$$

$$R_v = 1315 \text{ CF}$$

Adjusted Volume is  $(1/.848) * 1315 \text{ cf} = 1550.1 \text{ cf}$  required.

1586 cf provided. **The design meets the standard.**

Infiltration BMPs must drawdown within 72 hours. The following calculation for drawdown is provided:

$$\text{Time} = \frac{R_v}{(K) (\text{Bottom Area})}$$

$$R_v = 1884 \text{ cf}$$

$$K = 0.17 \text{ conservatively}$$

$$\text{Bottom Area} = 150 * 1' * 46 * 26.5\% = 1830 \text{ SF}$$

$$\text{Time} = 1884 / (0.17/12) \times 1830 \text{ SF} = 72 \text{ hours. } \mathbf{\underline{\text{This standard is met.}}}$$

#### *Standard #4-Water Quality*

Stormwater will be treated using best management practices. The project site has been separated into several sub-areas to manage stormwater. Upstream undeveloped forests are by-passed around the development to avoid mixing with developed area stormwater. Clean by-pass water will replenish the downgradient wetlands with hydrology. Further rooftop runoff is also captured, infiltrated, and discharged separately to avoid the treatment train and avoid mingling of "contaminated" and "uncontaminated" runoff. The remaining development area is reduced to 4± acres of impervious area including both the easterly and westerly subareas. The Westerly sub area includes about 2.9 acres of gravel drive and unaccounted for

rooftop that is collected and treated. The treatment train for the westerly greenhouse group #1 and #3 is as follows:

Location:

B	C	D	E	F
BMP <sup>1</sup>	TSS Removal Rate <sup>1</sup>	Starting TSS Load*	Amount Removed (C*D)	Remaining Load (D-E)
Bioretention Area	0.90	1.00	0.90	0.10
Extended Dry Detention Basin	0.50	0.10	0.05	0.05
	0.00	0.05	0.00	0.05
	0.00	0.05	0.00	0.05
	0.00	0.05	0.00	0.05

**TSS Removal Calculation Worksheet**

**Total TSS Removal =**  Separate Form Needs to be Completed for Each Outlet or BMP Train

Project:   
 Prepared By:   
 Date:

\*Equals remaining load from previous BMP (E) which enters the BMP

The easterly area includes the metal building and a portion of greenhouse group #2. The treatment train is as follows:

Location:

**TSS Removal Calculation Worksheet**

B BMP <sup>1</sup>	C TSS Removal Rate <sup>1</sup>	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Bioretention Area	0.90	1.00	0.90	0.10
Extended Dry Detention Basin	0.50	0.10	0.05	0.05
	0.00	0.05	0.00	0.05
	0.00	0.05	0.00	0.05
	0.00	0.05	0.00	0.05

**Total TSS Removal =**

95%

Separate Form Needs to be Completed for Each Outlet or BMP Train

Location:

**TSS Removal Calculation Worksheet**

B BMP <sup>1</sup>	C TSS Removal Rate <sup>1</sup>	D Starting TSS Load*	E Amount Removed (C*D)	F Remaining Load (D-E)
Treebox Filter	0.80	1.00	0.80	0.20
Extended Dry Detention Basin	0.50	0.20	0.10	0.10
	0.00	0.10	0.00	0.10
	0.00	0.10	0.00	0.10
	0.00	0.10	0.00	0.10

**Total TSS Removal =**

90%

Separate Form Needs to be Completed for Each Outlet or BMP Train

WESTERLY:

Water quality treatment volume:

$$V_{wQ} = (D_{wQ}/12 \text{ inch/ft}) * (A_{imp} * 43560 \text{ Sf/Acre})$$

$$V_{wQ} = (1/2\text{-inch}/12 \text{ in/ft}) * (2.9 * 43560 \text{ sf/acre})$$

$$= 5264 \text{ cf}$$

$$\text{Volume of 5P (forebay)} = 13547 \text{ CF}$$

$$5264 < 13547 \text{ OK}$$

EASTERLY:

Water quality treatment volume:

$$\begin{aligned} V_{wQ} &= (1/2\text{-inch}/12 \text{ in/ft}) * (1.0 * 43560 \text{ sf/acre}) \\ &= 1815 \text{ CF} < (0.109 \text{ AF} * 43560) = 4748 \text{ OK} \end{aligned}$$

The parking area around the building is supplemented with tree box filters thus it has a similar treatment train to the above.

*Standard #5-Land Uses with Higher Potential Pollutant Loads (LUPPL)*

The proposed project is not considered a LUPPL therefore this does not apply.

*Standard #6-Critical Areas*

The proposed project is not located in, nor discharge to a Critical Area, therefore this does not apply.

*Standard #7-Redevelopment*

The proposed project is not considered a redevelopment project therefore this does not apply.

*Standard #8-Construction Period Controls*

The proposed site disturbance is greater than 1 acre, therefore a National Pollutant Discharge Elimination System (NPDES) Stormwater Pollution Prevention Plan (SWPPP) will be filed. This will be completed prior to land disturbance. In the interim, an Erosion Control plan with details is included herein. Silt fence with straw wattles are located along all downgradient limits of work, sediment traps are proposed in each catch basin and each building will be phased to minimize site disturbance.

*Standard #9-Operation and Maintenance*

Swales and ponds shall be inspected annually, and sediment should be removed when it has accumulated to 12" in depth.

**ILLCIT DISCHARGE COMPLIANCE STATEMENT**

**SAMA Productions LLC**

**Abby Road Sandisfield, MA**

This statement is provided in accordance with the provisions of the Massachusetts Stormwater Management Standard 10 and of the Massachusetts Stormwater Management Handbook.

Note the following:

All stormwater management systems contain no connection to the site's wastewater sewer system or to any other non-stormwater collection system.

Groundwater collection systems on the site are not connected to the site's wastewater sewer system or to any other non-stormwater collection system.

The facility's Operations & Maintenance Plan is designed to prevent any discharge of non-stormwater to the drainage system.

Any illicit discharges identified during or after construction will be immediately disconnected.

Date: February 2021

**Operation and Maintenance Plan**

**of**

**The Stormwater System**

**For**

**SAMA Productions**

**Located at**

**Abby Road**

**Sandisfield, Massachusetts**

**February 2021**

## Background

Once rain reaches the ground, what happens next depends largely on land cover type. Rain falling in a forest is slowed, filtered, and absorbed as it makes its way into the ground or to the nearest stream, river, or reservoir. In contrast, hard, impervious surfaces such as roof tops and roads send stormwater rushing to the nearest ditch, culvert, storm drain, and stream. This stormwater picks up pollutants, such as heavy metals, gas, oil, nutrients, and sediment along the way. Uncontrolled stormwater erodes stream banks, causes flooding, and carries nutrients and sediment downstream. An excess of nutrients contributes to the expansion of oxygen-depleted “dead zones” in local waterways. The solution to improve the quality and reduce the quantity of stormwater runoff before it enters natural waterways are referred to as Best Management Practices, or BMPs. **BMPs** range from structural facilities, such as ponds, bio-retention areas, and underground vaults, to non-structural practices, such as street sweeping and educational efforts.

Routine maintenance will keep a **BMPs** functioning properly and will pay off in the long run by preventing unnecessary repairs. Also, preventing pollutants from reaching the **BMP** will result in lower maintenance costs and cleaner water.

### Common Routine Maintenance Needs for Most BMPs

- Regular Inspections
- Vegetation Management
- Mowing
- Pest and Weed control
- Removing Sediment Build-Up
- Stabilize Eroded Areas or Bare Spots
- Unwanted Vegetation
- Embankment and Outlet Stabilization
- Debris and Litter Control
- Mechanical Components Maintenance
- Insect Control
- Access Maintenance
- Overall Pond Maintenance
- Regular Inspections

**Each of the above is briefly described below:**

### Vegetation Management

Most **BMPs** rely on vegetation to filter sediment from stormwater. Vegetation also serves to prevent erosion of the banks and stabilize the bottom of the facility. While turf grass is the most common groundcover, many **BMPs** are designed with woody vegetation and wetland plants to increase pollutant removal.



### **Mowing.**

Most grass is hardiest if it is maintained as an upland meadow. Therefore, grass within BMPs (drainage channels) should not be mowed shorter than six to eight inches. Grass should be cut at least twice during the growing seasons and once during the summer.

### **Pest and Weed Control**

To reduce the amount of pollutants reaching the **BMP**, avoid using any fertilization and/or pesticides.

### **Removing Sediment Build-Up.**

Since the vegetation surrounding the **BMP** is designed to trap sediment, it is likely to become laden with sediment. When this happens, the sediment should be removed prior to it rendering the **BMPs** ineffective.

### **Stabilize Eroded Areas or Bare Spots.**

Bare spots should be vigorously raked, backfilled if needed, covered with top soil, and seeded. This is particularly important during the initial construction period.

### **Unwanted Vegetation.**

Some vegetation is destructive to a **BMP**. Keeping dams and bottom areas free of deep-rooted vegetation is critical as roots may destabilize the structure. Consistent mowing and monitoring will control any unwanted vegetation.

### **Embankment and Outlet Stabilization**

A stable embankment is important to ensure that erosion does not contribute to water quality problems and that embankments are not breached resulting in downstream flooding. Maintaining a healthy vegetative cover and preventing the growth of deep-rooted (woody) vegetation on embankment areas is an important component to stabilization. Animal burrows will also deteriorate the structural integrity of an embankment. Muskrats and groundhogs in particular will burrow tunnels up to six inches in diameter. Efforts should be made to control excessive animal burrowing and existing burrows should be filled as soon as possible. Outlet structures are particularly prone to undercutting and erosion. Unchecked, a small problem can easily result in the need to replace the entire structure. A professional engineer should be consulted if sink holes, cracking, wet areas around the outlet pipe, displacement, or rusting of the pipe is observed.

### **Debris and Litter Control**

Regular removal of debris and litter can be expected to help in the following areas: reduce the chance of clogging outlet structures and trash racks; prevent damage to vegetated structures; reduce mosquito breeding habitats; maintain facility appearance; and, reduce conditions for excessive algae growth. Special attention should be given to the removal of floating debris which can clog inlets, outlets, and low-flow orifices.

### **Mechanical Components Maintenance**

Some **BMPs** have mechanical components that need periodic attention - valves, sluice gates, pumps, **anti-vortex devices**, fence gates, locks, and access hatches should be functional at all times. This type of routine maintenance is best left to a professional.

### **Insect Control**

A healthy ecosystem actually promotes biological controls of mosquitoes. However, mosquito and other insect breeding grounds can be created by standing water. Though perceived as a significant nuisance, mosquitoes are not as big a problem as is often thought, and there are ways to address the issue.

The best technique is to ensure that stagnant pools of water do not develop. For **BMPs** that have a permanent pool of water, this means the prompt removal of floatable debris.

The development of a mosquito problem, particularly in dry ponds, infiltration trenches, and rain gardens, is usually an early indication that there is a maintenance problem, such as clogging. In such cases, the infiltration capacity of the **BMP** needs to be increased or sediment needs to be removed.

### **Access Maintenance**

Most **BMPs** are designed so that heavy equipment can safely and easily reach the facility for non-routine maintenance. Routine maintenance of access areas is particularly important since one never knows when emergency access will be needed. Maintenance includes removal of woody vegetation, upkeep of gravel areas, fences, and locks.

### **Overall Pond Maintenance**

An often overlooked aspect of maintenance, especially for wet ponds, is the need to ensure a healthy aquatic ecosystem. A healthy ecosystem should require little maintenance. An indicator of an unhealthy system is excessive algal growth or the proliferation of a single species of plant in the permanent pool of a wet pond. This may be caused by excess nutrients from fertilization practices (of a landscape company or surrounding neighbors), or by excess sediment. Steps should be taken to reduce excess nutrients at their source and to encourage the growth of native aquatic and semi-aquatic vegetation in and around the permanent pool.

### **Goals of this document**

- **Section 1: Identify Facility Characteristics and Maintenance Needs**
- **Section 2: Outline Routine Inspections**
- **Section 3 Define Maintenance Tasks, for each BMP**
- **Section 4 Establish a Record Keeping Procedure**

## **SECTION #1**

### **Identify Facility Characteristics and Maintenance Needs**

Understand how the facility works and its specific maintenance needs. The system owner and the Best Management Practices (BMPs) included in the system are outlined below:

#### **Stormwater Management System Operator: SAMA Productions LLC**

- The owner of the property will be responsible for the operation and maintenance of the system. The maintenance will be performed by a combination of hired staff and an appropriate contractor.
- Future property owners will be made aware of this Operation & Maintenance Plan by means of a condition in the Order of Conditions. The Condition attached to the deed will indicate that there is a long-term plan that must be followed in perpetuity.
- The following documentation describes the maintenance required for each stormwater best management practice (BMP). The BMPS are as follows:
  - Deep Sump Catch Basins
  - Rain Garden
  - Underground Infiltration Bed
- The project plans have been attached, which shows the locations of the BMPs that must be maintained.

Note that all waste should be disposed in accordance with applicable local, state, and federal guidelines and regulations.

## SECTION #2

### Perform Routine Inspections

The frequency of required inspections is dependant upon the **BMP** and is outlined below. Inspections and maintenance of the stormwater system is the responsibility of the property owner and/or operator. BMPs will be visually inspected in accordance with the following chart

BMPs	Inspection Frequency	What to Look For							Remarks
		Erosion	Tree growth	Vegetation	Slope integrity	Trash Debris	Sediment accumulatio	Outlet	
1. Bio Retention cells	Annually	✓	✓	✓	✓	✓	✓	✓	Inspect for debris, mulch annually.
	Monthly					✓			
2. Forebay	Quarterly	✓	✓	✓	✓	✓	✓	✓	Inspect for debris, and remove sediment as needed
	Monthly					✓			
3. Dry Detention Basin	Bi-Annually	✓	✓	✓	✓	✓	✓	✓	Inspect Inlet Manholes, Remove debris as needed. Use vacuum truck to clean isolator row as needed
	Every 5 years						✓		

## SECTION #3

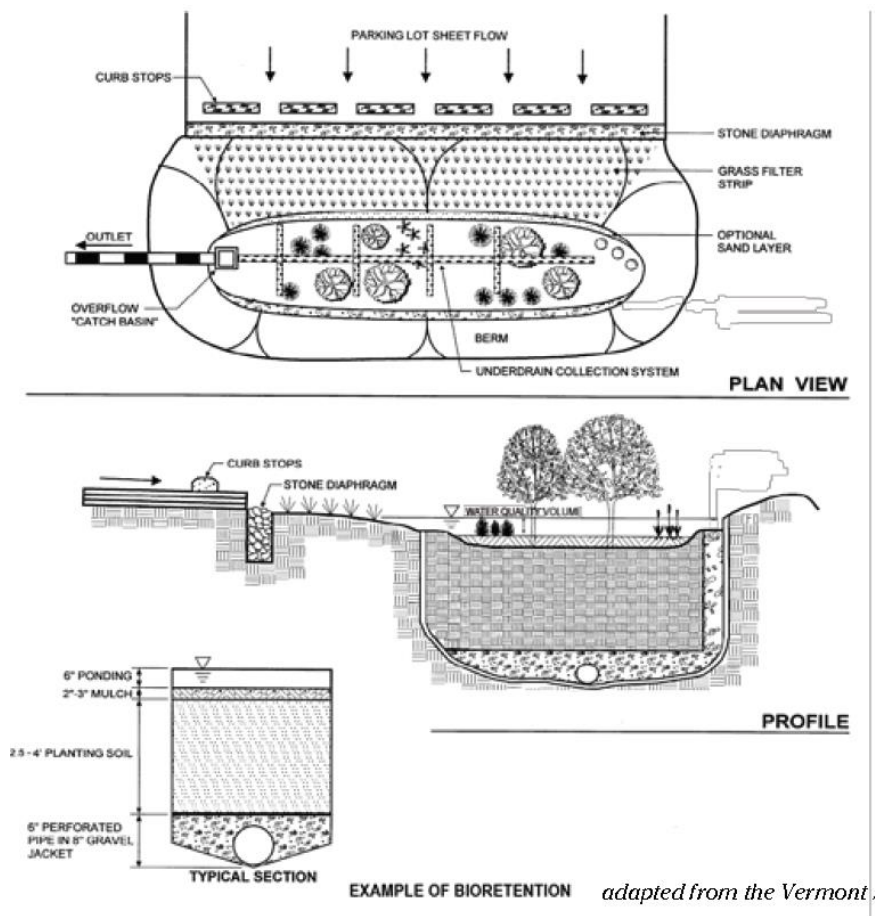
### Define Maintenance Tasks, for each BMP

Defining maintenance tasks and who will undertake these tasks – along with establishing a regular inspection program - is the core of a successful stormwater management facility maintenance program. Each BMP is outlined as follows:

- Section 3.1 – Rain Garden
- Section 3.2 – Sediment Forebays
- Section 3.3 – Dry Detention Basin

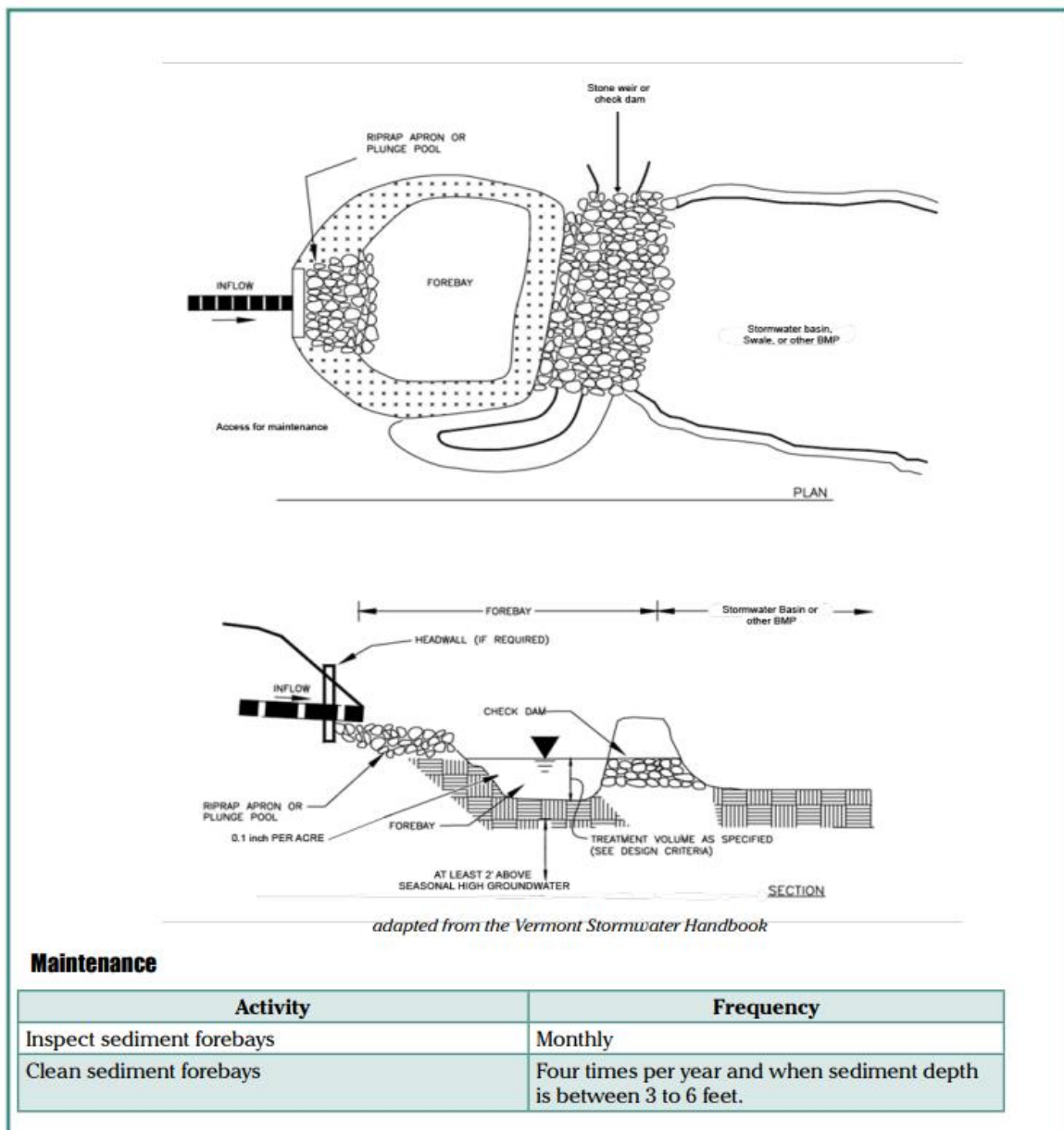
### Section 3.1- Rain Garden

Rain gardens remove pollutant through filtration, microbe activity and uptake by plants. Therefore, careful maintenance is required to ensure proper vegetation and soil media.



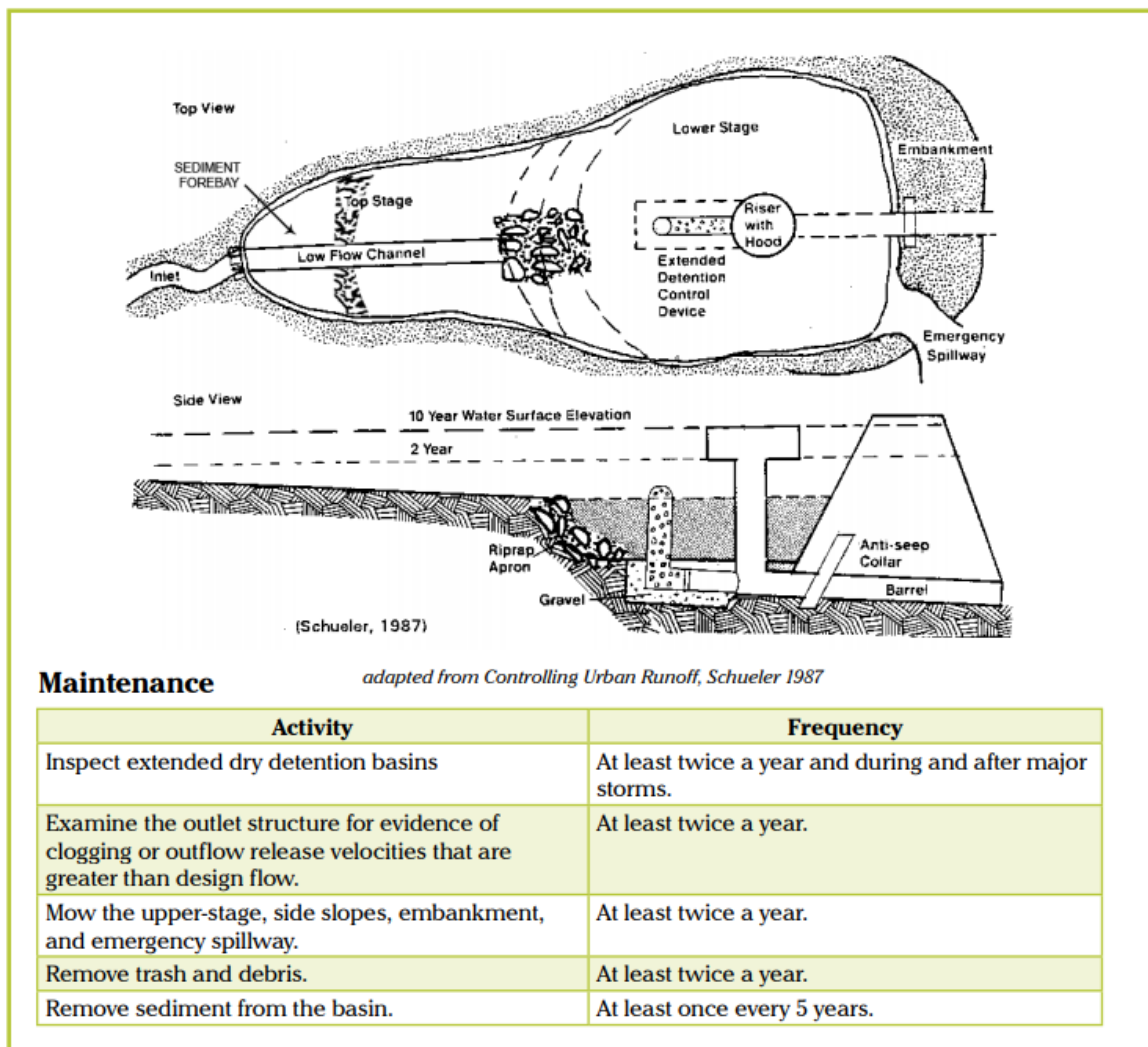
<b>Bioretention Maintenance Schedule</b>		
<i>Activity</i>	<i>Time of Year</i>	<i>Frequency</i>
Inspect & remove trash	Year round	Monthly
Mulch	Spring	Annually
Remove dead vegetation	Fall or Spring	Annually
Replace dead vegetation	Spring	Annually
Prune	Spring or Fall	Annually
Replace entire media & all vegetation	Late Spring/early Summer	As needed*

### Section 3.2 – Sediment Forebays



Sediments and associated pollutants are removed only when sediment forebays are actually cleaned out, so regular maintenance is essential. Frequently removing accumulated sediments will make it less likely that sediments will be resuspended. At a minimum, inspect sediment forebays monthly and clean them out at least four times per year. Stabilize the floor and sidewalls of the sediment forebay before making it operational, otherwise the practice will discharge excess amounts of suspended sediments. When mowing grasses, keep the grass height no greater than 6 inches. Set mower blades no lower than 3 to 4 inches. Check for signs of rilling and gulying and repair as needed. After removing the sediment, replace any vegetation damaged during the clean-out by either reseeding or resodding. When reseeding, incorporate practices such as hydroseeding with a tackifier, blanket, or similar practice to ensure that no scour occurs in the forebay, while the seeds germinate and develop roots.

### Section 3.3 – Dry detention basin



Inspect extended dry detention basins at least once per year to ensure that the basins are operating as intended. Inspect extended dry detention basins during and after major storms to

determine if the basin is meeting the expected detention times. Examine the outlet structure for evidence of clogging or outflow release velocities that are greater than design flow. Potential problems that should be checked include subsidence, erosion, cracking or tree growth on the embankment; damage to the emergency spillway; sediment accumulation around the outlet; inadequacy of the inlet/outlet channel erosion control measures; changes in the condition of the pilot channel; and erosion within the basin and banks. Make any necessary repairs immediately. During inspections, note any changes to the extended dry detention basin or the contributing watershed, because these could affect basin performance. Mow the upper-stage, side slopes, embankment, and emergency spillway at least twice per year. Also remove trash and debris at least twice per year. Remove sediment from the extended dry detention basin as necessary, but at least once every 5 years. Providing an on-site sediment disposal area will reduce the overall sediment removal costs.

## **SECTION #4**

### **Establish a Record Keeping Procedure**

Establishing a record keeping procedure will help to define chronic maintenance problems and aid in future budget preparation. A periodic examination of maintenance practices will assist in identifying persistent problems early.

Attached is a sample Stormwater Inspection Form.

#### **Budget for Maintenance:**

<b>BMP STRUCTURE</b>	<b>EST. AVG. ANNUAL MAINTENANCE COST</b>
Inspection Services	\$ 1500.00
Plowing Services	\$ 400.00
Pavement Sweeping	\$ 350.00
Infiltration Trenches	\$ 600.00
Roof Gutters	\$ 100.00
Catchbasins	\$ 300.00
Bio-Retention System	\$ 600.00
<b>TOTAL</b>	<b>\$ 3850.00</b>



## Stormwater Inspection Form

Project: \_\_\_\_\_

Inspector Name: \_\_\_\_\_ Weather: \_\_\_\_\_

Date: \_\_\_\_\_ Time: \_\_\_\_\_

### 1. Rain Garden:

Depth of sediment            Yes    No    Depth: \_\_\_\_\_

Conditions of Mulch:        Good            Poor

Conditions of Vegetation:    Good            Poor

Action Required: \_\_\_\_\_

\_\_\_\_\_

\*Attach additional pages as necessary.

### 2. Forebay:

Inspect Inlet and outlets

Depth of sediment: \_\_\_\_\_ inches

Structural Integrity:    Good            Poor

Inspect Outlet

Depth of sediment: \_\_\_\_\_ inches

Structural Integrity:    Good            Poor

Action Required: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\*Attach additional pages as necessary.

**3. Dry detention Basin**

Inspect Inlet and outlets

Depth of sediment: \_\_\_\_\_ inches

Structural Integrity:    Good            Poor

Inspect Outlet

Depth of sediment: \_\_\_\_\_ inches

Structural Integrity:    Good            Poor

Action Required: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

\*Attach additional pages as necessary.

### Statement of Compliance

Based on the above observations, this report can serve as confirmation that the stormwater system **is** being maintained and operated in general conformance with the approved plans and the discharge permit referenced above, and that the stormwater system is in good operating condition.

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Signature

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Date

---

Print or Type Name

#### **Record Keeping:**

A copy of this Inspection Form and any supporting documents, including but not limited to, photographs, vendor receipts, notes or other records, must be kept on file at the premises for a minimum of three (3) years.

# Extreme Precipitation Tables

## Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

<b>Smoothing</b>	Yes
<b>State</b>	Massachusetts
<b>Location</b>	
<b>Longitude</b>	73.137 degrees West
<b>Latitude</b>	42.145 degrees North
<b>Elevation</b>	0 feet
<b>Date/Time</b>	Fri, 29 Jan 2021 11:18:19 -0500

### Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
<b>1yr</b>	0.31	0.47	0.59	0.77	0.96	1.20	<b>1yr</b>	0.83	1.10	1.37	1.70	2.09	2.58	2.85	<b>1yr</b>	2.28	2.74	3.25	3.89	4.45	<b>1yr</b>
<b>2yr</b>	0.36	0.56	0.69	0.91	1.15	1.44	<b>2yr</b>	0.99	1.31	1.65	2.04	2.50	3.07	3.47	<b>2yr</b>	2.72	3.34	3.85	4.57	5.24	<b>2yr</b>
<b>5yr</b>	0.43	0.67	0.84	1.13	1.45	1.82	<b>5yr</b>	1.25	1.63	2.10	2.59	3.17	3.86	4.39	<b>5yr</b>	3.41	4.22	4.90	5.70	6.54	<b>5yr</b>
<b>10yr</b>	0.49	0.77	0.98	1.33	1.72	2.19	<b>10yr</b>	1.49	1.93	2.52	3.11	3.79	4.59	5.26	<b>10yr</b>	4.06	5.05	5.87	6.74	7.74	<b>10yr</b>
<b>25yr</b>	0.58	0.92	1.18	1.63	2.17	2.78	<b>25yr</b>	1.87	2.40	3.22	3.97	4.82	5.78	6.67	<b>25yr</b>	5.11	6.41	7.47	8.42	9.68	<b>25yr</b>
<b>50yr</b>	0.67	1.07	1.38	1.93	2.59	3.34	<b>50yr</b>	2.24	2.83	3.87	4.76	5.76	6.88	7.99	<b>50yr</b>	6.09	7.68	8.97	9.97	11.47	<b>50yr</b>
<b>100yr</b>	0.77	1.24	1.60	2.27	3.10	4.02	<b>100yr</b>	2.68	3.35	4.66	5.72	6.89	8.19	9.57	<b>100yr</b>	7.25	9.20	10.77	11.81	13.59	<b>100yr</b>
<b>200yr</b>	0.88	1.44	1.87	2.69	3.71	4.83	<b>200yr</b>	3.20	3.96	5.61	6.88	8.26	9.77	11.47	<b>200yr</b>	8.64	11.03	12.95	13.99	16.13	<b>200yr</b>
<b>500yr</b>	1.08	1.78	2.32	3.37	4.71	6.16	<b>500yr</b>	4.07	4.95	7.16	8.76	10.48	12.32	14.60	<b>500yr</b>	10.91	14.04	16.52	17.53	20.23	<b>500yr</b>

### Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
<b>1yr</b>	0.24	0.37	0.45	0.61	0.74	0.97	<b>1yr</b>	0.64	0.95	1.06	1.40	1.71	2.25	2.41	<b>1yr</b>	1.99	2.31	2.67	3.38	3.74	<b>1yr</b>
<b>2yr</b>	0.35	0.54	0.66	0.90	1.10	1.31	<b>2yr</b>	0.95	1.28	1.48	1.90	2.44	2.98	3.35	<b>2yr</b>	2.64	3.23	3.71	4.42	5.08	<b>2yr</b>
<b>5yr</b>	0.40	0.62	0.76	1.05	1.33	1.53	<b>5yr</b>	1.15	1.50	1.74	2.24	2.83	3.54	4.01	<b>5yr</b>	3.13	3.86	4.45	5.21	5.95	<b>5yr</b>
<b>10yr</b>	0.44	0.68	0.84	1.18	1.52	1.73	<b>10yr</b>	1.31	1.69	1.98	2.52	3.15	4.02	4.56	<b>10yr</b>	3.56	4.38	5.10	5.82	6.55	<b>10yr</b>
<b>25yr</b>	0.51	0.78	0.97	1.39	1.83	2.05	<b>25yr</b>	1.58	2.00	2.32	2.92	3.62	4.76	5.41	<b>25yr</b>	4.21	5.20	6.09	6.77	7.96	<b>25yr</b>
<b>50yr</b>	0.57	0.87	1.08	1.56	2.10	2.31	<b>50yr</b>	1.81	2.26	2.64	3.25	4.02	5.39	6.17	<b>50yr</b>	4.77	5.93	6.97	7.56	9.06	<b>50yr</b>
<b>100yr</b>	0.64	0.97	1.21	1.75	2.40	2.61	<b>100yr</b>	2.08	2.55	3.00	3.62	4.47	6.10	7.02	<b>100yr</b>	5.40	6.75	7.97	8.40	8.73	<b>100yr</b>
<b>200yr</b>	0.72	1.08	1.37	1.99	2.77	2.95	<b>200yr</b>	2.39	2.88	3.41	4.06	4.98	6.88	8.01	<b>200yr</b>	6.09	7.70	9.12	9.33	9.42	<b>200yr</b>
<b>500yr</b>	0.85	1.26	1.62	2.36	3.36	3.47	<b>500yr</b>	2.90	3.40	4.06	4.73	5.72	8.05	9.54	<b>500yr</b>	7.13	9.18	10.88	10.66	10.31	<b>500yr</b>

### Upper Confidence Limits

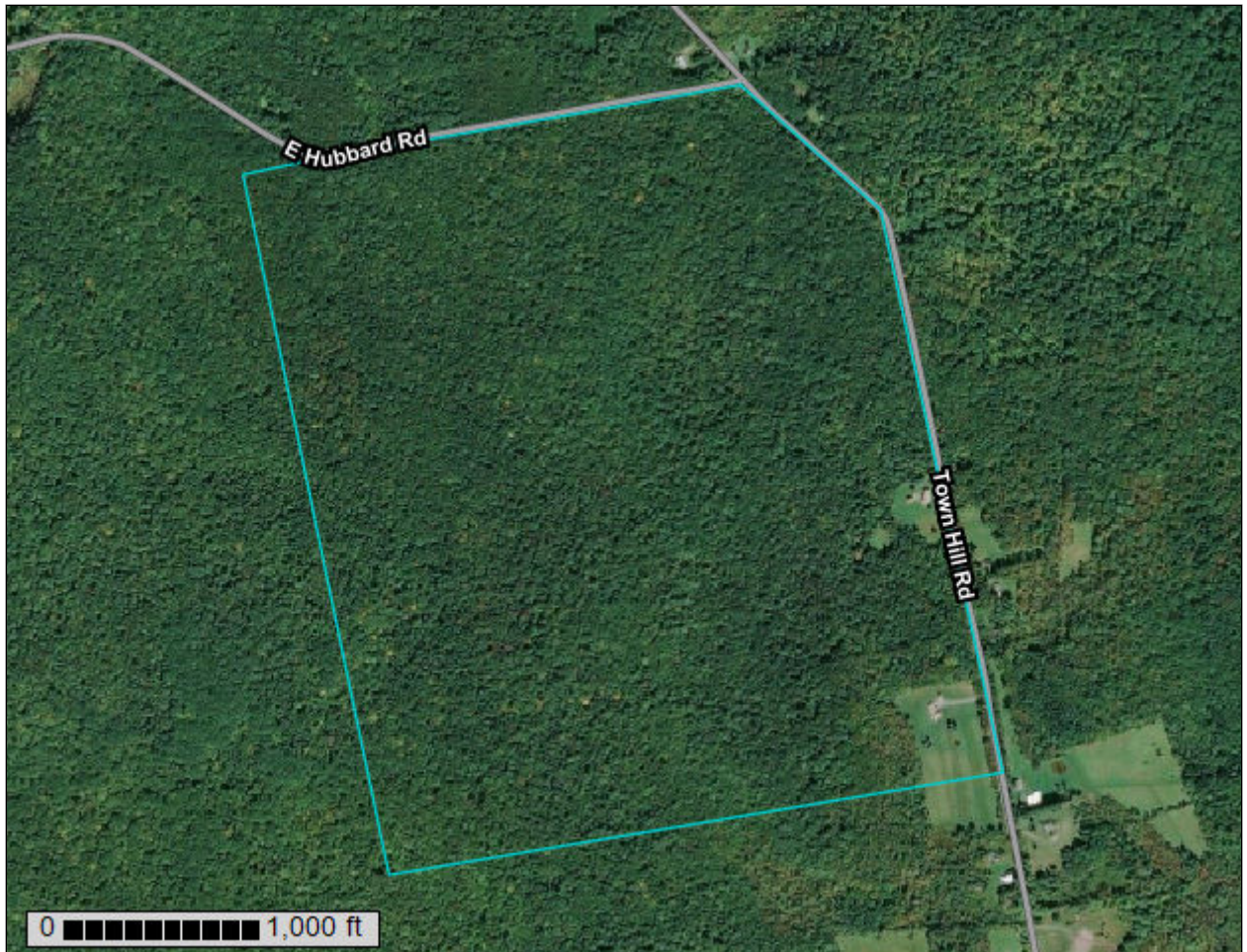
	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
<b>1yr</b>	0.35	0.53	0.65	0.88	1.08	1.27	<b>1yr</b>	0.93	1.24	1.44	1.79	2.34	2.80	3.10	<b>1yr</b>	2.47	2.98	3.49	4.17	4.83	<b>1yr</b>
<b>2yr</b>	0.38	0.59	0.72	0.98	1.21	1.40	<b>2yr</b>	1.04	1.37	1.59	2.08	2.62	3.19	3.63	<b>2yr</b>	2.82	3.49	4.02	4.77	5.54	<b>2yr</b>
<b>5yr</b>	0.47	0.72	0.90	1.23	1.57	1.82	<b>5yr</b>	1.36	1.78	2.04	2.65	3.35	4.20	4.78	<b>5yr</b>	3.72	4.60	5.39	6.24	7.09	<b>5yr</b>
<b>10yr</b>	0.56	0.86	1.07	1.50	1.93	2.21	<b>10yr</b>	1.67	2.16	2.49	3.22	4.04	5.20	5.91	<b>10yr</b>	4.60	5.68	6.75	7.69	8.67	<b>10yr</b>
<b>25yr</b>	0.72	1.09	1.36	1.94	2.55	2.89	<b>25yr</b>	2.20	2.82	3.22	4.19	5.21	6.90	7.84	<b>25yr</b>	6.11	7.54	9.10	10.17	11.47	<b>25yr</b>
<b>50yr</b>	0.86	1.31	1.63	2.34	3.15	3.53	<b>50yr</b>	2.72	3.45	3.93	5.10	6.31	8.55	9.72	<b>50yr</b>	7.57	9.35	11.40	12.56	14.03	<b>50yr</b>
<b>100yr</b>	1.04	1.57	1.97	2.84	3.89	4.32	<b>100yr</b>	3.36	4.22	4.79	6.23	7.64	10.61	12.05	<b>100yr</b>	9.39	11.58	14.30	15.57	17.02	<b>100yr</b>
<b>200yr</b>	1.25	1.88	2.38	3.45	4.81	5.30	<b>200yr</b>	4.15	5.18	5.83	7.60	9.26	13.17	14.94	<b>200yr</b>	11.66	14.36	17.96	19.31	20.93	<b>200yr</b>
<b>500yr</b>	1.61	2.40	3.08	4.48	6.37	6.94	<b>500yr</b>	5.50	6.78	7.59	9.89	12.01	17.59	19.87	<b>500yr</b>	15.56	19.11	24.32	25.69	27.63	<b>500yr</b>







# Custom Soil Resource Report for Berkshire County, Massachusetts



# Preface

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Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist ([http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2\\_053951](http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951)).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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# How Soil Surveys Are Made

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Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

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scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

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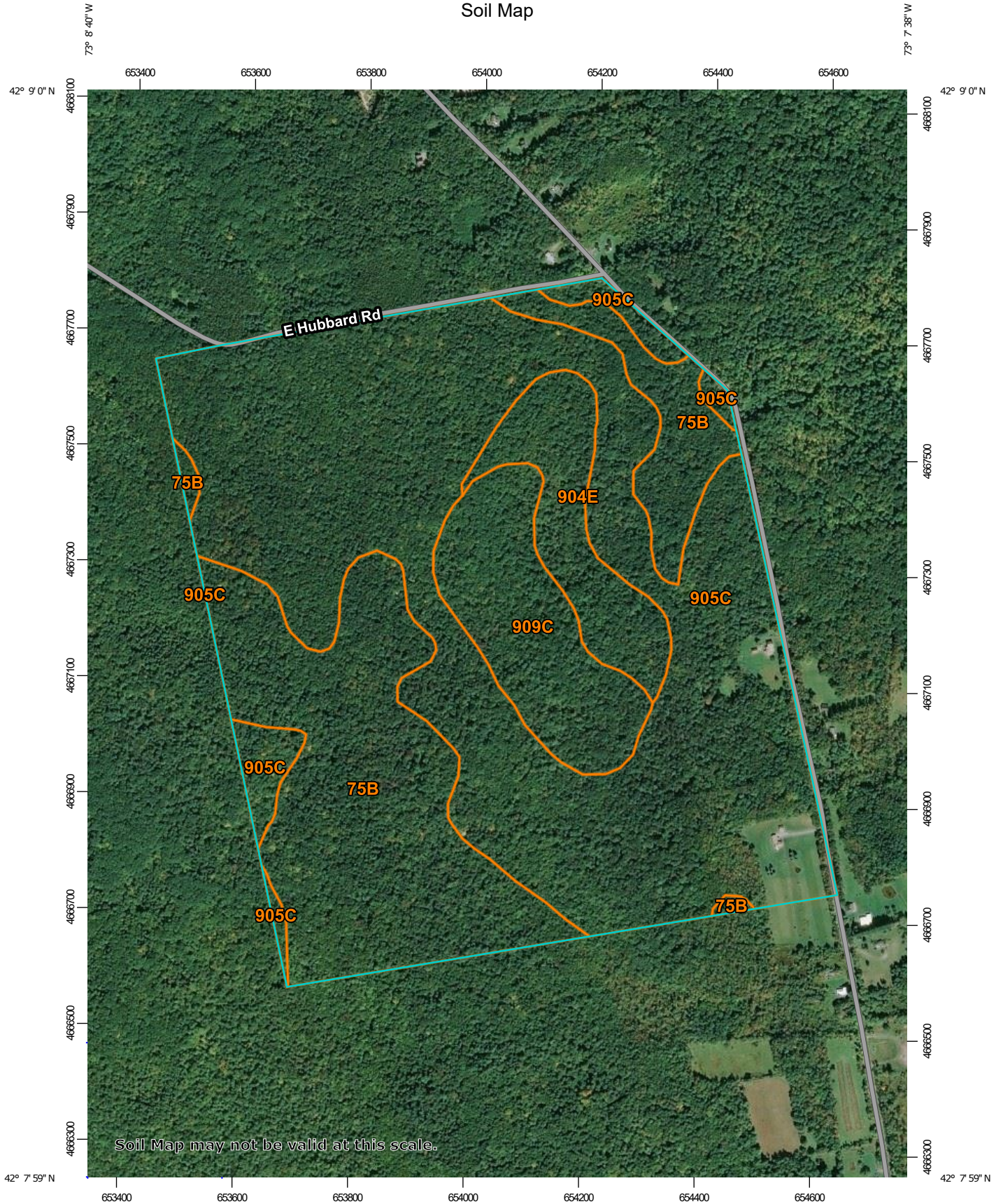
identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

# Soil Map

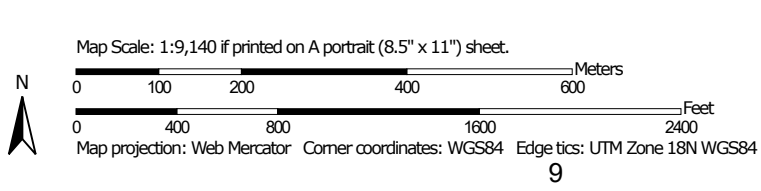
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The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

# Custom Soil Resource Report Soil Map



Soil Map may not be valid at this scale.



### MAP LEGEND

**Area of Interest (AOI)**

 Area of Interest (AOI)


**Soils**


 Soil Map Unit Polygons


 Soil Map Unit Lines


 Soil Map Unit Points

**Special Point Features**

 Blowout

 Borrow Pit


 Clay Spot


 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water


 Perennial Water

 Rock Outcrop


 Saline Spot

 Sandy Spot

 Severely Eroded Spot


 Sinkhole


 Slide or Slip


 Sodic Spot


 Spoil Area

 Stony Spot


 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

**Water Features**

 Streams and Canals


**Transportation**

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

**Background**

 Aerial Photography

### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
 Web Soil Survey URL:  
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Berkshire County, Massachusetts  
 Survey Area Data: Version 15, Jun 9, 2020

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 8, 2019—Sep 17, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
75B	Pillsbury fine sandy loam, 0 to 8 percent slopes, very stony	71.6	27.4%
904E	Lyman-Tunbridge association, 15 to 60 percent slopes, extremely stony	16.9	6.5%
905C	Peru-Marlow association, 3 to 15 percent slopes, extremely stony	148.4	56.9%
909C	Tunbridge-Lyman association, 3 to 15 percent slopes, extremely stony	23.9	9.2%
<b>Totals for Area of Interest</b>		<b>260.8</b>	<b>100.0%</b>

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it

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was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Berkshire County, Massachusetts

### 75B—Pillsbury fine sandy loam, 0 to 8 percent slopes, very stony

#### Map Unit Setting

*National map unit symbol:* 2ty6x  
*Elevation:* 360 to 2,070 feet  
*Mean annual precipitation:* 31 to 95 inches  
*Mean annual air temperature:* 27 to 52 degrees F  
*Frost-free period:* 90 to 140 days  
*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Pillsbury, very stony, and similar soils:* 79 percent  
*Minor components:* 21 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Pillsbury, Very Stony

##### Setting

*Landform:* Mountains, hills  
*Landform position (two-dimensional):* Toeslope, footslope  
*Landform position (three-dimensional):* Mountainbase, interfluve, base slope  
*Down-slope shape:* Concave  
*Across-slope shape:* Concave  
*Parent material:* Loamy lodgment till derived from gneiss and/or loamy lodgment till derived from mica schist and/or loamy lodgment till derived from granite

##### Typical profile

*Oe - 0 to 1 inches:* mucky peat  
*A - 1 to 6 inches:* fine sandy loam  
*Bg1 - 6 to 13 inches:* cobbly fine sandy loam  
*Bg2 - 13 to 23 inches:* cobbly fine sandy loam  
*Cd - 23 to 65 inches:* cobbly fine sandy loam

##### Properties and qualities

*Slope:* 0 to 8 percent  
*Surface area covered with cobbles, stones or boulders:* 1.1 percent  
*Depth to restrictive feature:* 21 to 43 inches to densic material  
*Drainage class:* Poorly drained  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately low to moderately high (0.01 to 1.42 in/hr)  
*Depth to water table:* About 0 to 12 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Maximum salinity:* Nonsaline (0.0 to 1.9 mmhos/cm)  
*Available water capacity:* Low (about 3.3 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 6s  
*Hydrologic Soil Group:* D  
*Hydric soil rating:* Yes

**Minor Components**

**Peru, very stony**

*Percent of map unit:* 9 percent  
*Landform:* Mountains, hills  
*Landform position (two-dimensional):* Backslope, footslope  
*Landform position (three-dimensional):* Mountainbase, base slope, interfluve  
*Microfeatures of landform position:* Rises, rises  
*Down-slope shape:* Convex  
*Across-slope shape:* Linear, convex  
*Hydric soil rating:* No

**Peacham, very stony**

*Percent of map unit:* 5 percent  
*Landform:* Mountains, hills  
*Landform position (two-dimensional):* Toeslope, footslope  
*Landform position (three-dimensional):* Mountainbase, interfluve, base slope  
*Microfeatures of landform position:* Closed depressions, closed depressions  
*Down-slope shape:* Concave  
*Across-slope shape:* Concave  
*Hydric soil rating:* Yes

**Wonsqueak**

*Percent of map unit:* 4 percent  
*Landform:* Mountains, hills  
*Landform position (two-dimensional):* Toeslope, footslope  
*Landform position (three-dimensional):* Mountainbase, interfluve, base slope  
*Microfeatures of landform position:* Closed depressions, closed depressions  
*Down-slope shape:* Concave  
*Across-slope shape:* Concave  
*Hydric soil rating:* Yes

**Lyman, very stony**

*Percent of map unit:* 3 percent  
*Landform:* Mountains, hills  
*Landform position (two-dimensional):* Backslope, shoulder, summit  
*Landform position (three-dimensional):* Mountainbase, interfluve, base slope  
*Microfeatures of landform position:* Rises, rises  
*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Hydric soil rating:* No

**904E—Lyman-Tunbridge association, 15 to 60 percent slopes, extremely stony**

**Map Unit Setting**

*National map unit symbol:* 2ty75  
*Elevation:* 850 to 2,360 feet  
*Mean annual precipitation:* 31 to 95 inches  
*Mean annual air temperature:* 27 to 52 degrees F

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*Frost-free period:* 60 to 160 days  
*Farmland classification:* Not prime farmland

### Map Unit Composition

*Lyman, extremely stony, and similar soils:* 45 percent  
*Tunbridge, extremely stony, and similar soils:* 40 percent  
*Minor components:* 15 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

### Description of Lyman, Extremely Stony

#### Setting

*Landform:* Mountains, hills  
*Landform position (two-dimensional):* Backslope, shoulder, summit  
*Landform position (three-dimensional):* Mountaintop, mountainflank, crest, side slope  
*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Parent material:* Loamy supraglacial till derived from granite and gneiss and/or loamy supraglacial till derived from phyllite and/or loamy supraglacial till derived from mica schist

#### Typical profile

*Oe - 0 to 1 inches:* moderately decomposed plant material  
*A - 1 to 3 inches:* loam  
*E - 3 to 5 inches:* fine sandy loam  
*Bhs - 5 to 7 inches:* loam  
*Bs1 - 7 to 11 inches:* loam  
*Bs2 - 11 to 18 inches:* channery loam  
*R - 18 to 28 inches:* bedrock

#### Properties and qualities

*Slope:* 15 to 60 percent  
*Surface area covered with cobbles, stones or boulders:* 6.0 percent  
*Depth to restrictive feature:* 11 to 24 inches to lithic bedrock  
*Drainage class:* Somewhat excessively drained  
*Capacity of the most limiting layer to transmit water (Ksat):* Very low to high (0.00 to 14.03 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water capacity:* Low (about 3.4 inches)

#### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 7s  
*Hydrologic Soil Group:* D  
*Hydric soil rating:* No

### Description of Tunbridge, Extremely Stony

#### Setting

*Landform:* Mountains, hills  
*Landform position (two-dimensional):* Backslope, shoulder, summit  
*Landform position (three-dimensional):* Mountaintop, mountainflank, crest, side slope  
*Down-slope shape:* Convex

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*Across-slope shape:* Convex

*Parent material:* Loamy supraglacial till derived from granite and gneiss and/or loamy supraglacial till derived from phyllite and/or loamy supraglacial till derived from mica schist

### Typical profile

*Oe - 0 to 3 inches:* moderately decomposed plant material

*Oa - 3 to 5 inches:* highly decomposed plant material

*E - 5 to 8 inches:* fine sandy loam

*Bhs - 8 to 11 inches:* fine sandy loam

*Bs - 11 to 26 inches:* fine sandy loam

*BC - 26 to 28 inches:* fine sandy loam

*R - 28 to 38 inches:* bedrock

### Properties and qualities

*Slope:* 15 to 60 percent

*Surface area covered with cobbles, stones or boulders:* 6.0 percent

*Depth to restrictive feature:* 20 to 40 inches to lithic bedrock

*Drainage class:* Well drained

*Capacity of the most limiting layer to transmit water (Ksat):* Very low to high (0.00 to 14.03 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water capacity:* Moderate (about 6.1 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 7s

*Hydrologic Soil Group:* C

*Hydric soil rating:* No

### Minor Components

#### Berkshire, extremely stony

*Percent of map unit:* 9 percent

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Backslope, summit, shoulder

*Landform position (three-dimensional):* Mountaintop, mountainflank, crest, side slope

*Down-slope shape:* Convex

*Across-slope shape:* Convex

*Hydric soil rating:* No

#### Wonsqueak

*Percent of map unit:* 2 percent

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Toeslope, footslope

*Landform position (three-dimensional):* Mountainbase, mountaintop, crest, side slope

*Microfeatures of landform position:* Open depressions, open depressions

*Down-slope shape:* Concave

*Across-slope shape:* Concave

*Hydric soil rating:* Yes

#### Peacham, extremely stony

*Percent of map unit:* 2 percent

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*Landform:* Mountains, hills

*Landform position (two-dimensional):* Toeslope, footslope

*Landform position (three-dimensional):* Mountaintop, mountainflank, crest, side slope

*Microfeatures of landform position:* Open depressions, open depressions

*Down-slope shape:* Concave

*Across-slope shape:* Concave

*Hydric soil rating:* Yes

### **Pillsbury, extremely stony**

*Percent of map unit:* 2 percent

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Footslope, toeslope

*Landform position (three-dimensional):* Mountaintop, mountainflank, crest, side slope

*Microfeatures of landform position:* Open depressions, open depressions

*Down-slope shape:* Concave

*Across-slope shape:* Concave

*Hydric soil rating:* Yes

## **905C—Peru-Marlow association, 3 to 15 percent slopes, extremely stony**

### **Map Unit Setting**

*National map unit symbol:* 2ty7p

*Elevation:* 790 to 2,100 feet

*Mean annual precipitation:* 31 to 95 inches

*Mean annual air temperature:* 27 to 52 degrees F

*Frost-free period:* 90 to 160 days

*Farmland classification:* Not prime farmland

### **Map Unit Composition**

*Peru, extremely stony, and similar soils:* 61 percent

*Marlow, extremely stony, and similar soils:* 20 percent

*Minor components:* 19 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

### **Description of Peru, Extremely Stony**

#### **Setting**

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Backslope, footslope

*Landform position (three-dimensional):* Mountainflank, mountainbase, interfluve, nose slope, side slope

*Down-slope shape:* Convex

*Across-slope shape:* Linear

*Parent material:* Loamy lodgment till derived from granite and/or loamy lodgment till derived from mica schist and/or loamy lodgment till derived from phyllite

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### Typical profile

*Oe - 0 to 1 inches:* moderately decomposed plant material  
*A - 1 to 5 inches:* fine sandy loam  
*E - 5 to 6 inches:* fine sandy loam  
*Bs1 - 6 to 7 inches:* fine sandy loam  
*Bs2 - 7 to 13 inches:* fine sandy loam  
*Bs3 - 13 to 18 inches:* fine sandy loam  
*BC - 18 to 21 inches:* fine sandy loam  
*Cd1 - 21 to 37 inches:* fine sandy loam  
*Cd2 - 37 to 65 inches:* fine sandy loam

### Properties and qualities

*Slope:* 3 to 15 percent  
*Surface area covered with cobbles, stones or boulders:* 6.0 percent  
*Depth to restrictive feature:* 21 to 43 inches to densic material  
*Drainage class:* Moderately well drained  
*Capacity of the most limiting layer to transmit water (Ksat):* Moderately low to moderately high (0.01 to 1.42 in/hr)  
*Depth to water table:* About 17 to 34 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Maximum salinity:* Nonsaline (0.0 to 1.9 mmhos/cm)  
*Available water capacity:* Low (about 3.6 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified  
*Land capability classification (nonirrigated):* 7s  
*Hydrologic Soil Group:* C/D  
*Hydric soil rating:* No

## Description of Marlow, Extremely Stony

### Setting

*Landform:* Mountains, hills  
*Landform position (two-dimensional):* Summit, shoulder, backslope  
*Landform position (three-dimensional):* Mountainbase, mountainflank, interfluve, nose slope, side slope  
*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Parent material:* Loamy lodgment till derived from granite and/or loamy lodgment till derived from mica schist and/or loamy lodgment till derived from phyllite

### Typical profile

*Oi - 0 to 2 inches:* slightly decomposed plant material  
*A - 2 to 5 inches:* fine sandy loam  
*E - 5 to 8 inches:* fine sandy loam  
*Bs1 - 8 to 15 inches:* fine sandy loam  
*Bs2 - 15 to 19 inches:* fine sandy loam  
*BC - 19 to 33 inches:* gravelly fine sandy loam  
*Cd - 33 to 65 inches:* fine sandy loam

### Properties and qualities

*Slope:* 3 to 15 percent  
*Surface area covered with cobbles, stones or boulders:* 6.0 percent  
*Depth to restrictive feature:* 20 to 41 inches to densic material  
*Drainage class:* Well drained



## Custom Soil Resource Report

*Capacity of the most limiting layer to transmit water (Ksat):* Moderately low to moderately high (0.01 to 1.42 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Maximum salinity:* Nonsaline (0.0 to 1.9 mmhos/cm)

*Available water capacity:* Low (about 5.1 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 7s

*Hydrologic Soil Group:* C

*Hydric soil rating:* No

### Minor Components

#### Lyman, extremely stony

*Percent of map unit:* 6 percent

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Shoulder, summit, backslope

*Landform position (three-dimensional):* Mountainflank, mountainbase, side slope, interfluve, nose slope

*Microfeatures of landform position:* Rises, rises

*Down-slope shape:* Convex

*Across-slope shape:* Convex

*Hydric soil rating:* No

#### Pillsbury, extremely stony

*Percent of map unit:* 5 percent

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Footslope, toeslope

*Landform position (three-dimensional):* Mountainflank, mountainbase, nose slope, side slope, interfluve

*Microfeatures of landform position:* Closed depressions, closed depressions, open depressions, open depressions

*Down-slope shape:* Concave

*Across-slope shape:* Concave

*Hydric soil rating:* Yes

#### Berkshire, extremely stony

*Percent of map unit:* 5 percent

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Backslope, summit, shoulder

*Landform position (three-dimensional):* Mountainflank, mountainbase, nose slope, side slope, interfluve

*Microfeatures of landform position:* Rises, rises

*Down-slope shape:* Convex

*Across-slope shape:* Convex

*Hydric soil rating:* No

#### Monadnock, extremely stony

*Percent of map unit:* 3 percent

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Backslope, summit, shoulder

*Landform position (three-dimensional):* Mountainbase, mountainflank, interfluve, nose slope, side slope

*Microfeatures of landform position:* Rises, rises

Custom Soil Resource Report

*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Hydric soil rating:* No

**909C—Tunbridge-Lyman association, 3 to 15 percent slopes, extremely stony**

**Map Unit Setting**

*National map unit symbol:* 2trrm  
*Elevation:* 1,080 to 2,390 feet  
*Mean annual precipitation:* 31 to 95 inches  
*Mean annual air temperature:* 27 to 52 degrees F  
*Frost-free period:* 60 to 160 days  
*Farmland classification:* Not prime farmland

**Map Unit Composition**

*Tunbridge, extremely stony, and similar soils:* 50 percent  
*Lyman, extremely stony, and similar soils:* 35 percent  
*Minor components:* 15 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

**Description of Tunbridge, Extremely Stony**

**Setting**

*Landform:* Mountains, hills  
*Landform position (two-dimensional):* Summit, shoulder, backslope  
*Landform position (three-dimensional):* Mountainbase, mountainflank, mountaintop, side slope, crest  
*Down-slope shape:* Convex  
*Across-slope shape:* Convex  
*Parent material:* Loamy supraglacial till derived from granite and gneiss and/or loamy supraglacial till derived from phyllite and/or loamy supraglacial till derived from mica schist

**Typical profile**

*Oe - 0 to 3 inches:* moderately decomposed plant material  
*Oa - 3 to 5 inches:* highly decomposed plant material  
*E - 5 to 8 inches:* fine sandy loam  
*Bhs - 8 to 11 inches:* fine sandy loam  
*Bs - 11 to 26 inches:* fine sandy loam  
*BC - 26 to 28 inches:* fine sandy loam  
*R - 28 to 38 inches:* bedrock

**Properties and qualities**

*Slope:* 3 to 15 percent  
*Surface area covered with cobbles, stones or boulders:* 7.1 percent  
*Depth to restrictive feature:* 20 to 40 inches to lithic bedrock  
*Drainage class:* Well drained

## Custom Soil Resource Report

*Capacity of the most limiting layer to transmit water (Ksat):* Very low to high (0.00 to 14.03 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water capacity:* Moderate (about 6.1 inches)

### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 7s

*Hydrologic Soil Group:* C

*Hydric soil rating:* No

### Description of Lyman, Extremely Stony

#### Setting

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Summit, shoulder, backslope

*Landform position (three-dimensional):* Mountainbase, mountainflank, mountaintop, side slope, crest

*Down-slope shape:* Convex

*Across-slope shape:* Convex

*Parent material:* Loamy supraglacial till derived from granite and gneiss and/or loamy supraglacial till derived from phyllite and/or loamy supraglacial till derived from mica schist

#### Typical profile

*Oe - 0 to 1 inches:* moderately decomposed plant material

*A - 1 to 3 inches:* loam

*E - 3 to 5 inches:* fine sandy loam

*Bhs - 5 to 7 inches:* loam

*Bs1 - 7 to 11 inches:* loam

*Bs2 - 11 to 18 inches:* channery loam

*R - 18 to 28 inches:* bedrock

#### Properties and qualities

*Slope:* 3 to 15 percent

*Surface area covered with cobbles, stones or boulders:* 6.0 percent

*Depth to restrictive feature:* 11 to 24 inches to lithic bedrock

*Drainage class:* Somewhat excessively drained

*Capacity of the most limiting layer to transmit water (Ksat):* Very low to high (0.00 to 14.03 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water capacity:* Low (about 3.4 inches)

#### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 7s

*Hydrologic Soil Group:* D

*Hydric soil rating:* No

### Minor Components

#### Berkshire, extremely stony

*Percent of map unit:* 8 percent

Custom Soil Resource Report

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Backslope, summit, shoulder

*Landform position (three-dimensional):* Mountainflank, mountainbase, side slope, interfluve, nose slope

*Down-slope shape:* Convex

*Across-slope shape:* Convex

*Hydric soil rating:* No

**Peacham, extremely stony**

*Percent of map unit:* 4 percent

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Toeslope, footslope

*Landform position (three-dimensional):* Mountainbase, base slope, interfluve

*Down-slope shape:* Concave

*Across-slope shape:* Concave

*Hydric soil rating:* Yes

**Pillsbury, extremely stony**

*Percent of map unit:* 3 percent

*Landform:* Mountains, hills

*Landform position (two-dimensional):* Footslope, toeslope

*Landform position (three-dimensional):* Mountainbase, mountainflank, side slope, nose slope, interfluve

*Down-slope shape:* Concave

*Across-slope shape:* Concave

*Hydric soil rating:* Yes

# References

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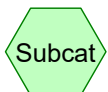
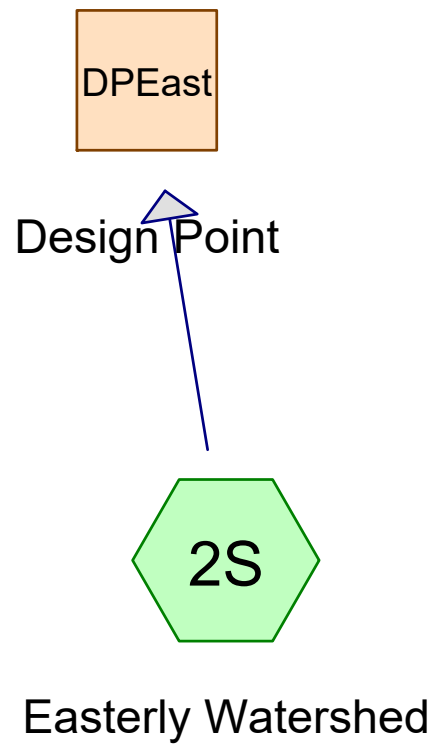
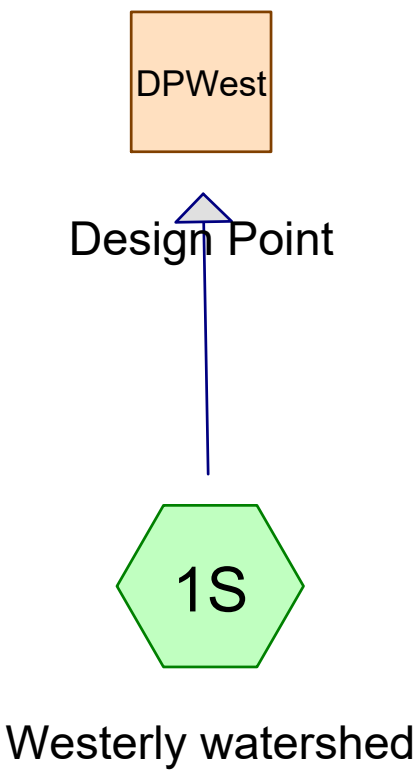
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## Custom Soil Resource Report

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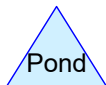
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Subcat



Reach



Pond



Link

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**Area Listing (all nodes)**

Area (acres)	CN	Description (subcatchment-numbers)
8.424	73	Woods, Fair, HSG C (2S)
13.939	73	Woods, Fair, HSG C (fair condition C/D soil) (1S)
<b>22.363</b>	<b>73</b>	<b>TOTAL AREA</b>



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**Soil Listing (all nodes)**

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
22.363	HSG C	1S, 2S
0.000	HSG D	
0.000	Other	
<b>22.363</b>		<b>TOTAL AREA</b>

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**Ground Covers (all nodes)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	22.363	0.000	0.000	22.363	Woods, Fair	1S, 2S
<b>0.000</b>	<b>0.000</b>	<b>22.363</b>	<b>0.000</b>	<b>0.000</b>	<b>22.363</b>	<b>TOTAL AREA</b>	

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment 1S: Westerly watershed** Runoff Area=607,173 sf 0.00% Impervious Runoff Depth>0.80"  
Flow Length=1,729' Tc=54.5 min CN=73 Runoff=5.64 cfs 0.928 af

**Subcatchment 2S: Easterly Watershed** Runoff Area=366,938 sf 0.00% Impervious Runoff Depth>0.81"  
Flow Length=1,160' Tc=26.0 min CN=73 Runoff=4.94 cfs 0.569 af

**Reach DPEast: Design Point** Inflow=4.94 cfs 0.569 af  
Outflow=4.94 cfs 0.569 af

**Reach DPWest: Design Point** Inflow=5.64 cfs 0.928 af  
Outflow=5.64 cfs 0.928 af

**Total Runoff Area = 22.363 ac Runoff Volume = 1.496 af Average Runoff Depth = 0.80"**  
**100.00% Pervious = 22.363 ac 0.00% Impervious = 0.000 ac**

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Type III 24-hr 2 Year Rainfall=3.07"

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**Summary for Subcatchment 1S: Westerly watershed**

Runoff = 5.64 cfs @ 12.81 hrs, Volume= 0.928 af, Depth> 0.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
Type III 24-hr 2 Year Rainfall=3.07"

Area (sf)	CN	Description
* 607,173	73	Woods, Fair, HSG C (fair condition C/D soil)
607,173		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.8	50	0.0100	0.03		<b>Sheet Flow, Sheet Flow</b> Woods: Dense underbrush n= 0.800 P2= 2.90"
1.9	138	0.0580	1.20		<b>Shallow Concentrated Flow, hillside segment 1</b> Woodland Kv= 5.0 fps
0.8	127	0.2519	2.51		<b>Shallow Concentrated Flow, Hillside segment 2</b> Woodland Kv= 5.0 fps
5.8	461	0.0694	1.32		<b>Shallow Concentrated Flow, Hillside Segment 3</b> Woodland Kv= 5.0 fps
4.9	258	0.0310	0.88		<b>Shallow Concentrated Flow, Hillside Segment 4</b> Woodland Kv= 5.0 fps
11.3	695	0.0417	1.02		<b>Shallow Concentrated Flow, Hillside Segment 5</b> Woodland Kv= 5.0 fps
54.5	1,729	Total			

**Existing conditions**

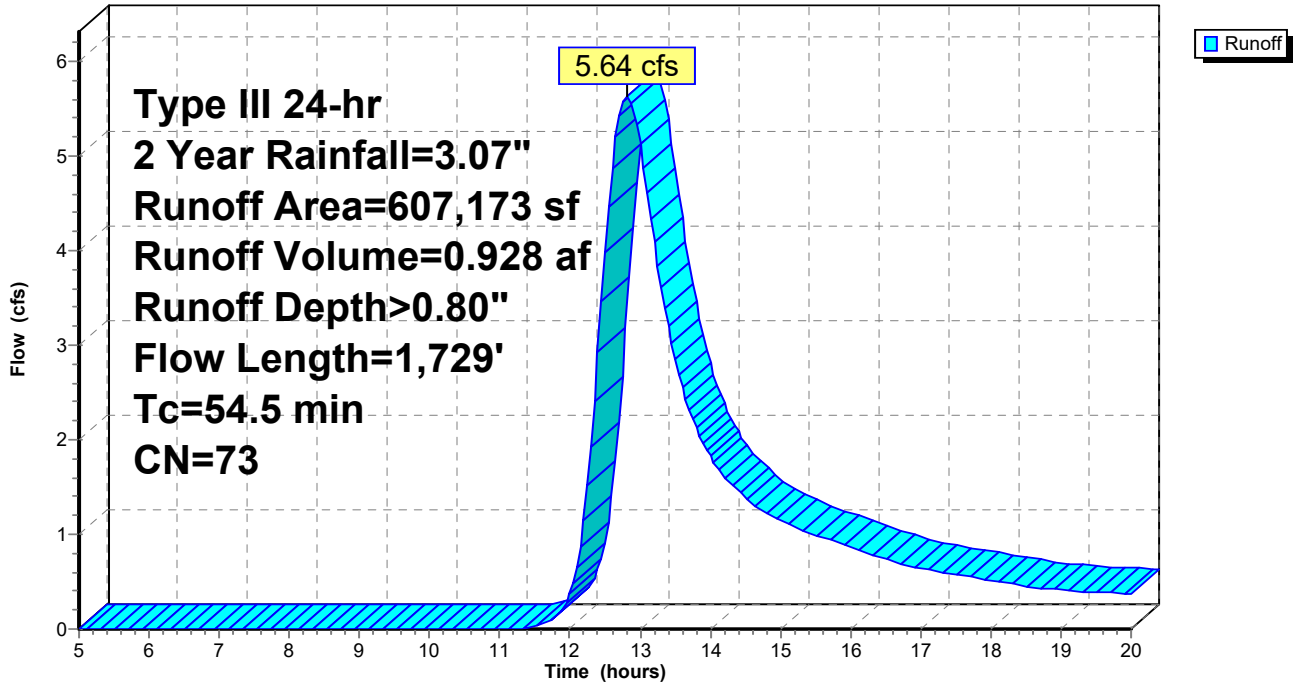
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**Subcatchment 1S: Westerly watershed**

Hydrograph



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**Summary for Subcatchment 2S: Easterly Watershed**

Runoff = 4.94 cfs @ 12.40 hrs, Volume= 0.569 af, Depth> 0.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 2 Year Rainfall=3.07"

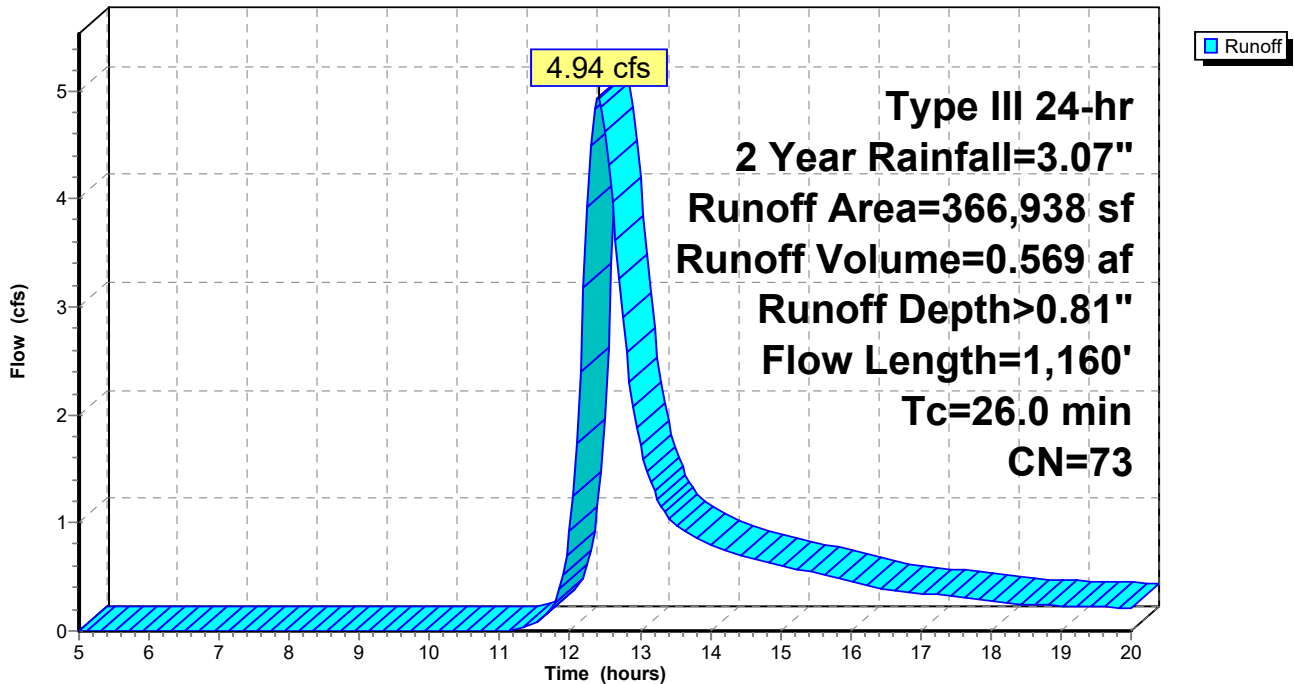
Area (sf)	CN	Description
366,938	73	Woods, Fair, HSG C
366,938		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0	50	0.0200	0.06		<b>Sheet Flow, Sheet Flow</b> Woods: Light underbrush n= 0.400 P2= 2.90"
2.7	277	0.1155	1.70		<b>Shallow Concentrated Flow, Hillside Segment 1</b> Woodland Kv= 5.0 fps
6.9	524	0.0648	1.27		<b>Shallow Concentrated Flow, Hillside Segemnt #2</b> Woodland Kv= 5.0 fps
3.4	309	0.0906	1.50		<b>Shallow Concentrated Flow, Hillside segment #3</b> Woodland Kv= 5.0 fps
26.0	1,160	Total			

**Subcatchment 2S: Easterly Watershed**

Hydrograph



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Type III 24-hr 2 Year Rainfall=3.07"

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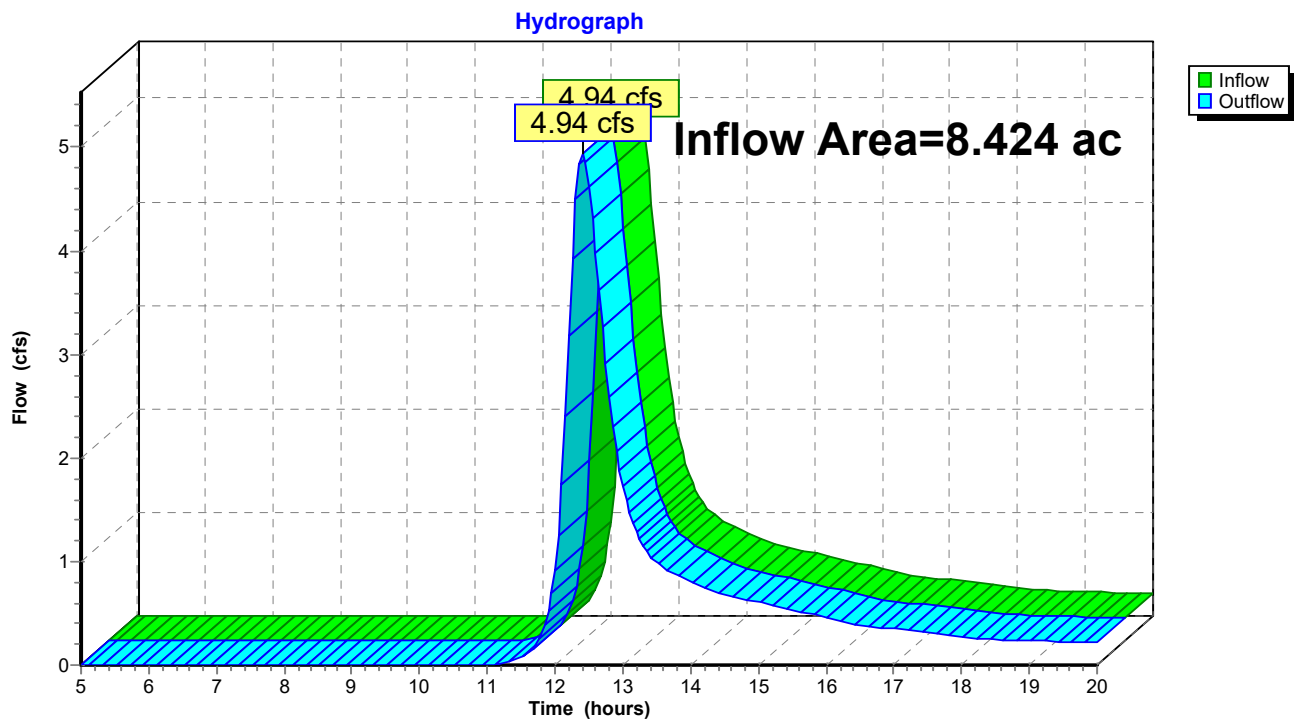
## Summary for Reach DPEast: Design Point

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8.424 ac, 0.00% Impervious, Inflow Depth > 0.81" for 2 Year event  
Inflow = 4.94 cfs @ 12.40 hrs, Volume= 0.569 af  
Outflow = 4.94 cfs @ 12.40 hrs, Volume= 0.569 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

## Reach DPEast: Design Point



### Existing conditions

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Type III 24-hr 2 Year Rainfall=3.07"

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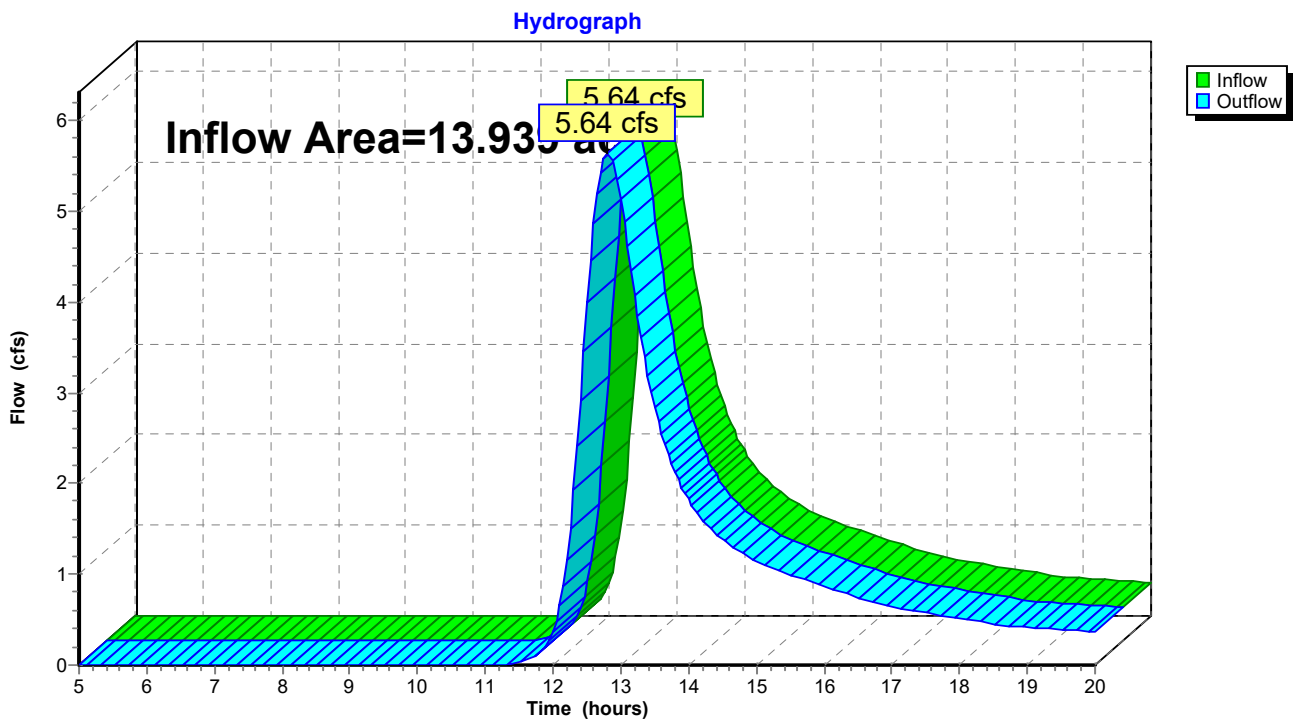
### Summary for Reach DPWest: Design Point

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 13.939 ac, 0.00% Impervious, Inflow Depth > 0.80" for 2 Year event  
Inflow = 5.64 cfs @ 12.81 hrs, Volume= 0.928 af  
Outflow = 5.64 cfs @ 12.81 hrs, Volume= 0.928 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

### Reach DPWest: Design Point





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Type III 24-hr 10 Year Rainfall=4.59"  
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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment 1S: Westerly watershed** Runoff Area=607,173 sf 0.00% Impervious Runoff Depth>1.78"  
Flow Length=1,729' Tc=54.5 min CN=73 Runoff=13.12 cfs 2.064 af

**Subcatchment 2S: Easterly Watershed** Runoff Area=366,938 sf 0.00% Impervious Runoff Depth>1.80"  
Flow Length=1,160' Tc=26.0 min CN=73 Runoff=11.48 cfs 1.262 af

**Reach DPEast: Design Point** Inflow=11.48 cfs 1.262 af  
Outflow=11.48 cfs 1.262 af

**Reach DPWest: Design Point** Inflow=13.12 cfs 2.064 af  
Outflow=13.12 cfs 2.064 af

**Total Runoff Area = 22.363 ac Runoff Volume = 3.326 af Average Runoff Depth = 1.78"**  
**100.00% Pervious = 22.363 ac 0.00% Impervious = 0.000 ac**

**Existing conditions**

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 Type III 24-hr 10 Year Rainfall=4.59"

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**Summary for Subcatchment 1S: Westerly watershed**

Runoff = 13.12 cfs @ 12.77 hrs, Volume= 2.064 af, Depth> 1.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 10 Year Rainfall=4.59"

Area (sf)	CN	Description
* 607,173	73	Woods, Fair, HSG C (fair condition C/D soil)
607,173		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.8	50	0.0100	0.03		<b>Sheet Flow, Sheet Flow</b> Woods: Dense underbrush n= 0.800 P2= 2.90"
1.9	138	0.0580	1.20		<b>Shallow Concentrated Flow, hillside segment 1</b> Woodland Kv= 5.0 fps
0.8	127	0.2519	2.51		<b>Shallow Concentrated Flow, Hillside segment 2</b> Woodland Kv= 5.0 fps
5.8	461	0.0694	1.32		<b>Shallow Concentrated Flow, Hillside Segment 3</b> Woodland Kv= 5.0 fps
4.9	258	0.0310	0.88		<b>Shallow Concentrated Flow, Hillside Segment 4</b> Woodland Kv= 5.0 fps
11.3	695	0.0417	1.02		<b>Shallow Concentrated Flow, Hillside Segment 5</b> Woodland Kv= 5.0 fps
54.5	1,729	Total			

**Existing conditions**

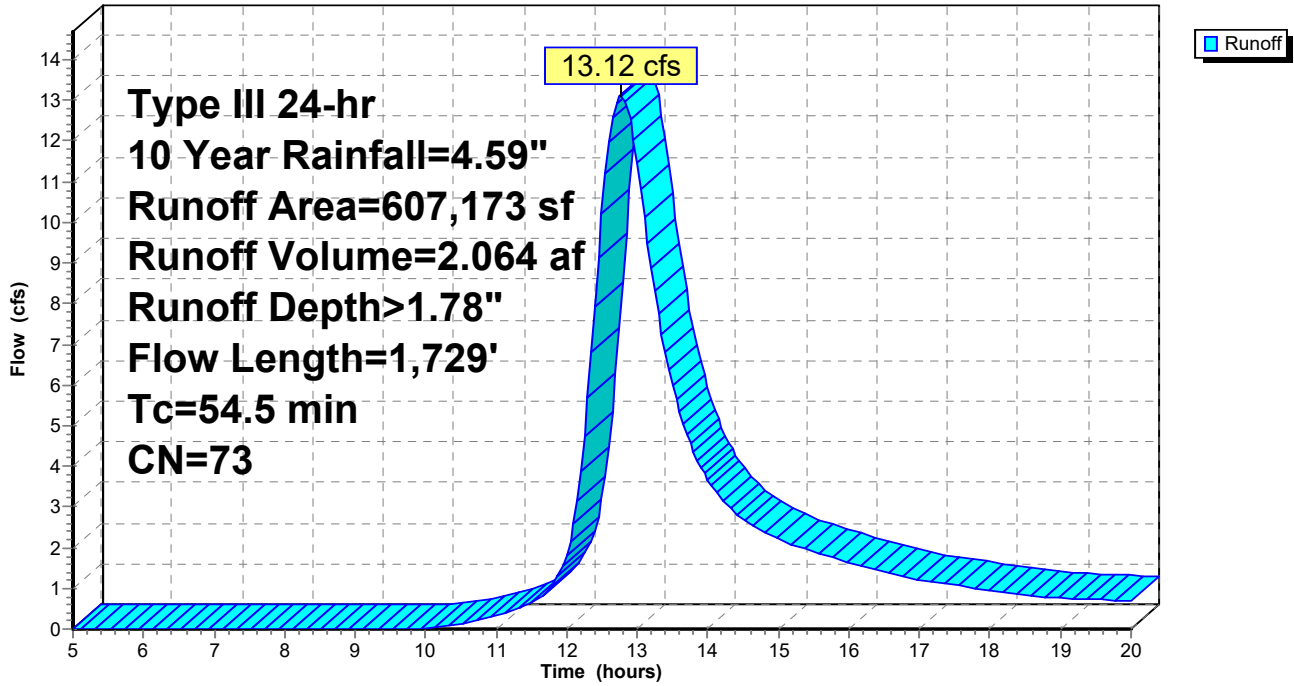
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Type III 24-hr 10 Year Rainfall=4.59"

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**Subcatchment 1S: Westerly watershed**

Hydrograph



**Existing conditions**

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**Summary for Subcatchment 2S: Easterly Watershed**

Runoff = 11.48 cfs @ 12.38 hrs, Volume= 1.262 af, Depth> 1.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 10 Year Rainfall=4.59"

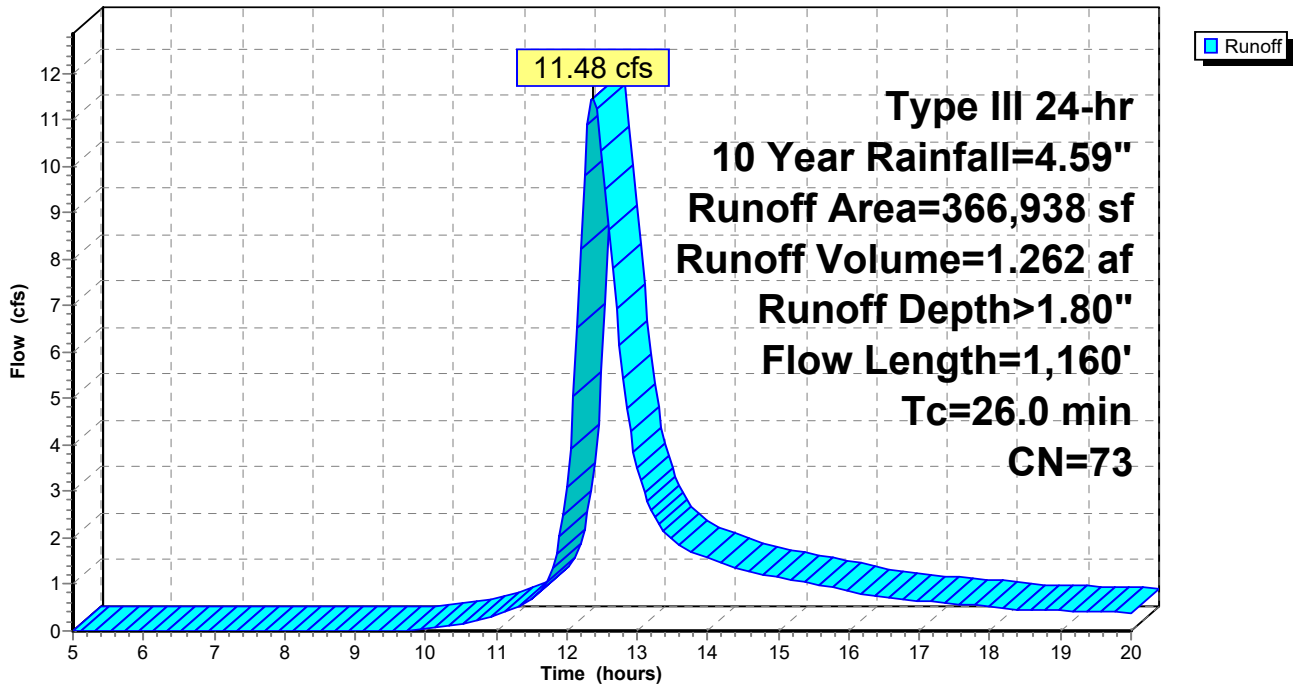
Area (sf)	CN	Description
366,938	73	Woods, Fair, HSG C
366,938		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0	50	0.0200	0.06		<b>Sheet Flow, Sheet Flow</b> Woods: Light underbrush n= 0.400 P2= 2.90"
2.7	277	0.1155	1.70		<b>Shallow Concentrated Flow, Hillside Segment 1</b> Woodland Kv= 5.0 fps
6.9	524	0.0648	1.27		<b>Shallow Concentrated Flow, Hillside Segemnt #2</b> Woodland Kv= 5.0 fps
3.4	309	0.0906	1.50		<b>Shallow Concentrated Flow, Hillside segment #3</b> Woodland Kv= 5.0 fps
26.0	1,160	Total			

**Subcatchment 2S: Easterly Watershed**

Hydrograph



## Existing conditions

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Type III 24-hr 10 Year Rainfall=4.59"

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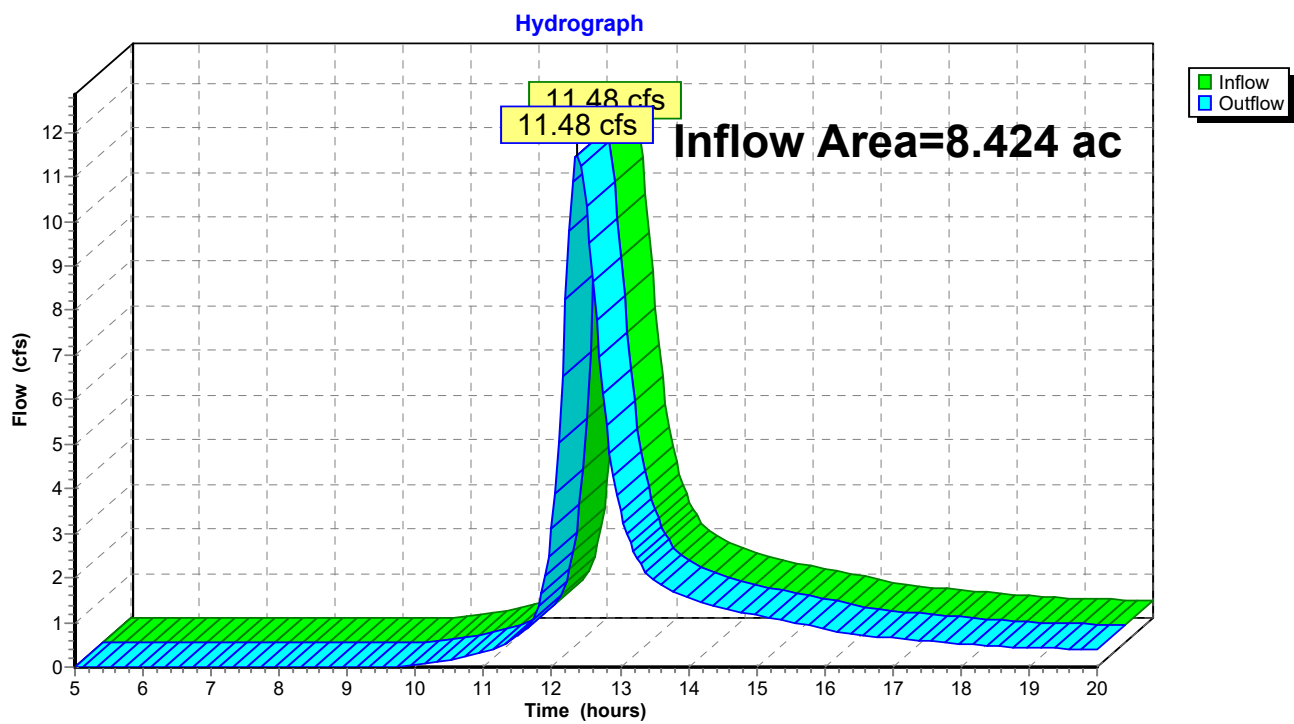
## Summary for Reach DPEast: Design Point

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8.424 ac, 0.00% Impervious, Inflow Depth > 1.80" for 10 Year event  
Inflow = 11.48 cfs @ 12.38 hrs, Volume= 1.262 af  
Outflow = 11.48 cfs @ 12.38 hrs, Volume= 1.262 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

## Reach DPEast: Design Point



## Existing conditions

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Type III 24-hr 10 Year Rainfall=4.59"

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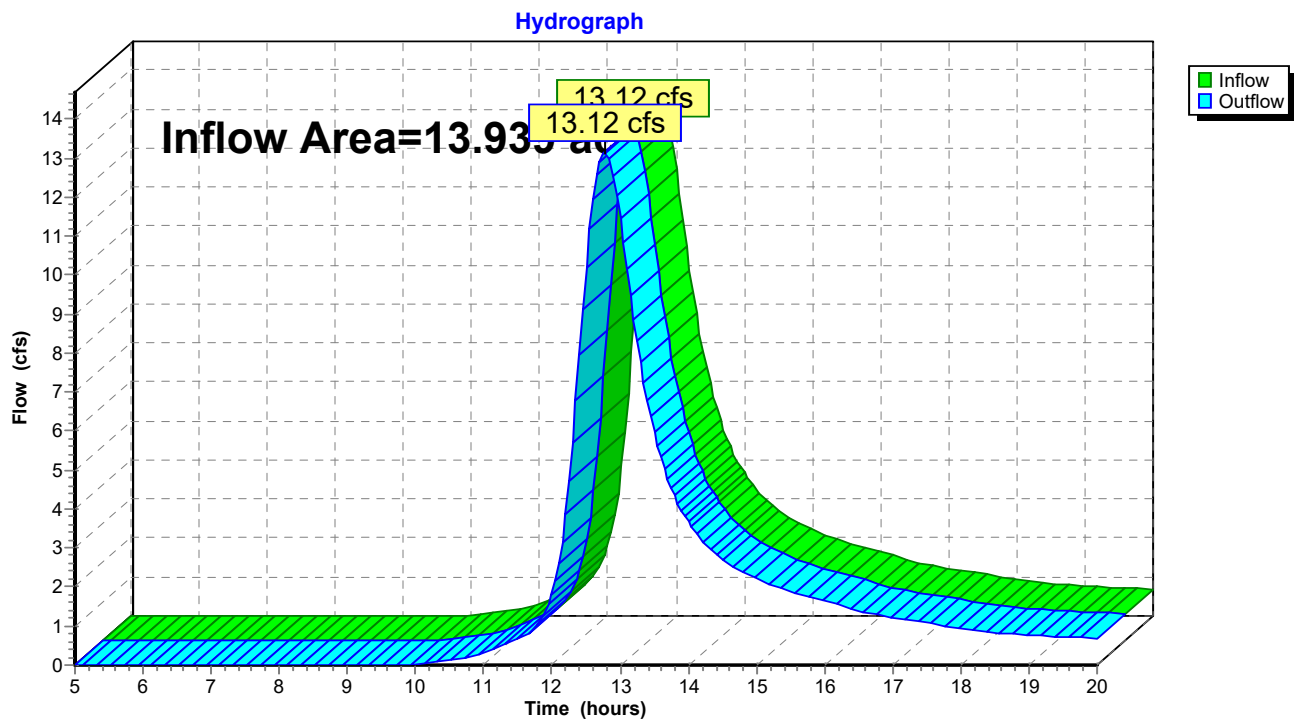
## Summary for Reach DPWest: Design Point

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 13.939 ac, 0.00% Impervious, Inflow Depth > 1.78" for 10 Year event  
Inflow = 13.12 cfs @ 12.77 hrs, Volume= 2.064 af  
Outflow = 13.12 cfs @ 12.77 hrs, Volume= 2.064 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

## Reach DPWest: Design Point



**Existing conditions**

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Type III 24-hr 100 Year Rainfall=8.19"  
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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment 1S: Westerly watershed** Runoff Area=607,173 sf 0.00% Impervious Runoff Depth>4.59"  
Flow Length=1,729' Tc=54.5 min CN=73 Runoff=33.83 cfs 5.326 af

**Subcatchment 2S: Easterly Watershed** Runoff Area=366,938 sf 0.00% Impervious Runoff Depth>4.63"  
Flow Length=1,160' Tc=26.0 min CN=73 Runoff=29.58 cfs 3.250 af

**Reach DPEast: Design Point** Inflow=29.58 cfs 3.250 af  
Outflow=29.58 cfs 3.250 af

**Reach DPWest: Design Point** Inflow=33.83 cfs 5.326 af  
Outflow=33.83 cfs 5.326 af

**Total Runoff Area = 22.363 ac Runoff Volume = 8.576 af Average Runoff Depth = 4.60"**  
**100.00% Pervious = 22.363 ac 0.00% Impervious = 0.000 ac**

**Existing conditions**

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**Summary for Subcatchment 1S: Westerly watershed**

Runoff = 33.83 cfs @ 12.74 hrs, Volume= 5.326 af, Depth> 4.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
Type III 24-hr 100 Year Rainfall=8.19"

Area (sf)	CN	Description
* 607,173	73	Woods, Fair, HSG C (fair condition C/D soil)
607,173		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.8	50	0.0100	0.03		<b>Sheet Flow, Sheet Flow</b> Woods: Dense underbrush n= 0.800 P2= 2.90"
1.9	138	0.0580	1.20		<b>Shallow Concentrated Flow, hillside segment 1</b> Woodland Kv= 5.0 fps
0.8	127	0.2519	2.51		<b>Shallow Concentrated Flow, Hillside segment 2</b> Woodland Kv= 5.0 fps
5.8	461	0.0694	1.32		<b>Shallow Concentrated Flow, Hillside Segment 3</b> Woodland Kv= 5.0 fps
4.9	258	0.0310	0.88		<b>Shallow Concentrated Flow, Hillside Segment 4</b> Woodland Kv= 5.0 fps
11.3	695	0.0417	1.02		<b>Shallow Concentrated Flow, Hillside Segment 5</b> Woodland Kv= 5.0 fps
54.5	1,729	Total			



**Existing conditions**

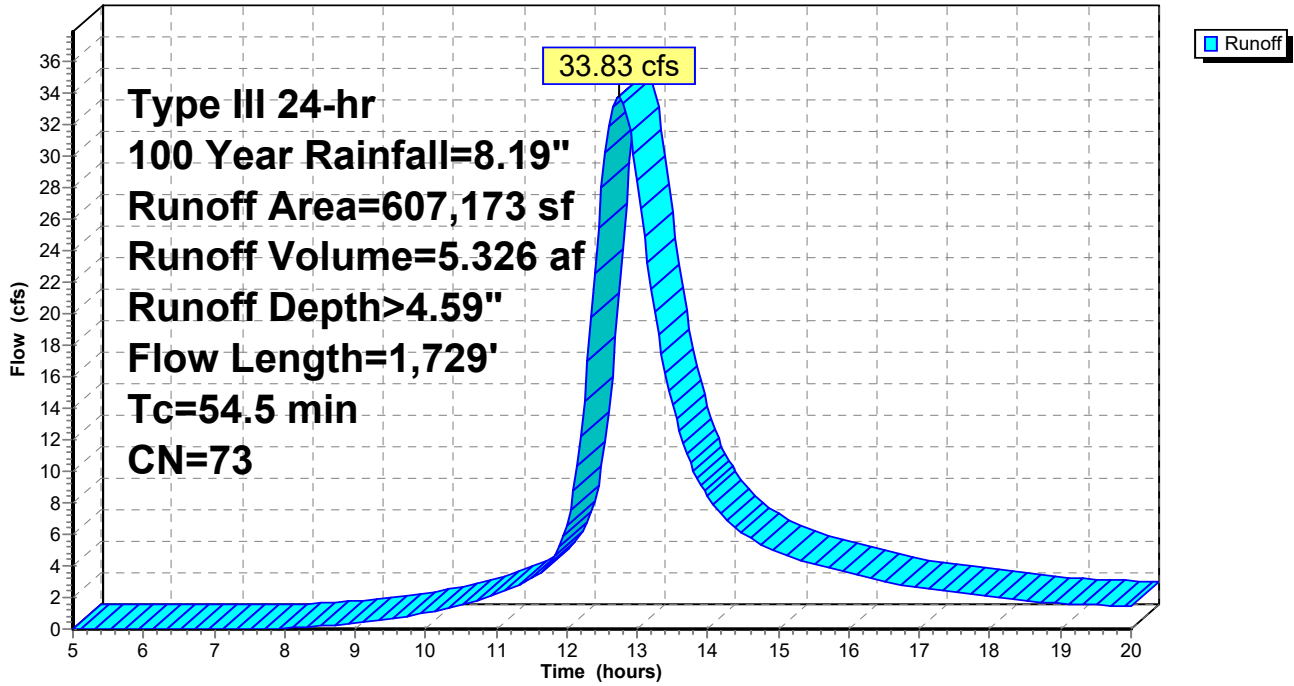
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SAMA Productions-Existing Conditions  
Type III 24-hr 100 Year Rainfall=8.19"

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**Subcatchment 1S: Westerly watershed**

Hydrograph



**Existing conditions**

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**Summary for Subcatchment 2S: Easterly Watershed**

Runoff = 29.58 cfs @ 12.36 hrs, Volume= 3.250 af, Depth> 4.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs  
 Type III 24-hr 100 Year Rainfall=8.19"

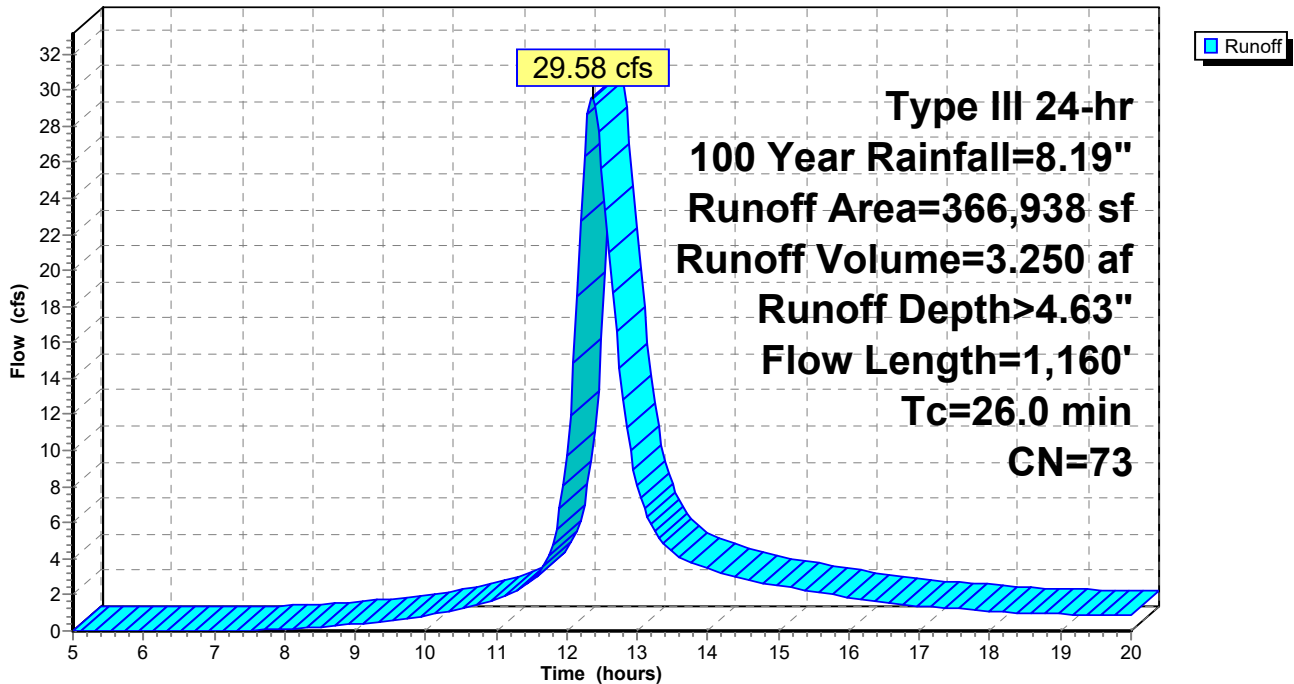
Area (sf)	CN	Description
366,938	73	Woods, Fair, HSG C
366,938		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0	50	0.0200	0.06		<b>Sheet Flow, Sheet Flow</b> Woods: Light underbrush n= 0.400 P2= 2.90"
2.7	277	0.1155	1.70		<b>Shallow Concentrated Flow, Hillside Segment 1</b> Woodland Kv= 5.0 fps
6.9	524	0.0648	1.27		<b>Shallow Concentrated Flow, Hillside Segemnt #2</b> Woodland Kv= 5.0 fps
3.4	309	0.0906	1.50		<b>Shallow Concentrated Flow, Hillside segment #3</b> Woodland Kv= 5.0 fps
26.0	1,160	Total			

**Subcatchment 2S: Easterly Watershed**

Hydrograph



# Existing conditions

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Type III 24-hr 100 Year Rainfall=8.19"

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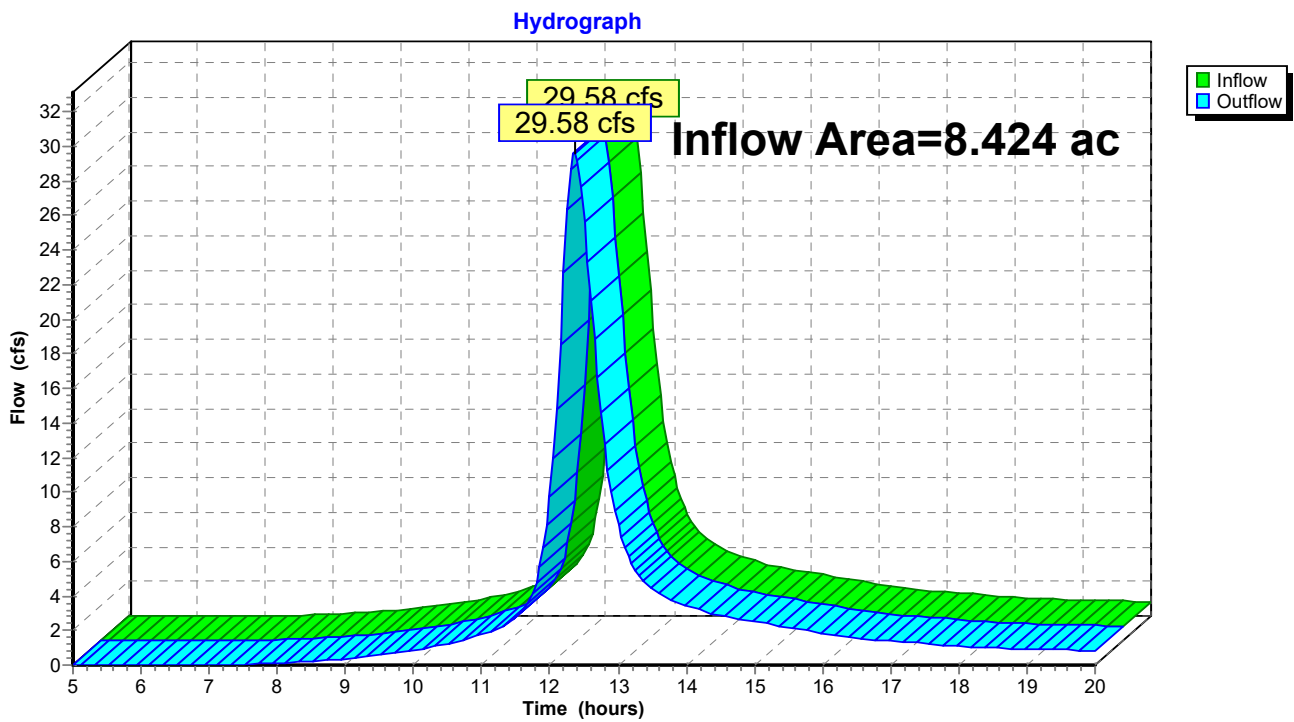
## Summary for Reach DPEast: Design Point

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 8.424 ac, 0.00% Impervious, Inflow Depth > 4.63" for 100 Year event  
Inflow = 29.58 cfs @ 12.36 hrs, Volume= 3.250 af  
Outflow = 29.58 cfs @ 12.36 hrs, Volume= 3.250 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

### Reach DPEast: Design Point



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Type III 24-hr 100 Year Rainfall=8.19"

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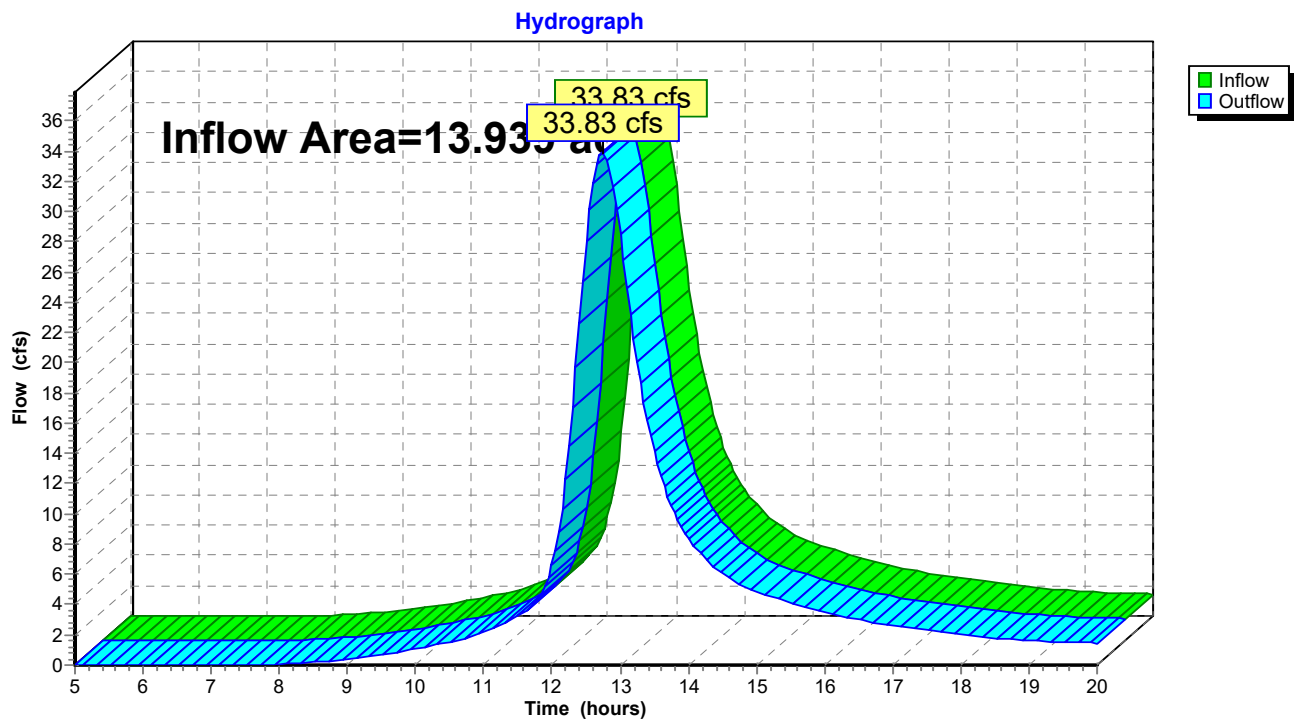
## Summary for Reach DPWest: Design Point

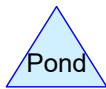
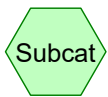
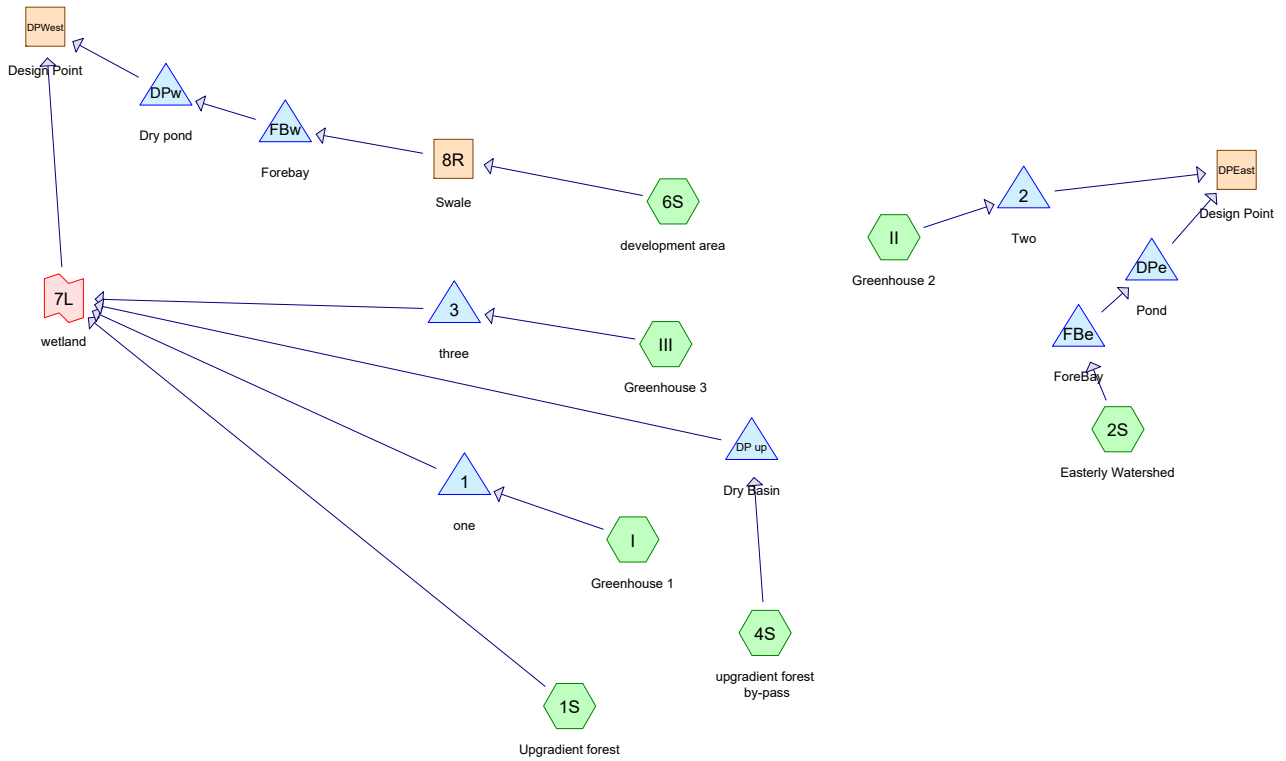
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 13.939 ac, 0.00% Impervious, Inflow Depth > 4.59" for 100 Year event  
Inflow = 33.83 cfs @ 12.74 hrs, Volume= 5.326 af  
Outflow = 33.83 cfs @ 12.74 hrs, Volume= 5.326 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

## Reach DPWest: Design Point





**Routing Diagram for Proposed Conditions**  
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**Area Listing (all nodes)**

Area (acres)	CN	Description (subcatchment-numbers)
0.781	74	>75% Grass cover, Good, HSG C (2S, I, II, III)
2.096	96	Gravel surface, HSG C (2S, 6S)
1.892	71	Meadow, non-grazed, HSG C (2S, 4S, 6S)
2.066	98	Roof (I, II, III)
0.115	98	Roofs, HSG C (2S)
1.848	73	Woods, Fair, HSG C (2S, 4S)
8.016	73	Woods, Fair, HSG C (fair condition C/D soil) (1S)
5.447	70	Woods, Good, HSG C (2S)
0.103	98	rooftop (6S)
<b>22.363</b>	<b>77</b>	<b>TOTAL AREA</b>

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**Soil Listing (all nodes)**

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
20.194	HSG C	1S, 2S, 4S, 6S, I, II, III
0.000	HSG D	
2.169	Other	6S, I, II, III
<b>22.363</b>		<b>TOTAL AREA</b>

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**Ground Covers (all nodes)**

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	0.781	0.000	0.000	0.781	>75% Grass cover, Good	2S, I, II, III
0.000	0.000	2.096	0.000	0.000	2.096	Gravel surface	2S, 6S
0.000	0.000	1.892	0.000	0.000	1.892	Meadow, non-grazed	2S, 4S, 6S
0.000	0.000	0.000	0.000	2.066	2.066	Roof	I, II, III
0.000	0.000	0.115	0.000	0.000	0.115	Roofs	2S
0.000	0.000	9.864	0.000	0.000	9.864	Woods, Fair	1S, 2S, 4S
0.000	0.000	5.447	0.000	0.000	5.447	Woods, Good	2S
0.000	0.000	0.000	0.000	0.103	0.103	rooftop	6S
<b>0.000</b>	<b>0.000</b>	<b>20.194</b>	<b>0.000</b>	<b>2.169</b>	<b>22.363</b>	<b>TOTAL AREA</b>	



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**Pipe Listing (all nodes)**

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	1	1,712.00	1,711.00	150.0	0.0067	0.011	6.0	0.0	0.0
2	2	1,712.00	1,711.00	150.0	0.0067	0.011	12.0	0.0	0.0
3	3	1,712.00	1,711.00	150.0	0.0067	0.011	6.0	0.0	0.0
4	DP up	1,731.25	1,716.00	300.0	0.0508	0.011	8.0	0.0	0.0
5	DPe	1,706.00	1,706.00	35.0	0.0000	0.011	15.0	0.0	0.0
6	DPw	1,702.50	1,699.00	40.0	0.0875	0.012	15.0	0.0	0.0
7	DPw	1,700.00	1,696.00	30.0	0.1333	0.011	4.0	0.0	0.0
8	FBe	1,708.00	1,706.00	24.0	0.0833	0.012	18.0	0.0	0.0
9	FBw	1,703.50	1,703.00	40.0	0.0125	0.012	18.0	0.0	0.0
10	FBw	1,702.25	1,702.00	35.0	0.0071	0.011	4.0	0.0	0.0

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Time span=1.00-40.00 hrs, dt=0.01 hrs, 3901 points  
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
 Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

<b>Subcatchment 1S: Upgradient forest</b>	Runoff Area=349,185 sf 0.00% Impervious Runoff Depth=0.90" Flow Length=1,729' Tc=54.5 min CN=73 Runoff=3.24 cfs 0.602 af
<b>Subcatchment 2S: Easterly Watershed</b>	Runoff Area=306,150 sf 1.63% Impervious Runoff Depth=0.95" Flow Length=1,201' Tc=23.3 min CN=74 Runoff=4.64 cfs 0.558 af
<b>Subcatchment 4S: upgradient forest by-pass</b>	Runoff Area=81,700 sf 0.00% Impervious Runoff Depth=0.90" Tc=0.0 min CN=73 Runoff=2.24 cfs 0.141 af
<b>Subcatchment 6S: development area</b>	Runoff Area=123,100 sf 3.66% Impervious Runoff Depth=1.50" Tc=0.0 min CN=83 Runoff=6.08 cfs 0.354 af
<b>Subcatchment I: Greenhouse 1</b>	Runoff Area=28,500 sf 78.95% Impervious Runoff Depth=2.32" Flow Length=25' Slope=0.2000 '/' Tc=0.2 min CN=93 Runoff=2.10 cfs 0.127 af
<b>Subcatchment II: Greenhouse 2</b>	Runoff Area=39,900 sf 78.95% Impervious Runoff Depth=2.32" Flow Length=25' Slope=0.2000 '/' Tc=0.2 min CN=93 Runoff=2.94 cfs 0.177 af
<b>Subcatchment III: Greenhouse 3</b>	Runoff Area=45,600 sf 78.95% Impervious Runoff Depth=2.32" Flow Length=25' Slope=0.2000 '/' Tc=0.2 min CN=93 Runoff=3.36 cfs 0.202 af
<b>Reach 8R: Swale</b>	Avg. Flow Depth=0.49' Max Vel=3.63 fps Inflow=6.08 cfs 0.354 af n=0.035 L=600.0' S=0.0300 '/' Capacity=93.83 cfs Outflow=5.25 cfs 0.354 af
<b>Reach DPEast: Design Point</b>	Inflow=4.87 cfs 0.733 af Outflow=4.87 cfs 0.733 af
<b>Reach DPWest: Design Point</b>	Inflow=5.55 cfs 1.261 af Outflow=5.55 cfs 1.261 af
<b>Pond 1: one</b>	Peak Elev=1,713.51' Storage=0.000 af Inflow=2.10 cfs 0.127 af Discarded=0.04 cfs 0.002 af Primary=2.05 cfs 0.124 af Outflow=2.09 cfs 0.127 af
<b>Pond 2: Two</b>	Peak Elev=1,713.51' Storage=0.000 af Inflow=2.94 cfs 0.177 af Discarded=0.04 cfs 0.003 af Primary=2.89 cfs 0.175 af Outflow=2.93 cfs 0.177 af
<b>Pond 3: three</b>	Peak Elev=1,713.51' Storage=0.000 af Inflow=3.36 cfs 0.202 af Discarded=0.06 cfs 0.004 af Primary=3.29 cfs 0.199 af Outflow=3.34 cfs 0.202 af
<b>Pond DP up: Dry Basin</b>	Peak Elev=1,731.40' Storage=0.105 af Inflow=2.24 cfs 0.141 af 8.0" Round Culvert n=0.011 L=300.0' S=0.0508 '/' Outflow=0.07 cfs 0.048 af
<b>Pond DPe: Pond</b>	Peak Elev=1,707.61' Storage=0.024 af Inflow=4.62 cfs 0.558 af Outflow=4.37 cfs 0.558 af
<b>Pond DPw: Dry pond</b>	Peak Elev=1,703.17' Storage=0.094 af Inflow=3.40 cfs 0.354 af Outflow=1.89 cfs 0.289 af

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**Pond FBe: ForeBay**

Peak Elev=1,708.70' Storage=0.008 af Inflow=4.64 cfs 0.558 af  
18.0" Round Culvert x 2.00 n=0.012 L=24.0' S=0.0833 '/ Outflow=4.62 cfs 0.558 af

**Pond FBw: Forebay**

Peak Elev=1,704.31' Storage=0.067 af Inflow=5.25 cfs 0.354 af  
Outflow=3.40 cfs 0.354 af

**Link 7L: wetland**

Inflow=5.55 cfs 0.972 af  
Primary=5.55 cfs 0.972 af

**Total Runoff Area = 22.363 ac Runoff Volume = 2.161 af Average Runoff Depth = 1.16"**  
**89.79% Pervious = 20.079 ac 10.21% Impervious = 2.284 ac**

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**Summary for Subcatchment 1S: Upgradient forest**

Runoff = 3.24 cfs @ 12.78 hrs, Volume= 0.602 af, Depth= 0.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Type III 24-hr 2 Year Rainfall=3.07"

Area (sf)	CN	Description
* 349,185	73	Woods, Fair, HSG C (fair condition C/D soil)
349,185		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.8	50	0.0100	0.03		<b>Sheet Flow, Sheet Flow</b> Woods: Dense underbrush n= 0.800 P2= 2.90"
1.9	138	0.0580	1.20		<b>Shallow Concentrated Flow, hillside segment 1</b> Woodland Kv= 5.0 fps
0.8	127	0.2519	2.51		<b>Shallow Concentrated Flow, Hillside segment 2</b> Woodland Kv= 5.0 fps
5.8	461	0.0694	1.32		<b>Shallow Concentrated Flow, Hillside Segment 3</b> Woodland Kv= 5.0 fps
4.9	258	0.0310	0.88		<b>Shallow Concentrated Flow, Hillside Segment 4</b> Woodland Kv= 5.0 fps
11.3	695	0.0417	1.02		<b>Shallow Concentrated Flow, Hillside Segment 5</b> Woodland Kv= 5.0 fps
54.5	1,729	Total			

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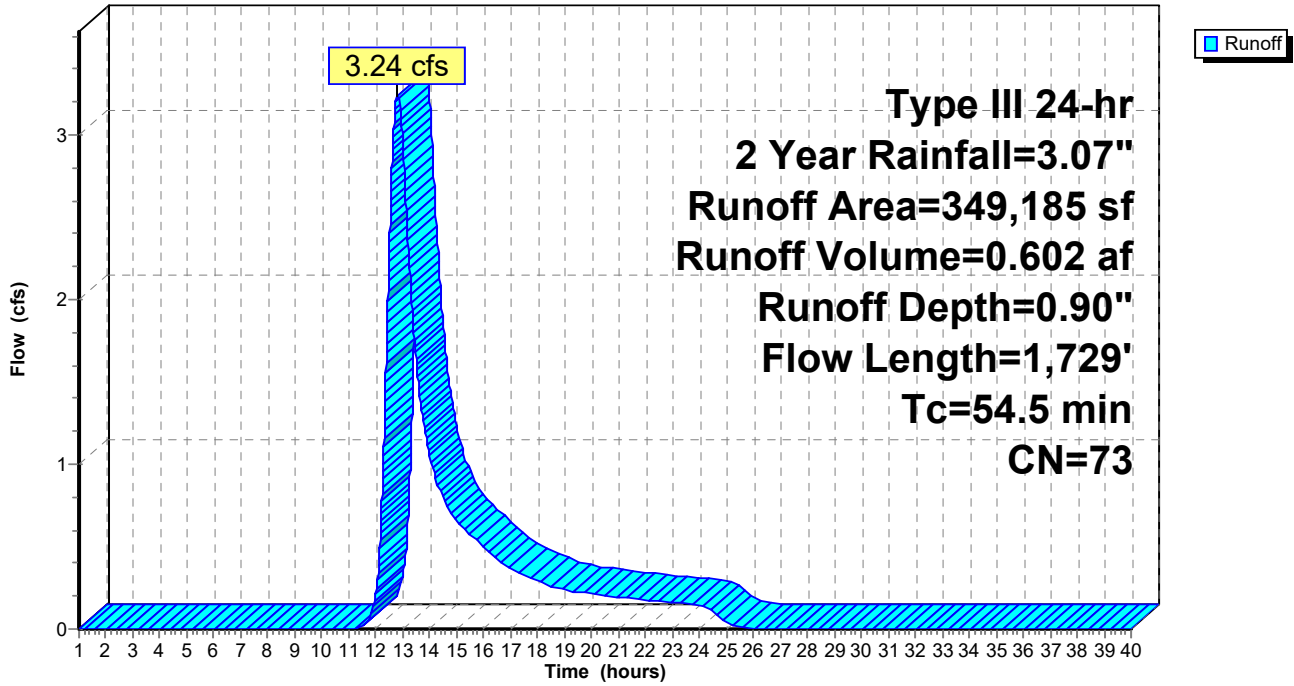
Type III 24-hr 2 Year Rainfall=3.07"

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**Subcatchment 1S: Upgradient forest**

Hydrograph



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**Summary for Subcatchment 2S: Easterly Watershed**

Runoff = 4.64 cfs @ 12.35 hrs, Volume= 0.558 af, Depth= 0.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Type III 24-hr 2 Year Rainfall=3.07"

Area (sf)	CN	Description
4,500	73	Woods, Fair, HSG C
5,000	98	Roofs, HSG C
36,500	96	Gravel surface, HSG C
10,000	74	>75% Grass cover, Good, HSG C
12,900	71	Meadow, non-grazed, HSG C
237,250	70	Woods, Good, HSG C
306,150	74	Weighted Average
301,150		98.37% Pervious Area
5,000		1.63% Impervious Area

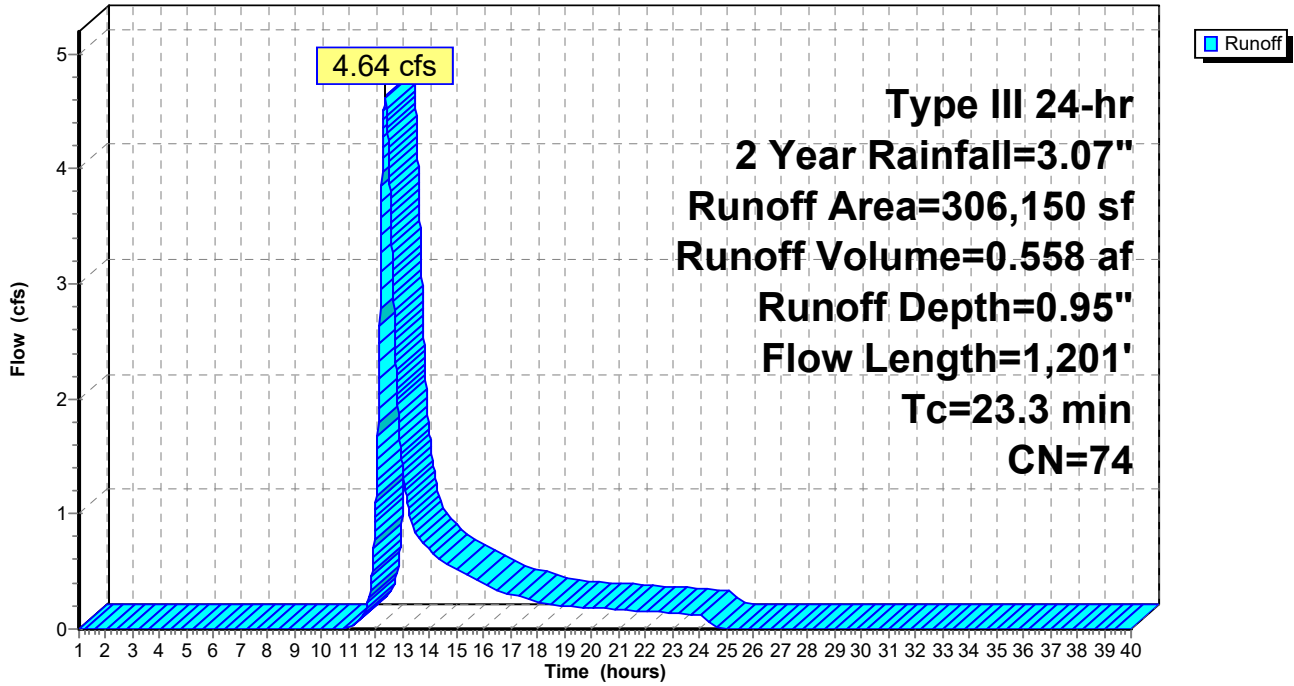
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0	50	0.0200	0.06		<b>Sheet Flow, Sheet Flow</b> Woods: Light underbrush n= 0.400 P2= 2.90"
2.7	277	0.1155	1.70		<b>Shallow Concentrated Flow, Hillside Segment 1</b> Woodland Kv= 5.0 fps
6.9	524	0.0648	1.27		<b>Shallow Concentrated Flow, Hillside Segment #2</b> Woodland Kv= 5.0 fps
0.7	350	0.0500	7.85	23.56	<b>Channel Flow,</b> Area= 3.0 sf Perim= 8.0' r= 0.38' n= 0.022 Earth, clean & straight
23.3	1,201	Total			

**Proposed Conditions**

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**Subcatchment 2S: Easterly Watershed**

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### Summary for Subcatchment 4S: upgradient forest by-pass

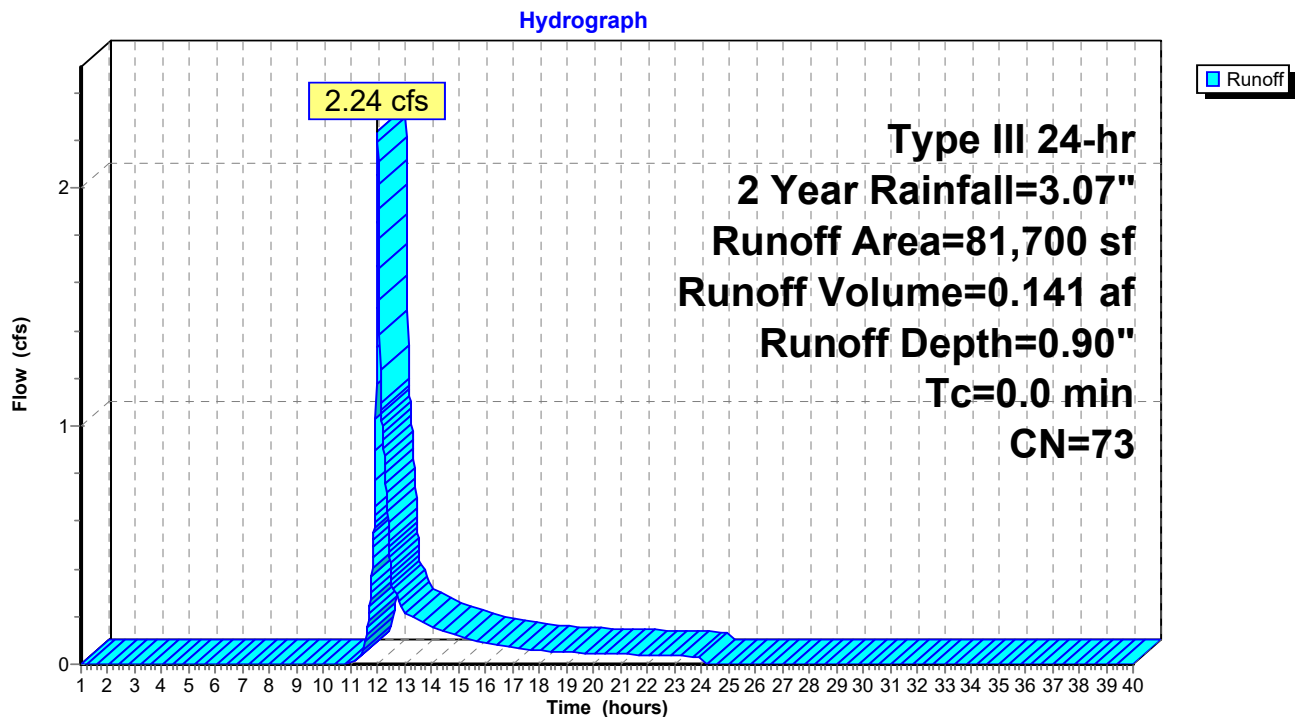
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 2.24 cfs @ 12.00 hrs, Volume= 0.141 af, Depth= 0.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Type III 24-hr 2 Year Rainfall=3.07"

Area (sf)	CN	Description
76,000	73	Woods, Fair, HSG C
5,700	71	Meadow, non-grazed, HSG C
81,700	73	Weighted Average
81,700		100.00% Pervious Area

### Subcatchment 4S: upgradient forest by-pass





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**Summary for Subcatchment 6S: development area**

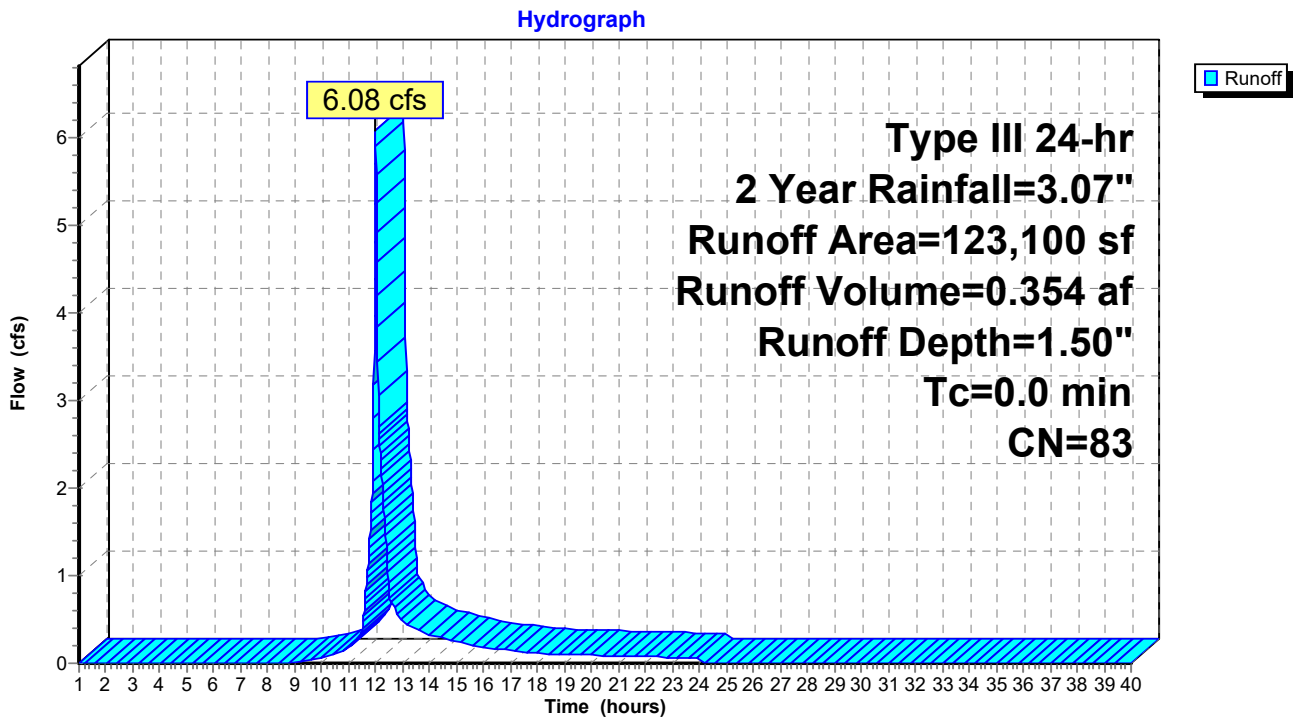
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 6.08 cfs @ 12.00 hrs, Volume= 0.354 af, Depth= 1.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 2 Year Rainfall=3.07"

Area (sf)	CN	Description
54,800	96	Gravel surface, HSG C
* 4,500	98	rooftop
63,800	71	Meadow, non-grazed, HSG C
123,100	83	Weighted Average
118,600		96.34% Pervious Area
4,500		3.66% Impervious Area

**Subcatchment 6S: development area**



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**Summary for Subcatchment I: Greenhouse 1**

[49] Hint:  $T_c < 2dt$  may require smaller dt

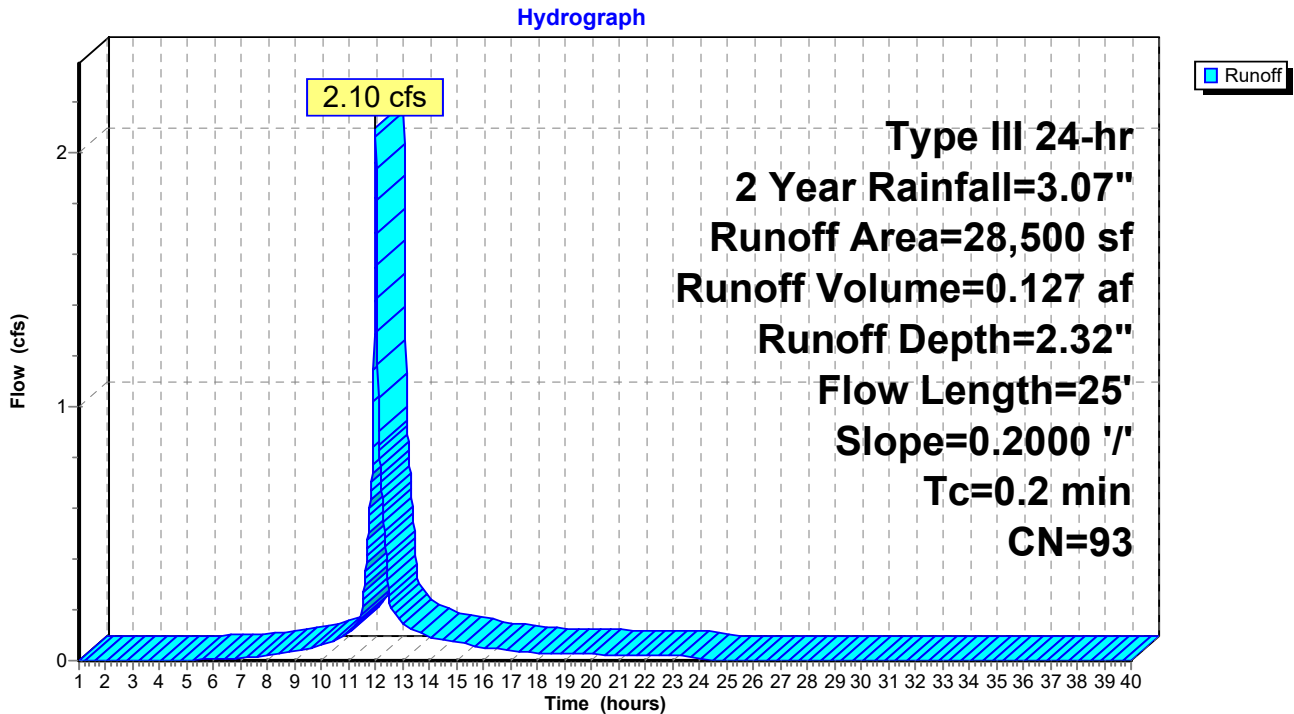
Runoff = 2.10 cfs @ 12.00 hrs, Volume= 0.127 af, Depth= 2.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 2 Year Rainfall=3.07"

	Area (sf)	CN	Description
*	22,500	98	Roof
	6,000	74	>75% Grass cover, Good, HSG C
	28,500	93	Weighted Average
	6,000		21.05% Pervious Area
	22,500		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	25	0.2000	2.49		Sheet Flow, Roof Smooth surfaces n= 0.011 P2= 2.90"

**Subcatchment I: Greenhouse 1**



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**Summary for Subcatchment II: Greenhouse 2**

[49] Hint:  $T_c < 2dt$  may require smaller dt

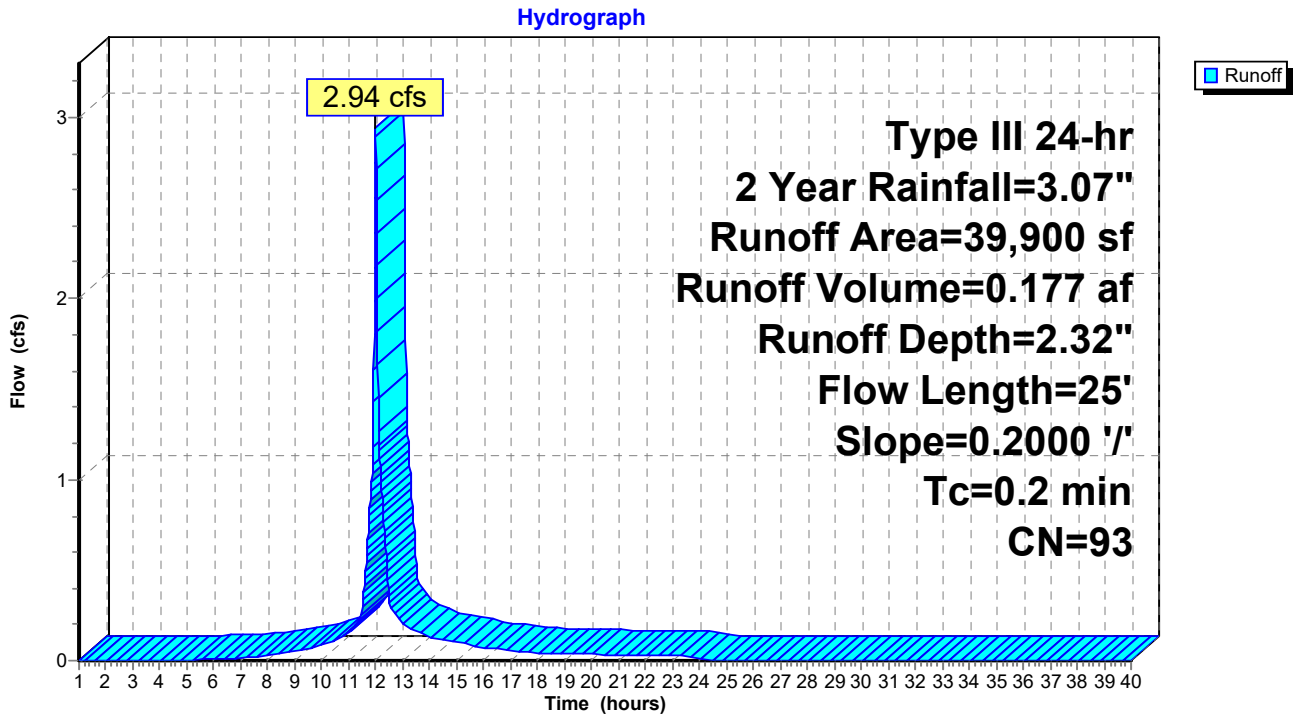
Runoff = 2.94 cfs @ 12.00 hrs, Volume= 0.177 af, Depth= 2.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 2 Year Rainfall=3.07"

	Area (sf)	CN	Description
*	31,500	98	Roof
	8,400	74	>75% Grass cover, Good, HSG C
	39,900	93	Weighted Average
	8,400		21.05% Pervious Area
	31,500		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	25	0.2000	2.49		Sheet Flow, Roof Smooth surfaces n= 0.011 P2= 2.90"

**Subcatchment II: Greenhouse 2**



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**Summary for Subcatchment III: Greenhouse 3**

[49] Hint:  $T_c < 2dt$  may require smaller dt

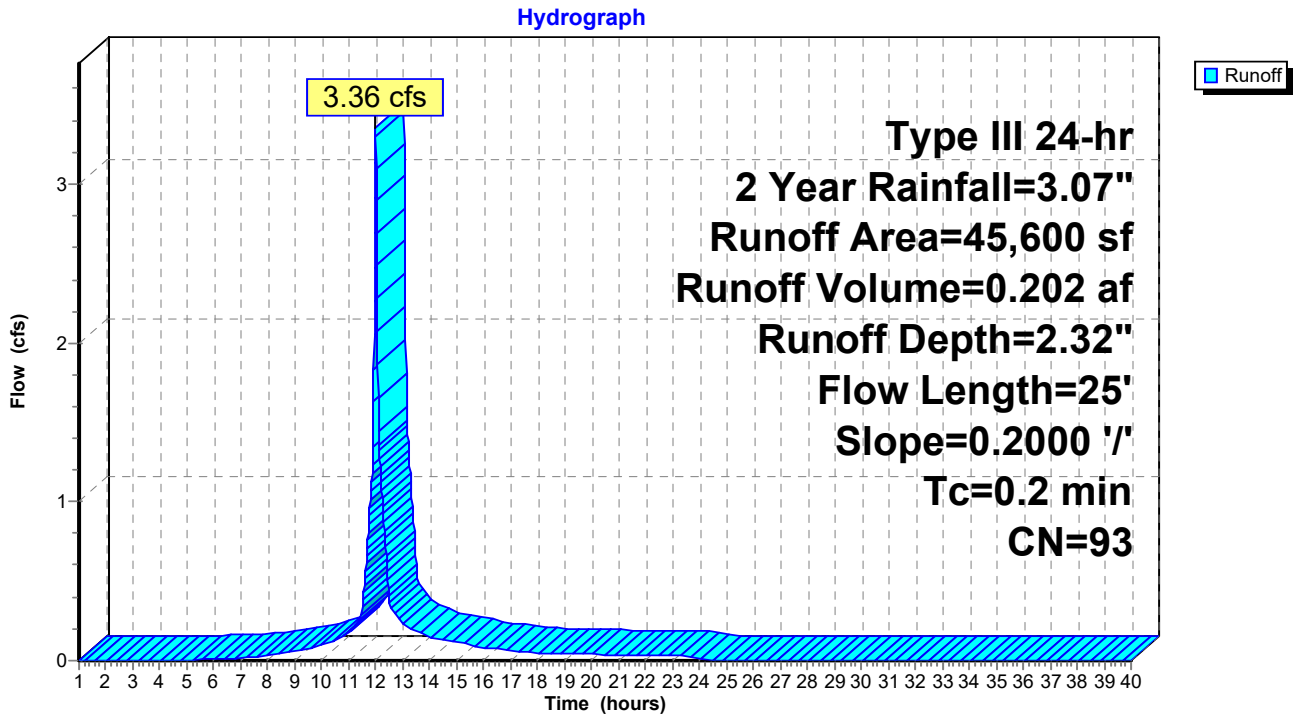
Runoff = 3.36 cfs @ 12.00 hrs, Volume= 0.202 af, Depth= 2.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 2 Year Rainfall=3.07"

	Area (sf)	CN	Description
*	36,000	98	Roof
	9,600	74	>75% Grass cover, Good, HSG C
	45,600	93	Weighted Average
	9,600		21.05% Pervious Area
	36,000		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	25	0.2000	2.49		Sheet Flow, Roof Smooth surfaces n= 0.011 P2= 2.90"

**Subcatchment III: Greenhouse 3**



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**Summary for Reach 8R: Swale**

Inflow Area = 2.826 ac, 3.66% Impervious, Inflow Depth = 1.50" for 2 Year event  
 Inflow = 6.08 cfs @ 12.00 hrs, Volume= 0.354 af  
 Outflow = 5.25 cfs @ 12.08 hrs, Volume= 0.354 af, Atten= 14%, Lag= 4.5 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Max. Velocity= 3.63 fps, Min. Travel Time= 2.8 min  
 Avg. Velocity = 1.09 fps, Avg. Travel Time= 9.1 min

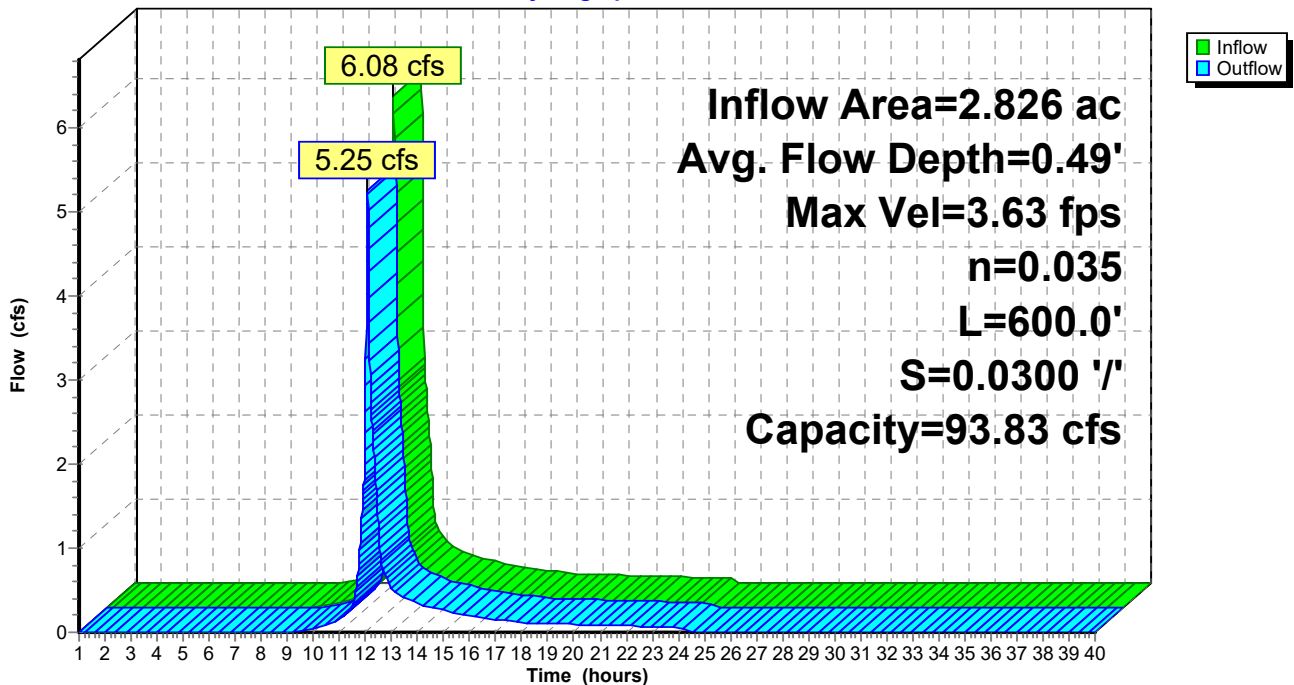
Peak Storage= 869 cf @ 12.03 hrs  
 Average Depth at Peak Storage= 0.49'  
 Bank-Full Depth= 2.00' Flow Area= 12.0 sf, Capacity= 93.83 cfs

2.00' x 2.00' deep channel, n= 0.035  
 Side Slope Z-value= 2.0 '/' Top Width= 10.00'  
 Length= 600.0' Slope= 0.0300 '/'  
 Inlet Invert= 1,720.00', Outlet Invert= 1,702.00'



**Reach 8R: Swale**

Hydrograph



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### Summary for Reach DPEast: Design Point

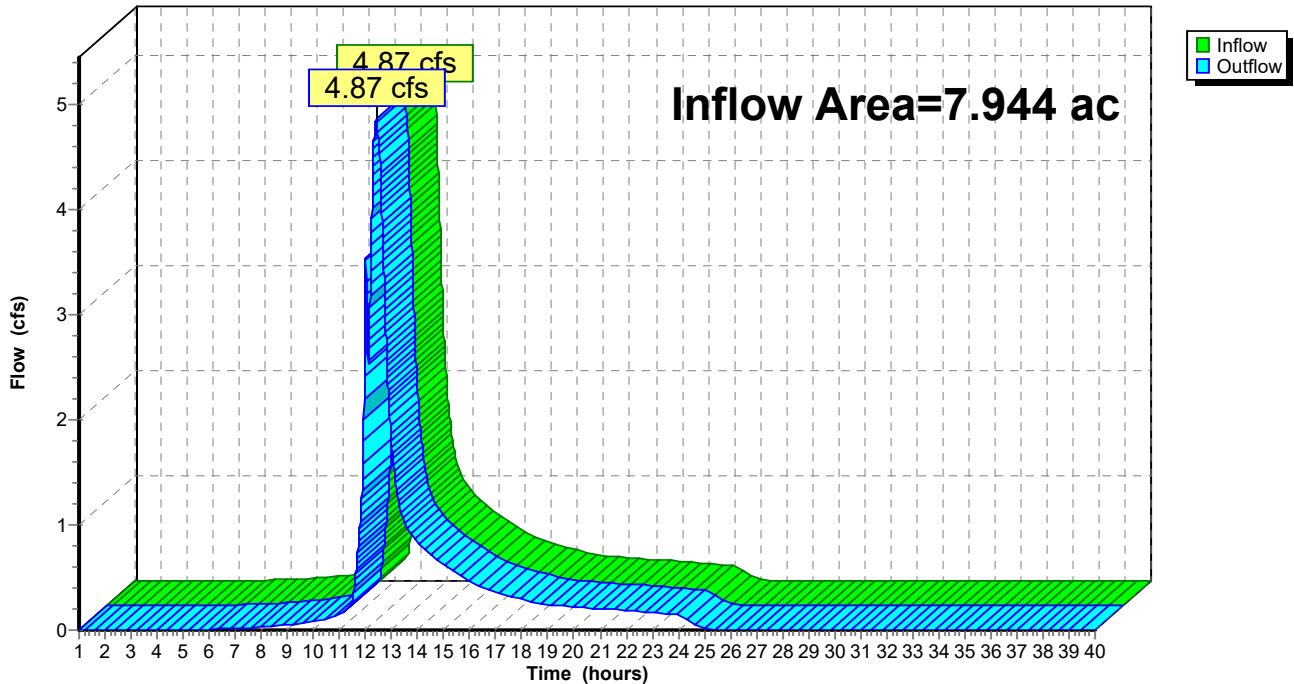
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 7.944 ac, 10.55% Impervious, Inflow Depth = 1.11" for 2 Year event  
Inflow = 4.87 cfs @ 12.40 hrs, Volume= 0.733 af  
Outflow = 4.87 cfs @ 12.40 hrs, Volume= 0.733 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs

### Reach DPEast: Design Point

Hydrograph



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SAMA Productions-Proposed Conditions

Type III 24-hr 2 Year Rainfall=3.07"

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## Summary for Reach DPWest: Design Point

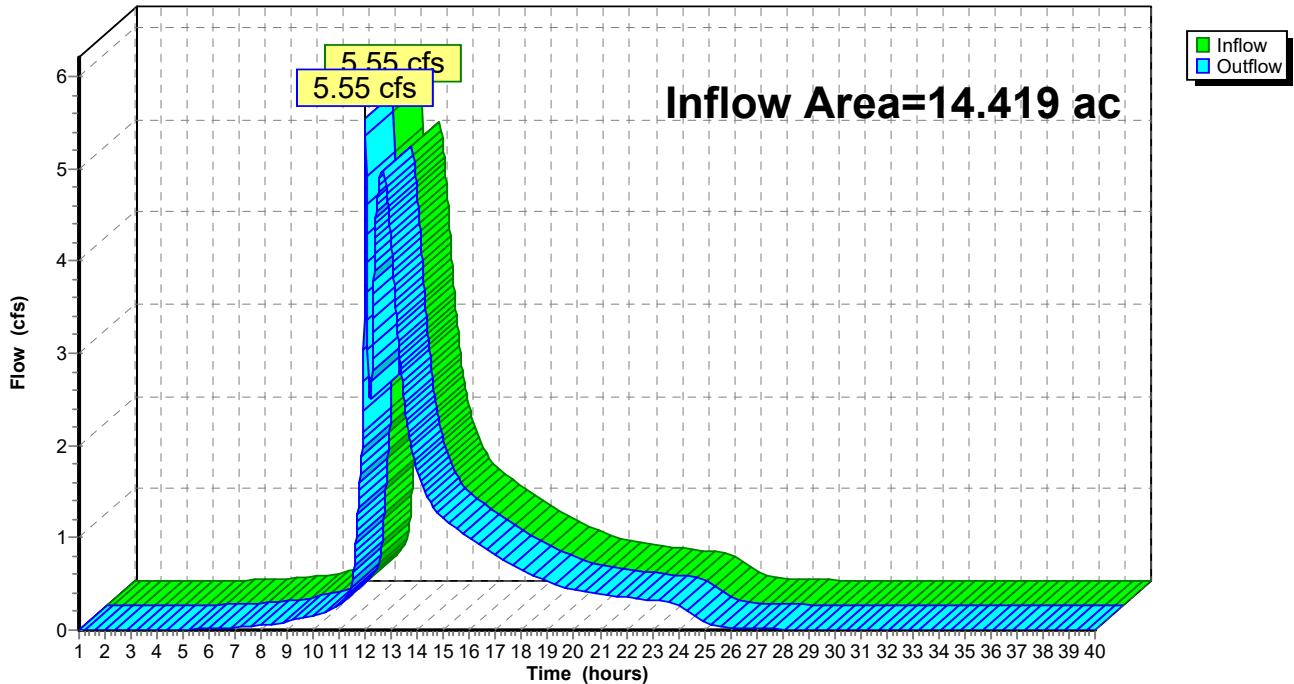
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 14.419 ac, 10.03% Impervious, Inflow Depth > 1.05" for 2 Year event  
Inflow = 5.55 cfs @ 12.00 hrs, Volume= 1.261 af  
Outflow = 5.55 cfs @ 12.00 hrs, Volume= 1.261 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs

### Reach DPWest: Design Point

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**Summary for Pond 1: one**

Inflow Area = 0.654 ac, 78.95% Impervious, Inflow Depth = 2.32" for 2 Year event  
 Inflow = 2.10 cfs @ 12.00 hrs, Volume= 0.127 af  
 Outflow = 2.09 cfs @ 12.00 hrs, Volume= 0.127 af, Atten= 1%, Lag= 0.1 min  
 Discarded = 0.04 cfs @ 12.00 hrs, Volume= 0.002 af  
 Primary = 2.05 cfs @ 12.00 hrs, Volume= 0.124 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,713.51' @ 12.00 hrs Surf.Area= 0.056 ac Storage= 0.000 af

Plug-Flow detention time= 0.1 min calculated for 0.127 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 788.8 - 788.7 )

Volume	Invert	Avail.Storage	Storage Description
#1A	1,713.50'	0.032 af	<b>16.37'W x 150.00'L x 1.48'H Field A</b> 0.083 af Overall - 0.004 af Embedded = 0.079 af x 40.0% Voids
#2A	1,713.83'	0.004 af	<b>CPP single-wall 4" x 84</b> Inside #1 Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf 12 Rows of 7 Chambers
		0.036 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	1,712.00'	<b>6.0" Round Culvert X 4.00</b> L= 150.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,712.00' / 1,711.00' S= 0.0067 '/' Cc= 0.900 n= 0.011, Flow Area= 0.20 sf
#2	Discarded	1,713.50'	<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 2.00'

**Discarded OutFlow** Max=0.06 cfs @ 12.00 hrs HW=1,713.51' (Free Discharge)  
 ↳ **2=Exfiltration** ( Controls 0.06 cfs)

**Primary OutFlow** Max=2.83 cfs @ 12.00 hrs HW=1,713.51' (Free Discharge)  
 ↳ **1=Culvert** (Barrel Controls 2.83 cfs @ 3.60 fps)



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**Pond 1: one - Chamber Wizard Field A**

**Chamber Model = CPP single-wall 4" (Single-wall corrugated HDPE pipe)**

Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf

Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf

4.7" Wide + 12.0" Spacing = 16.7" C-C Row Spacing

7 Chambers/Row x 20.00' Long = 140.00' Row Length +60.0" End Stone x 2 = 150.00' Base Length

12 Rows x 4.7" Wide + 12.0" Spacing x 11 + 4.0" Side Stone x 2 = 16.37' Base Width

4.0" Base + 4.7" Chamber Height + 9.0" Cover = 1.48' Field Height

84 Chambers x 2.2 cf = 182.8 cf Chamber Storage

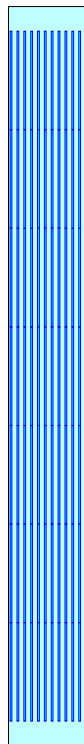
3,621.1 cf Field - 182.8 cf Chambers = 3,438.3 cf Stone x 40.0% Voids = 1,375.3 cf Stone Storage

Chamber Storage + Stone Storage = 1,558.1 cf = 0.036 af

Overall Storage Efficiency = 43.0%

Overall System Size = 150.00' x 16.37' x 1.48'

84 Chambers  
134.1 cy Field  
127.3 cy Stone



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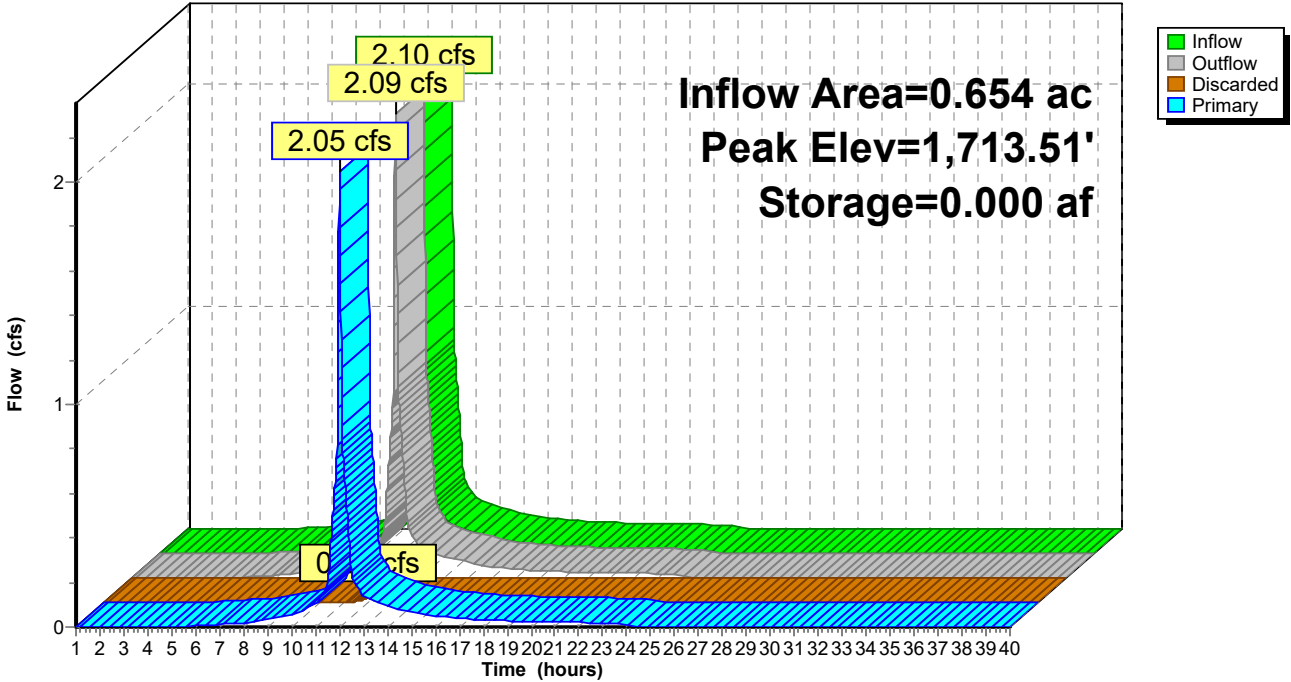
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**Pond 1: one**

Hydrograph



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**Summary for Pond 2: Two**

Inflow Area = 0.916 ac, 78.95% Impervious, Inflow Depth = 2.32" for 2 Year event  
 Inflow = 2.94 cfs @ 12.00 hrs, Volume= 0.177 af  
 Outflow = 2.93 cfs @ 12.00 hrs, Volume= 0.177 af, Atten= 0%, Lag= 0.1 min  
 Discarded = 0.04 cfs @ 12.00 hrs, Volume= 0.003 af  
 Primary = 2.89 cfs @ 12.00 hrs, Volume= 0.175 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,713.51' @ 12.00 hrs Surf.Area= 0.051 ac Storage= 0.000 af

Plug-Flow detention time= 0.1 min calculated for 0.177 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 788.8 - 788.7 )

Volume	Invert	Avail.Storage	Storage Description
#1A	1,713.50'	0.029 af	<b>21.93'W x 102.00'L x 1.48'H Field A</b> 0.076 af Overall - 0.004 af Embedded = 0.072 af x 40.0% Voids
#2A	1,713.83'	0.004 af	<b>CPP single-wall 4" x 80 Inside #1</b> Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf 16 Rows of 5 Chambers
		0.033 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	1,712.00'	<b>12.0" Round Culvert</b> L= 150.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,712.00' / 1,711.00' S= 0.0067 '/' Cc= 0.900 n= 0.011, Flow Area= 0.79 sf
#2	Discarded	1,713.50'	<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 2.00'

**Discarded OutFlow** Max=0.05 cfs @ 12.00 hrs HW=1,713.51' (Free Discharge)  
 ↳ **2=Exfiltration** ( Controls 0.05 cfs)

**Primary OutFlow** Max=3.51 cfs @ 12.00 hrs HW=1,713.51' (Free Discharge)  
 ↳ **1=Culvert** (Barrel Controls 3.51 cfs @ 4.47 fps)

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### Pond 2: Two - Chamber Wizard Field A

**Chamber Model = CPP single-wall 4" (Single-wall corrugated HDPE pipe)**

Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf

Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf

4.7" Wide + 12.0" Spacing = 16.7" C-C Row Spacing

5 Chambers/Row x 20.00' Long = 100.00' Row Length +12.0" End Stone x 2 = 102.00' Base Length

16 Rows x 4.7" Wide + 12.0" Spacing x 15 + 4.0" Side Stone x 2 = 21.93' Base Width

4.0" Base + 4.7" Chamber Height + 9.0" Cover = 1.48' Field Height

80 Chambers x 2.2 cf = 174.1 cf Chamber Storage

3,299.9 cf Field - 174.1 cf Chambers = 3,125.8 cf Stone x 40.0% Voids = 1,250.3 cf Stone Storage

Chamber Storage + Stone Storage = 1,424.4 cf = 0.033 af

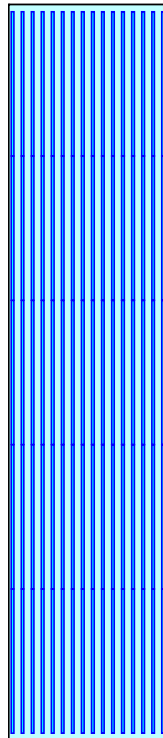
Overall Storage Efficiency = 43.2%

Overall System Size = 102.00' x 21.93' x 1.48'

80 Chambers

122.2 cy Field

115.8 cy Stone



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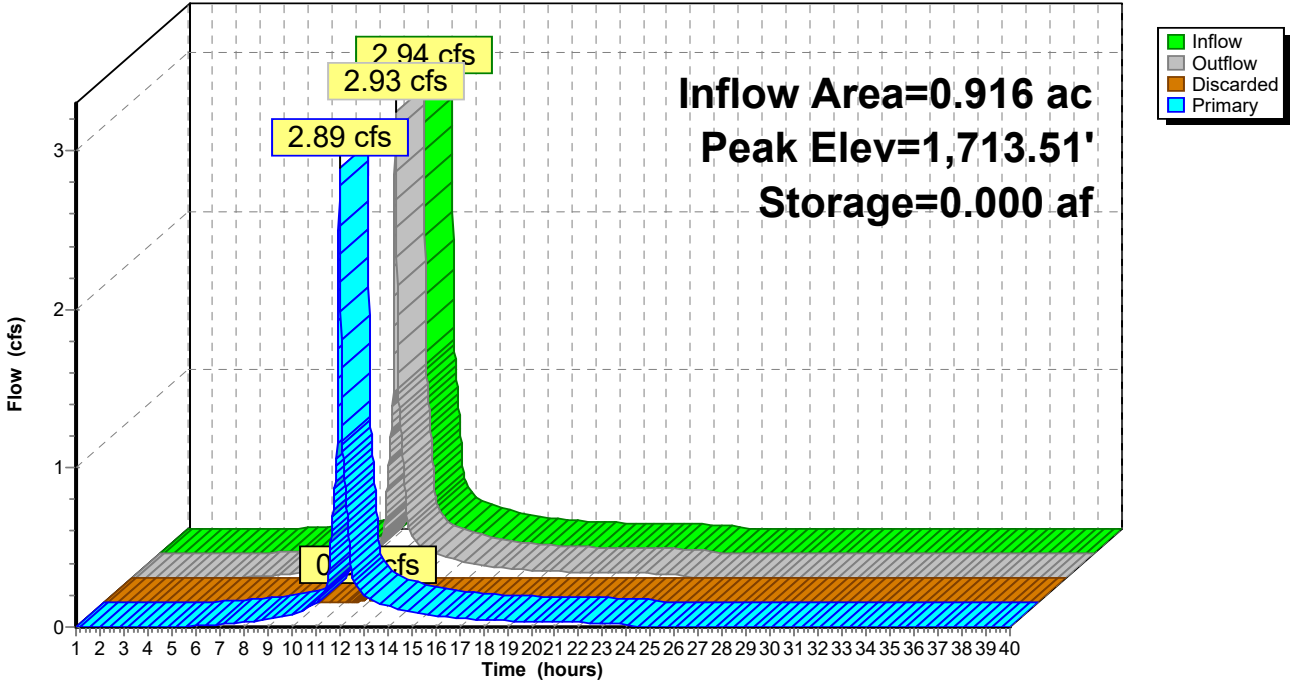
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**Pond 2: Two**

Hydrograph



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**Summary for Pond 3: three**

Inflow Area = 1.047 ac, 78.95% Impervious, Inflow Depth = 2.32" for 2 Year event  
 Inflow = 3.36 cfs @ 12.00 hrs, Volume= 0.202 af  
 Outflow = 3.34 cfs @ 12.00 hrs, Volume= 0.202 af, Atten= 1%, Lag= 0.1 min  
 Discarded = 0.06 cfs @ 12.00 hrs, Volume= 0.004 af  
 Primary = 3.29 cfs @ 12.00 hrs, Volume= 0.199 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,713.51' @ 12.00 hrs Surf.Area= 0.076 ac Storage= 0.000 af

Plug-Flow detention time= 0.1 min calculated for 0.202 af (100% of inflow)  
 Center-of-Mass det. time= 0.1 min ( 788.8 - 788.7 )

Volume	Invert	Avail.Storage	Storage Description
#1A	1,713.50'	0.042 af	<b>21.93'W x 150.00'L x 1.48'H Field A</b> 0.111 af Overall - 0.006 af Embedded = 0.106 af x 40.0% Voids
#2A	1,713.83'	0.006 af	<b>CPP single-wall 4" x 112 Inside #1</b> Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf 16 Rows of 7 Chambers
		0.048 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	1,712.00'	<b>6.0" Round Culvert X 6.00</b> L= 150.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,712.00' / 1,711.00' S= 0.0067 '/' Cc= 0.900 n= 0.011, Flow Area= 0.20 sf
#2	Discarded	1,713.50'	<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 2.00'

**Discarded OutFlow** Max=0.08 cfs @ 12.00 hrs HW=1,713.51' (Free Discharge)  
 ↑**2=Exfiltration** ( Controls 0.08 cfs)

**Primary OutFlow** Max=4.24 cfs @ 12.00 hrs HW=1,713.51' (Free Discharge)  
 ↑**1=Culvert** (Barrel Controls 4.24 cfs @ 3.60 fps)

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### Pond 3: three - Chamber Wizard Field A

#### Chamber Model = CPP single-wall 4" (Single-wall corrugated HDPE pipe)

Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf

Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf

4.7" Wide + 12.0" Spacing = 16.7" C-C Row Spacing

7 Chambers/Row x 20.00' Long = 140.00' Row Length +60.0" End Stone x 2 = 150.00' Base Length

16 Rows x 4.7" Wide + 12.0" Spacing x 15 + 4.0" Side Stone x 2 = 21.93' Base Width

4.0" Base + 4.7" Chamber Height + 9.0" Cover = 1.48' Field Height

112 Chambers x 2.2 cf = 243.8 cf Chamber Storage

4,852.7 cf Field - 243.8 cf Chambers = 4,609.0 cf Stone x 40.0% Voids = 1,843.6 cf Stone Storage

Chamber Storage + Stone Storage = 2,087.4 cf = 0.048 af

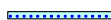
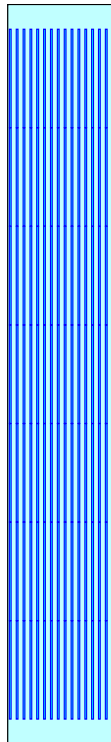
Overall Storage Efficiency = 43.0%

Overall System Size = 150.00' x 21.93' x 1.48'

112 Chambers

179.7 cy Field

170.7 cy Stone



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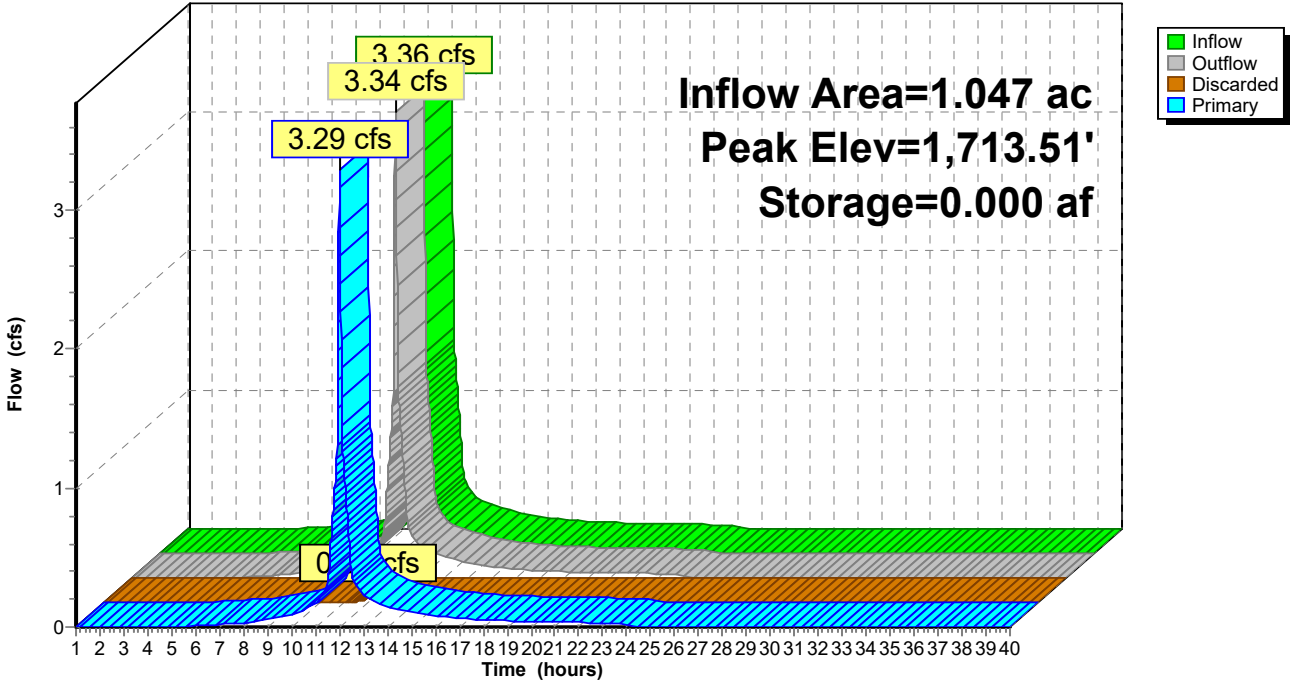
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**Pond 3: three**

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**Summary for Pond DP up: Dry Basin**

Inflow Area = 1.876 ac, 0.00% Impervious, Inflow Depth = 0.90" for 2 Year event  
 Inflow = 2.24 cfs @ 12.00 hrs, Volume= 0.141 af  
 Outflow = 0.07 cfs @ 16.98 hrs, Volume= 0.048 af, Atten= 97%, Lag= 298.7 min  
 Primary = 0.07 cfs @ 16.98 hrs, Volume= 0.048 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,731.40' @ 16.98 hrs Surf.Area= 0.082 ac Storage= 0.105 af

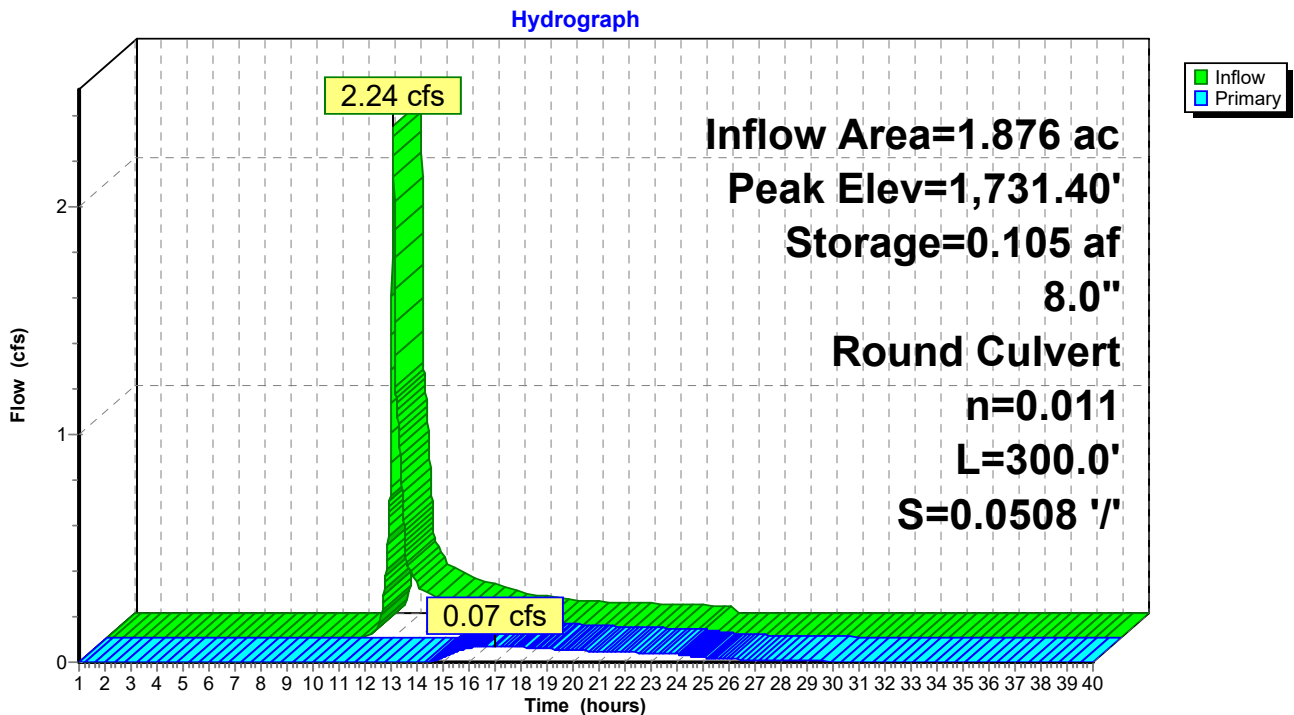
Plug-Flow detention time= 510.3 min calculated for 0.048 af (34% of inflow)  
 Center-of-Mass det. time= 366.5 min ( 1,229.1 - 862.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,730.00'	0.357 af	<b>45.00'W x 65.00'L x 4.00'H Hillside pond Z=2.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,731.25'	<b>8.0" Round Culvert</b> L= 300.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 1,731.25' / 1,716.00' S= 0.0508 '/ Cc= 0.900 n= 0.011, Flow Area= 0.35 sf

**Primary OutFlow** Max=0.07 cfs @ 16.98 hrs HW=1,731.40' (Free Discharge)  
 ←1=Culvert (Inlet Controls 0.07 cfs @ 1.18 fps)

**Pond DP up: Dry Basin**



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**Summary for Pond DPe: Pond**

[79] Warning: Submerged Pond FBe Primary device # 1 OUTLET by 1.61'

Inflow Area = 7.028 ac, 1.63% Impervious, Inflow Depth = 0.95" for 2 Year event  
 Inflow = 4.62 cfs @ 12.37 hrs, Volume= 0.558 af  
 Outflow = 4.37 cfs @ 12.45 hrs, Volume= 0.558 af, Atten= 5%, Lag= 5.1 min  
 Primary = 4.37 cfs @ 12.45 hrs, Volume= 0.558 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,707.61' @ 12.45 hrs Surf.Area= 0.022 ac Storage= 0.024 af

Plug-Flow detention time= 6.8 min calculated for 0.558 af (100% of inflow)  
 Center-of-Mass det. time= 6.8 min ( 890.3 - 883.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,706.00'	0.105 af	<b>10.00'W x 38.00'L x 4.00'H Pond Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,706.00'	<b>15.0" Round Culvert</b> L= 35.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,706.00' / 1,706.00' S= 0.0000 1' Cc= 0.900 n= 0.011, Flow Area= 1.23 sf
#2	Primary	1,708.00'	<b>10.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Primary OutFlow** Max=4.38 cfs @ 12.45 hrs HW=1,707.61' (Free Discharge)

- 1=Culvert (Barrel Controls 4.38 cfs @ 3.60 fps)
- 2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

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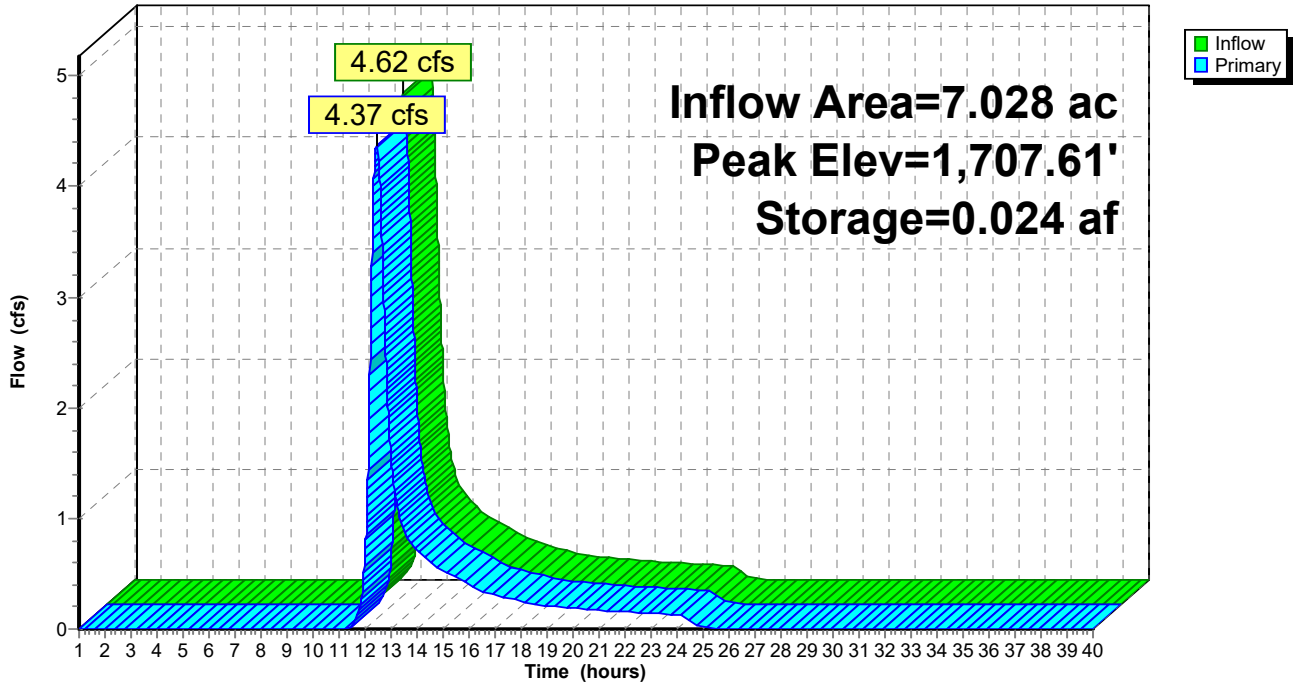
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## Pond DPe: Pond

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**Summary for Pond DPw: Dry pond**

[81] Warning: Exceeded Pond FBw by 0.13' @ 19.18 hrs

Inflow Area = 2.826 ac, 3.66% Impervious, Inflow Depth = 1.50" for 2 Year event  
 Inflow = 3.40 cfs @ 12.16 hrs, Volume= 0.354 af  
 Outflow = 1.89 cfs @ 12.47 hrs, Volume= 0.289 af, Atten= 44%, Lag= 18.4 min  
 Primary = 1.89 cfs @ 12.47 hrs, Volume= 0.289 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,703.17' @ 12.47 hrs Surf.Area= 0.048 ac Storage= 0.094 af

Plug-Flow detention time= 133.8 min calculated for 0.289 af (82% of inflow)  
 Center-of-Mass det. time= 61.1 min ( 932.5 - 871.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,700.00'	0.153 af	<b>16.00'W x 40.00'L x 4.25'H Prismatic Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,702.50'	<b>15.0" Round Culvert</b> L= 40.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,702.50' / 1,699.00' S= 0.0875 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf
#2	Primary	1,700.00'	<b>4.0" Round Culvert X 0.00</b> L= 30.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,700.00' / 1,696.00' S= 0.1333 '/ Cc= 0.900 n= 0.011, Flow Area= 0.09 sf
#3	Primary	1,703.50'	<b>8.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Primary OutFlow** Max=1.89 cfs @ 12.47 hrs HW=1,703.17' (Free Discharge)

- 1=Culvert (Inlet Controls 1.89 cfs @ 2.80 fps)
- 2=Culvert ( Controls 0.00 cfs)
- 3=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

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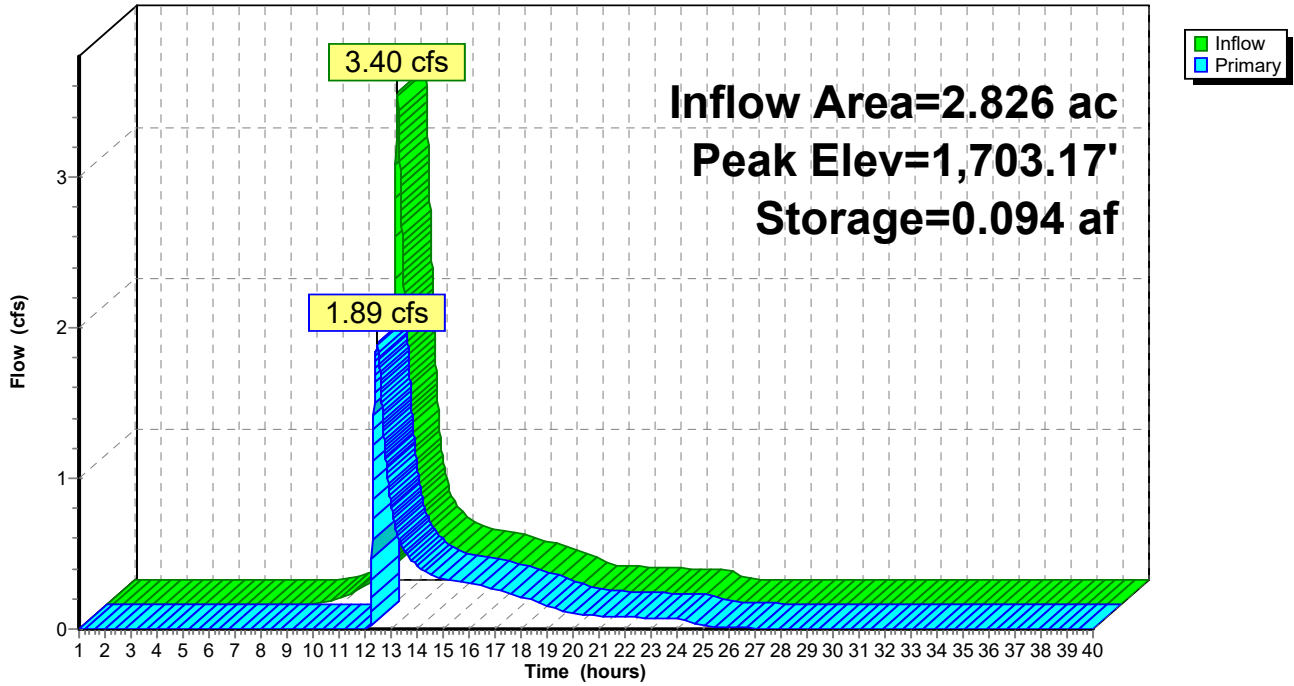
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## Pond DPw: Dry pond

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**Summary for Pond FBe: ForeBay**

Inflow Area = 7.028 ac, 1.63% Impervious, Inflow Depth = 0.95" for 2 Year event  
 Inflow = 4.64 cfs @ 12.35 hrs, Volume= 0.558 af  
 Outflow = 4.62 cfs @ 12.37 hrs, Volume= 0.558 af, Atten= 0%, Lag= 0.9 min  
 Primary = 4.62 cfs @ 12.37 hrs, Volume= 0.558 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,708.70' @ 12.37 hrs Surf.Area= 0.014 ac Storage= 0.008 af

Plug-Flow detention time= 2.7 min calculated for 0.558 af (100% of inflow)  
 Center-of-Mass det. time= 2.7 min ( 883.5 - 880.8 )

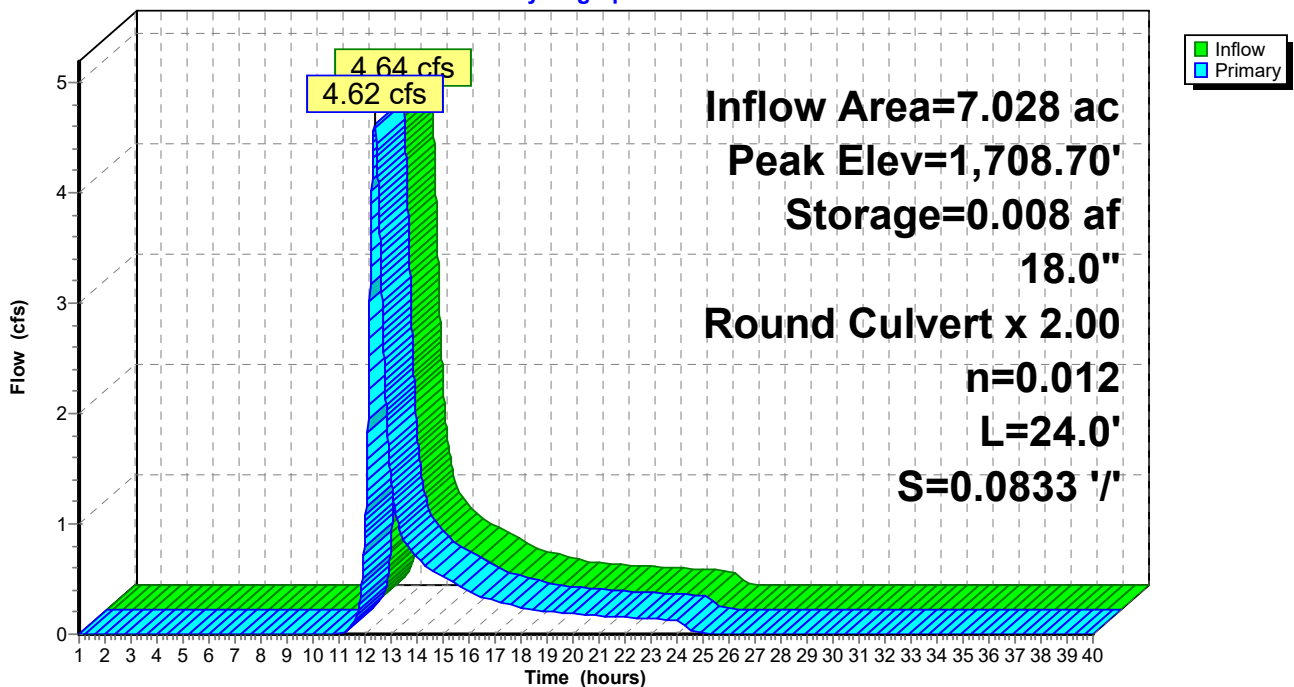
Volume	Invert	Avail.Storage	Storage Description
#1	1,708.00'	0.109 af	<b>10.00'W x 40.00'L x 4.00'H Prismaoid Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,708.00'	<b>18.0" Round Culvert X 2.00</b> L= 24.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,708.00' / 1,706.00' S= 0.0833 1/1' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

**Primary OutFlow** Max=4.62 cfs @ 12.37 hrs HW=1,708.70' (Free Discharge)  
 ←1=Culvert (Inlet Controls 4.62 cfs @ 2.85 fps)

**Pond FBe: ForeBay**

Hydrograph



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**Summary for Pond FBw: Forebay**

[62] Hint: Exceeded Reach 8R OUTLET depth by 1.97' @ 12.18 hrs

Inflow Area = 2.826 ac, 3.66% Impervious, Inflow Depth = 1.50" for 2 Year event  
 Inflow = 5.25 cfs @ 12.08 hrs, Volume= 0.354 af  
 Outflow = 3.40 cfs @ 12.16 hrs, Volume= 0.354 af, Atten= 35%, Lag= 5.1 min  
 Primary = 3.40 cfs @ 12.16 hrs, Volume= 0.354 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,704.31' @ 12.16 hrs Surf.Area= 0.050 ac Storage= 0.067 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 33.2 min ( 871.3 - 838.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,702.50'	0.237 af	<b>16.00'W x 70.00'L x 4.25'H Prismatic Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,703.50'	<b>18.0" Round Culvert</b> L= 40.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,703.50' / 1,703.00' S= 0.0125 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf
#2	Primary	1,702.25'	<b>4.0" Round Culvert</b> L= 35.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 1,702.25' / 1,702.00' S= 0.0071 '/' Cc= 0.900 n= 0.011, Flow Area= 0.09 sf

**Primary OutFlow** Max=3.39 cfs @ 12.16 hrs HW=1,704.31' (Free Discharge)

- 1=Culvert (Inlet Controls 2.96 cfs @ 3.06 fps)

- 2=Culvert (Barrel Controls 0.44 cfs @ 4.99 fps)

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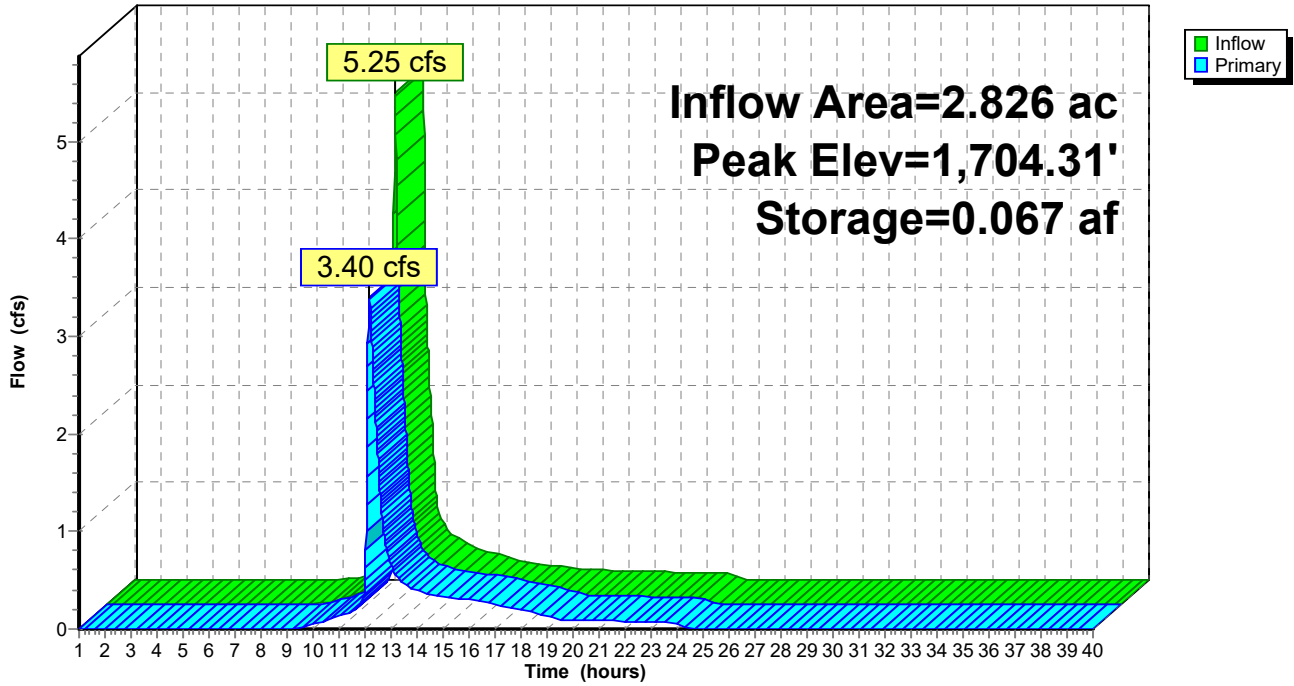
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## Pond FBw: Forebay

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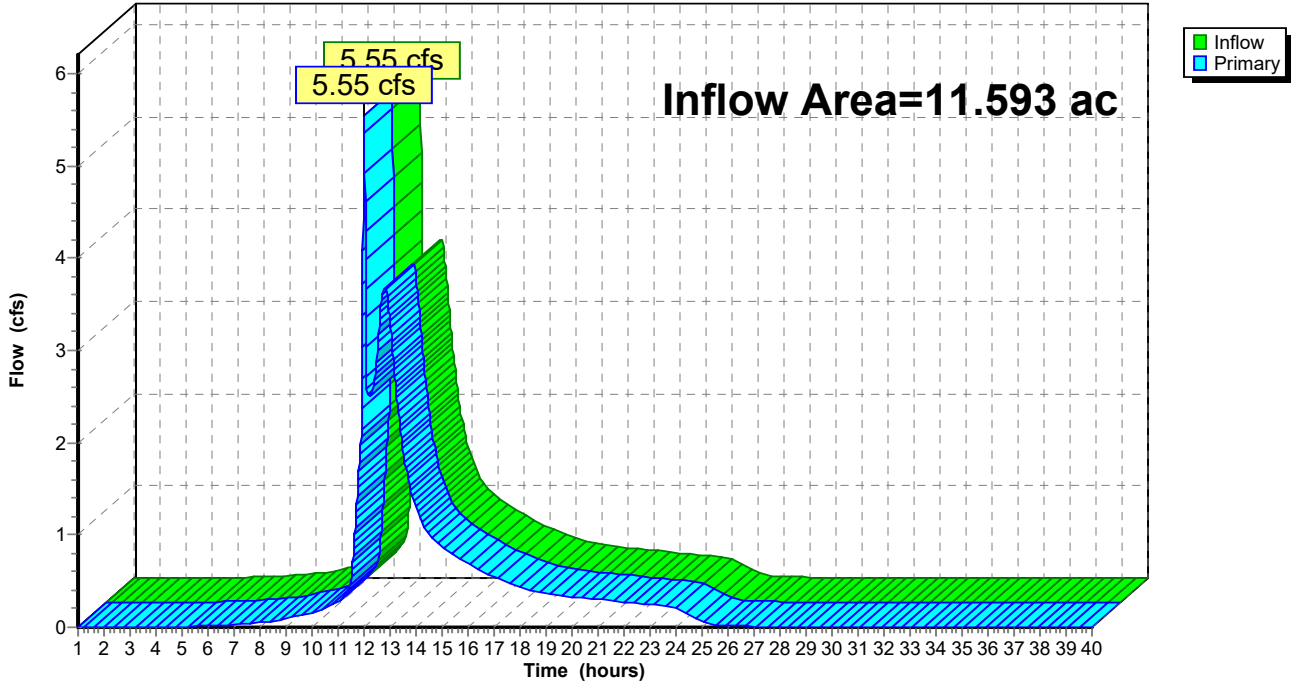
**Summary for Link 7L: wetland**

Inflow Area = 11.593 ac, 11.58% Impervious, Inflow Depth > 1.01" for 2 Year event  
Inflow = 5.55 cfs @ 12.00 hrs, Volume= 0.972 af  
Primary = 5.55 cfs @ 12.00 hrs, Volume= 0.972 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs

**Link 7L: wetland**

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Time span=1.00-40.00 hrs, dt=0.01 hrs, 3901 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment 1S: Upgradient forest** Runoff Area=349,185 sf 0.00% Impervious Runoff Depth=1.96"  
Flow Length=1,729' Tc=54.5 min CN=73 Runoff=7.57 cfs 1.312 af

**Subcatchment 2S: Easterly Watershed** Runoff Area=306,150 sf 1.63% Impervious Runoff Depth=2.04"  
Flow Length=1,201' Tc=23.3 min CN=74 Runoff=10.49 cfs 1.196 af

**Subcatchment 4S: upgradient forest by-pass** Runoff Area=81,700 sf 0.00% Impervious Runoff Depth=1.96"  
Tc=0.0 min CN=73 Runoff=5.22 cfs 0.307 af

**Subcatchment 6S: development area** Runoff Area=123,100 sf 3.66% Impervious Runoff Depth=2.81"  
Tc=0.0 min CN=83 Runoff=11.36 cfs 0.661 af

**Subcatchment I: Greenhouse 1** Runoff Area=28,500 sf 78.95% Impervious Runoff Depth=3.80"  
Flow Length=25' Slope=0.2000 '/' Tc=0.2 min CN=93 Runoff=3.35 cfs 0.207 af

**Subcatchment II: Greenhouse 2** Runoff Area=39,900 sf 78.95% Impervious Runoff Depth=3.80"  
Flow Length=25' Slope=0.2000 '/' Tc=0.2 min CN=93 Runoff=4.68 cfs 0.290 af

**Subcatchment III: Greenhouse 3** Runoff Area=45,600 sf 78.95% Impervious Runoff Depth=3.80"  
Flow Length=25' Slope=0.2000 '/' Tc=0.2 min CN=93 Runoff=5.35 cfs 0.331 af

**Reach 8R: Swale** Avg. Flow Depth=0.69' Max Vel=4.36 fps Inflow=11.36 cfs 0.661 af  
n=0.035 L=600.0' S=0.0300 '/' Capacity=93.83 cfs Outflow=10.06 cfs 0.661 af

**Reach DPEast: Design Point** Inflow=11.44 cfs 1.481 af  
Outflow=11.44 cfs 1.481 af

**Reach DPWest: Design Point** Inflow=12.09 cfs 2.650 af  
Outflow=12.09 cfs 2.650 af

**Pond 1: one** Peak Elev=1,713.56' Storage=0.001 af Inflow=3.35 cfs 0.207 af  
Discarded=0.06 cfs 0.004 af Primary=2.86 cfs 0.203 af Outflow=2.92 cfs 0.207 af

**Pond 2: Two** Peak Elev=1,713.68' Storage=0.004 af Inflow=4.68 cfs 0.290 af  
Discarded=0.05 cfs 0.004 af Primary=3.71 cfs 0.286 af Outflow=3.76 cfs 0.290 af

**Pond 3: three** Peak Elev=1,713.61' Storage=0.003 af Inflow=5.35 cfs 0.331 af  
Discarded=0.08 cfs 0.006 af Primary=4.34 cfs 0.325 af Outflow=4.42 cfs 0.331 af

**Pond DP up: Dry Basin** Peak Elev=1,731.82' Storage=0.139 af Inflow=5.22 cfs 0.307 af  
8.0" Round Culvert n=0.011 L=300.0' S=0.0508 '/' Outflow=0.72 cfs 0.214 af

**Pond DPe: Pond** Peak Elev=1,708.28' Storage=0.040 af Inflow=10.47 cfs 1.196 af  
Outflow=10.45 cfs 1.196 af

**Pond DPw: Dry pond** Peak Elev=1,703.73' Storage=0.123 af Inflow=7.43 cfs 0.661 af  
Outflow=6.71 cfs 0.595 af

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Type III 24-hr 10 Year Rainfall=4.59"

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**Pond FBe: ForeBay**

Peak Elev=1,709.14' Storage=0.015 af Inflow=10.49 cfs 1.196 af  
18.0" Round Culvert x 2.00 n=0.012 L=24.0' S=0.0833 '/' Outflow=10.47 cfs 1.196 af

**Pond FBw: Forebay**

Peak Elev=1,704.91' Storage=0.100 af Inflow=10.06 cfs 0.661 af  
Outflow=7.43 cfs 0.661 af

**Link 7L: wetland**

Inflow=8.94 cfs 2.054 af  
Primary=8.94 cfs 2.054 af

**Total Runoff Area = 22.363 ac Runoff Volume = 4.303 af Average Runoff Depth = 2.31"**  
**89.79% Pervious = 20.079 ac 10.21% Impervious = 2.284 ac**

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**Summary for Subcatchment 1S: Upgradient forest**

Runoff = 7.57 cfs @ 12.78 hrs, Volume= 1.312 af, Depth= 1.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 10 Year Rainfall=4.59"

Area (sf)	CN	Description
* 349,185	73	Woods, Fair, HSG C (fair condition C/D soil)
349,185		100.00% Pervious Area

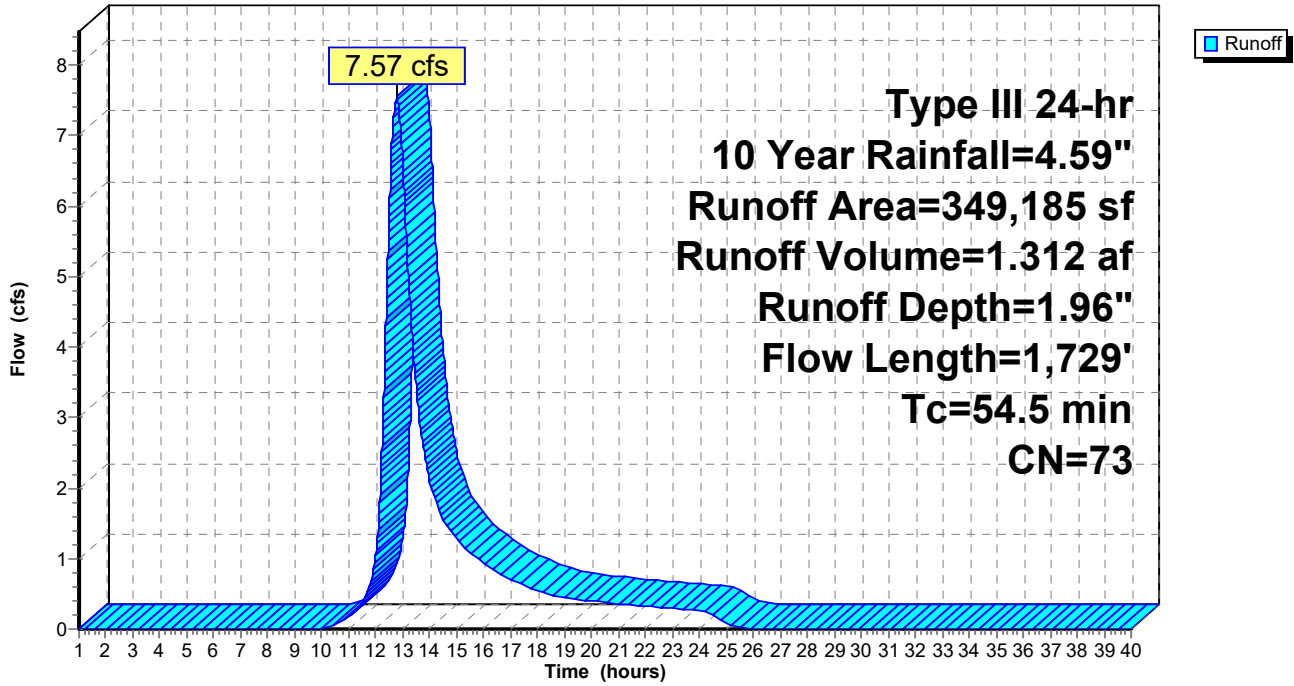
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.8	50	0.0100	0.03		<b>Sheet Flow, Sheet Flow</b> Woods: Dense underbrush n= 0.800 P2= 2.90"
1.9	138	0.0580	1.20		<b>Shallow Concentrated Flow, hillside segment 1</b> Woodland Kv= 5.0 fps
0.8	127	0.2519	2.51		<b>Shallow Concentrated Flow, Hillside segment 2</b> Woodland Kv= 5.0 fps
5.8	461	0.0694	1.32		<b>Shallow Concentrated Flow, Hillside Segment 3</b> Woodland Kv= 5.0 fps
4.9	258	0.0310	0.88		<b>Shallow Concentrated Flow, Hillside Segment 4</b> Woodland Kv= 5.0 fps
11.3	695	0.0417	1.02		<b>Shallow Concentrated Flow, Hillside Segment 5</b> Woodland Kv= 5.0 fps
54.5	1,729	Total			

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**Subcatchment 1S: Upgradient forest**

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**Summary for Subcatchment 2S: Easterly Watershed**

Runoff = 10.49 cfs @ 12.34 hrs, Volume= 1.196 af, Depth= 2.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 10 Year Rainfall=4.59"

Area (sf)	CN	Description
4,500	73	Woods, Fair, HSG C
5,000	98	Roofs, HSG C
36,500	96	Gravel surface, HSG C
10,000	74	>75% Grass cover, Good, HSG C
12,900	71	Meadow, non-grazed, HSG C
237,250	70	Woods, Good, HSG C
306,150	74	Weighted Average
301,150		98.37% Pervious Area
5,000		1.63% Impervious Area

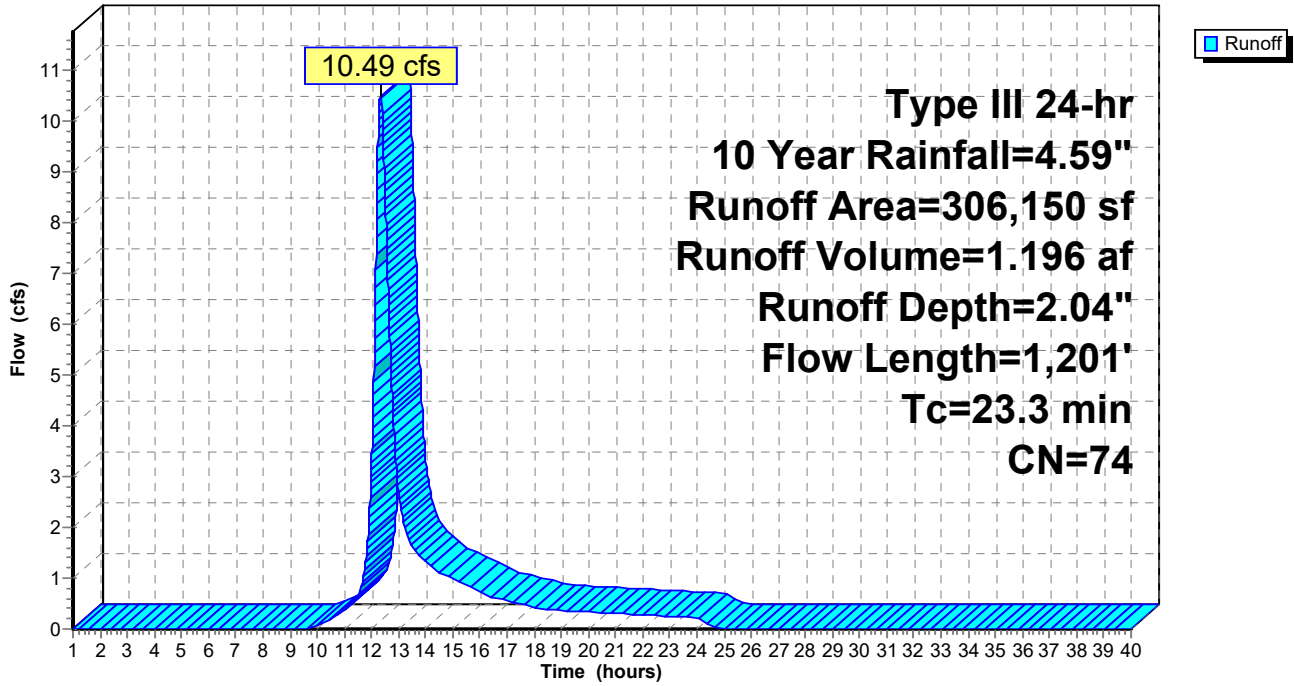
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0	50	0.0200	0.06		<b>Sheet Flow, Sheet Flow</b> Woods: Light underbrush n= 0.400 P2= 2.90"
2.7	277	0.1155	1.70		<b>Shallow Concentrated Flow, Hillside Segment 1</b> Woodland Kv= 5.0 fps
6.9	524	0.0648	1.27		<b>Shallow Concentrated Flow, Hillside Segment #2</b> Woodland Kv= 5.0 fps
0.7	350	0.0500	7.85	23.56	<b>Channel Flow,</b> Area= 3.0 sf Perim= 8.0' r= 0.38' n= 0.022 Earth, clean & straight
23.3	1,201	Total			

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**Subcatchment 2S: Easterly Watershed**

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## Summary for Subcatchment 4S: upgradient forest by-pass

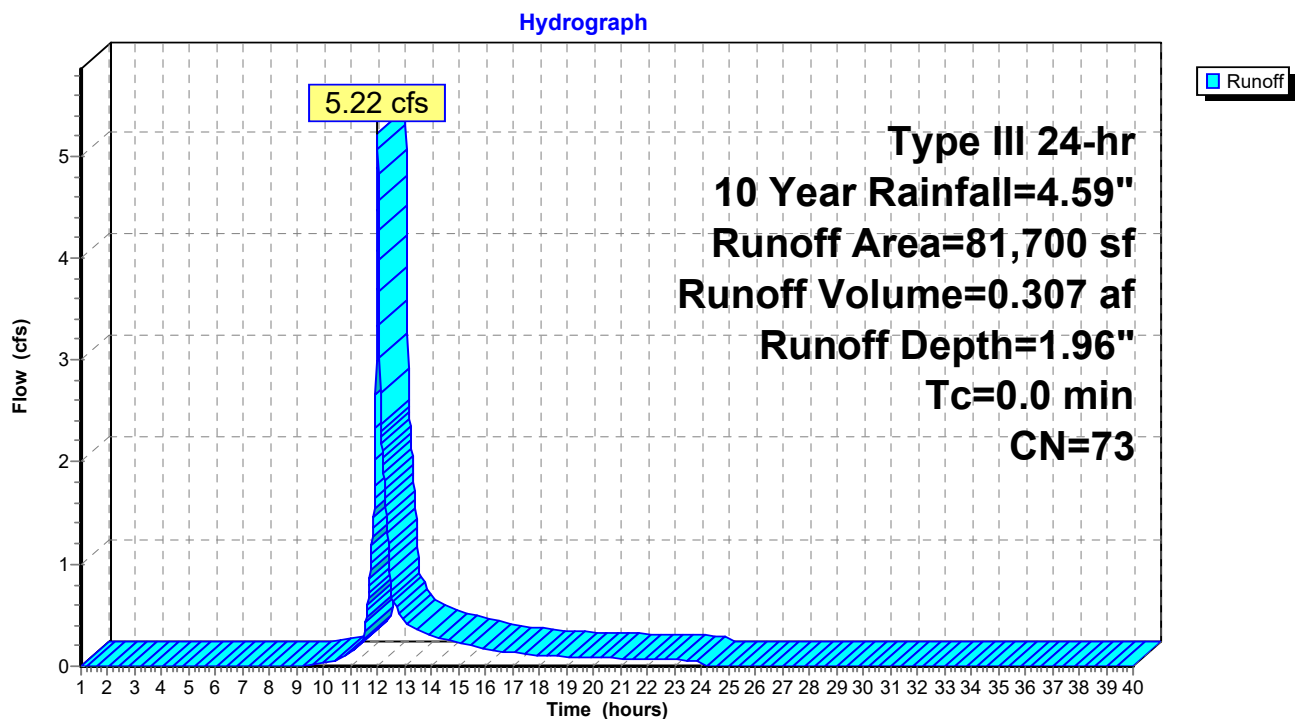
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 5.22 cfs @ 12.00 hrs, Volume= 0.307 af, Depth= 1.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Type III 24-hr 10 Year Rainfall=4.59"

Area (sf)	CN	Description
76,000	73	Woods, Fair, HSG C
5,700	71	Meadow, non-grazed, HSG C
81,700	73	Weighted Average
81,700		100.00% Pervious Area

## Subcatchment 4S: upgradient forest by-pass





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## Summary for Subcatchment 6S: development area

[46] Hint: Tc=0 (Instant runoff peak depends on dt)

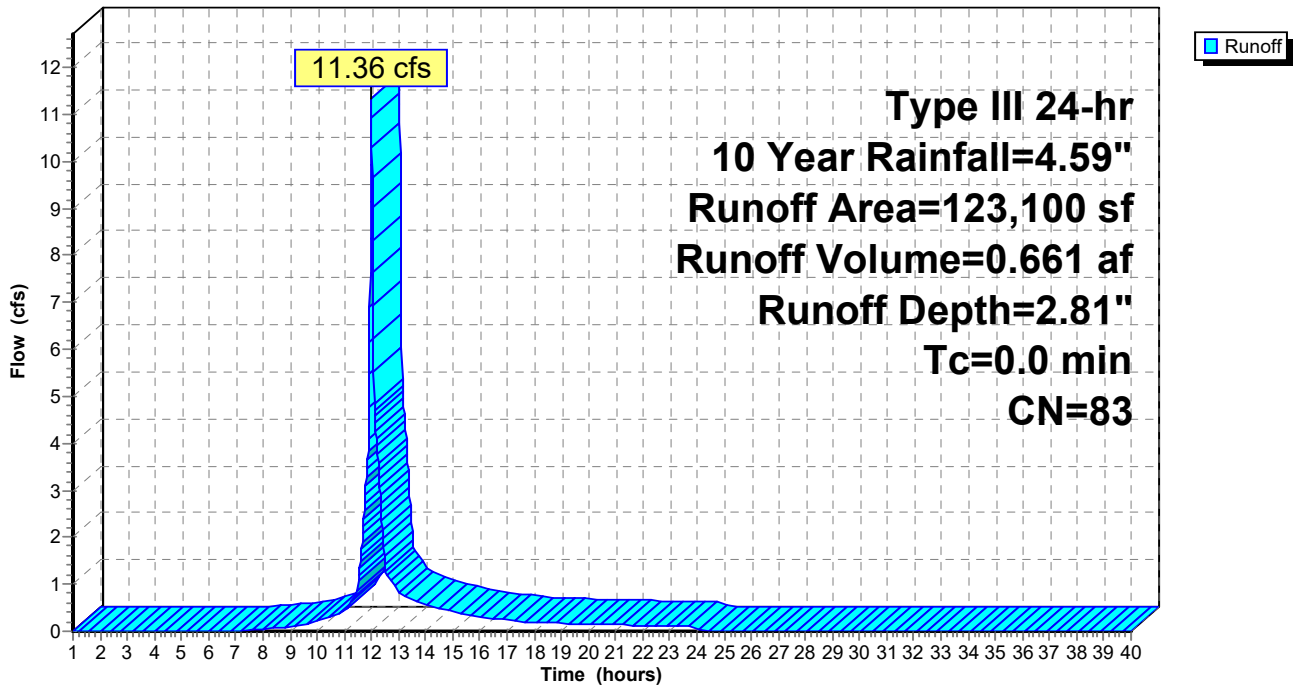
Runoff = 11.36 cfs @ 12.00 hrs, Volume= 0.661 af, Depth= 2.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Type III 24-hr 10 Year Rainfall=4.59"

Area (sf)	CN	Description
54,800	96	Gravel surface, HSG C
* 4,500	98	rooftop
63,800	71	Meadow, non-grazed, HSG C
123,100	83	Weighted Average
118,600		96.34% Pervious Area
4,500		3.66% Impervious Area

## Subcatchment 6S: development area

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 Type III 24-hr 10 Year Rainfall=4.59"  
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**Summary for Subcatchment I: Greenhouse 1**

[49] Hint: Tc<2dt may require smaller dt

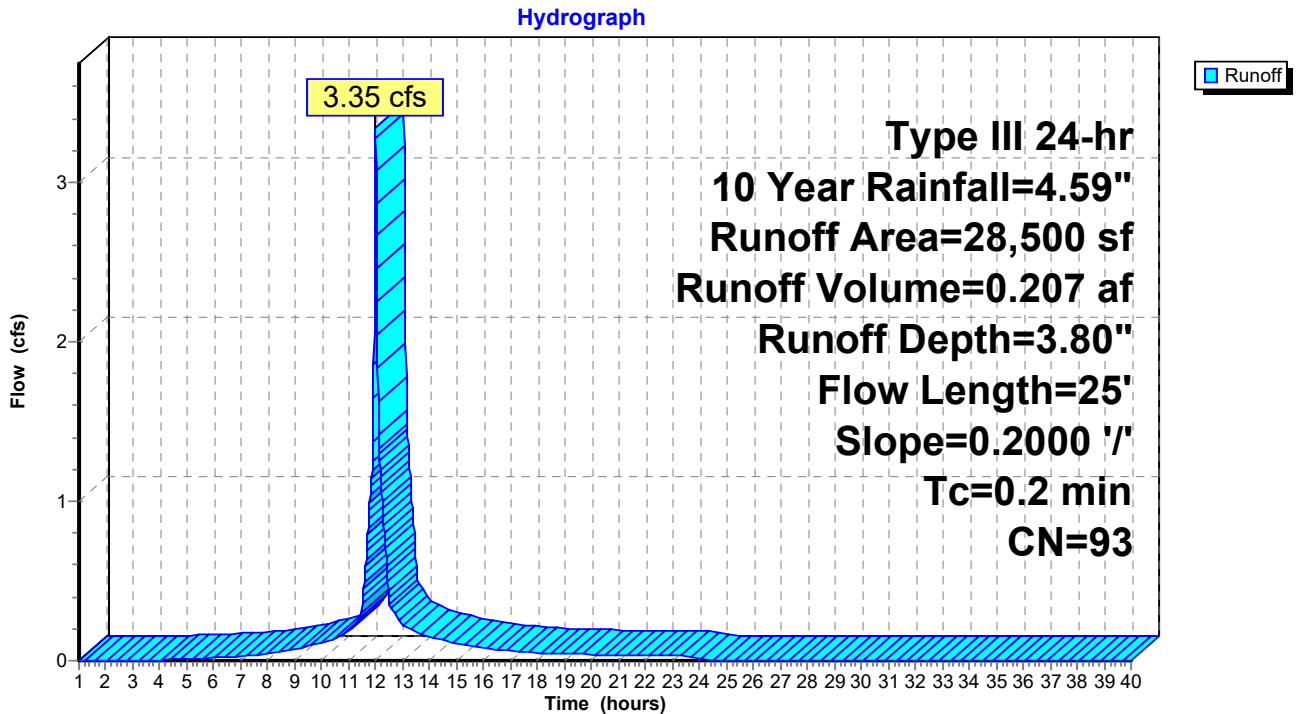
Runoff = 3.35 cfs @ 12.00 hrs, Volume= 0.207 af, Depth= 3.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 10 Year Rainfall=4.59"

	Area (sf)	CN	Description
*	22,500	98	Roof
	6,000	74	>75% Grass cover, Good, HSG C
	28,500	93	Weighted Average
	6,000		21.05% Pervious Area
	22,500		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	25	0.2000	2.49		<b>Sheet Flow, Roof</b> Smooth surfaces n= 0.011 P2= 2.90"

**Subcatchment I: Greenhouse 1**



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## Summary for Subcatchment II: Greenhouse 2

[49] Hint:  $T_c < 2dt$  may require smaller  $dt$

Runoff = 4.68 cfs @ 12.00 hrs, Volume= 0.290 af, Depth= 3.80"

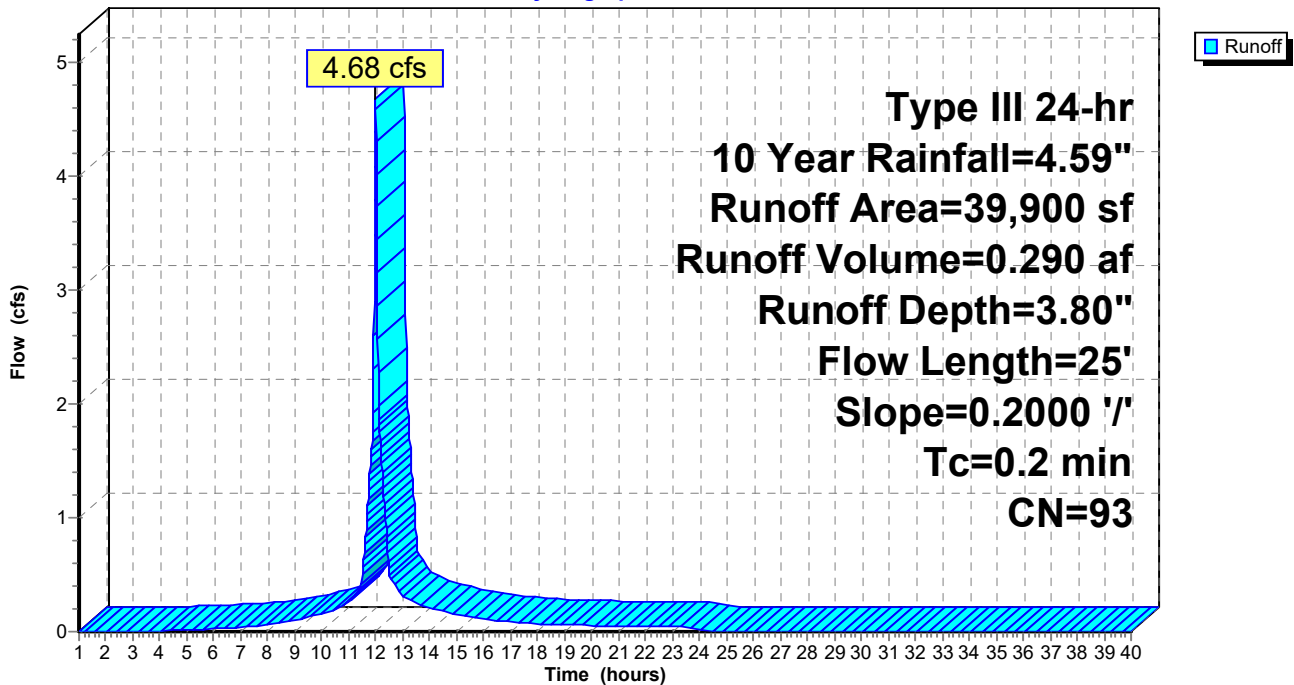
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs,  $dt=0.01$  hrs  
Type III 24-hr 10 Year Rainfall=4.59"

	Area (sf)	CN	Description
*	31,500	98	Roof
	8,400	74	>75% Grass cover, Good, HSG C
	39,900	93	Weighted Average
	8,400		21.05% Pervious Area
	31,500		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	25	0.2000	2.49		Sheet Flow, Roof Smooth surfaces n= 0.011 P2= 2.90"

## Subcatchment II: Greenhouse 2

Hydrograph



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**Summary for Subcatchment III: Greenhouse 3**

[49] Hint:  $T_c < 2dt$  may require smaller dt

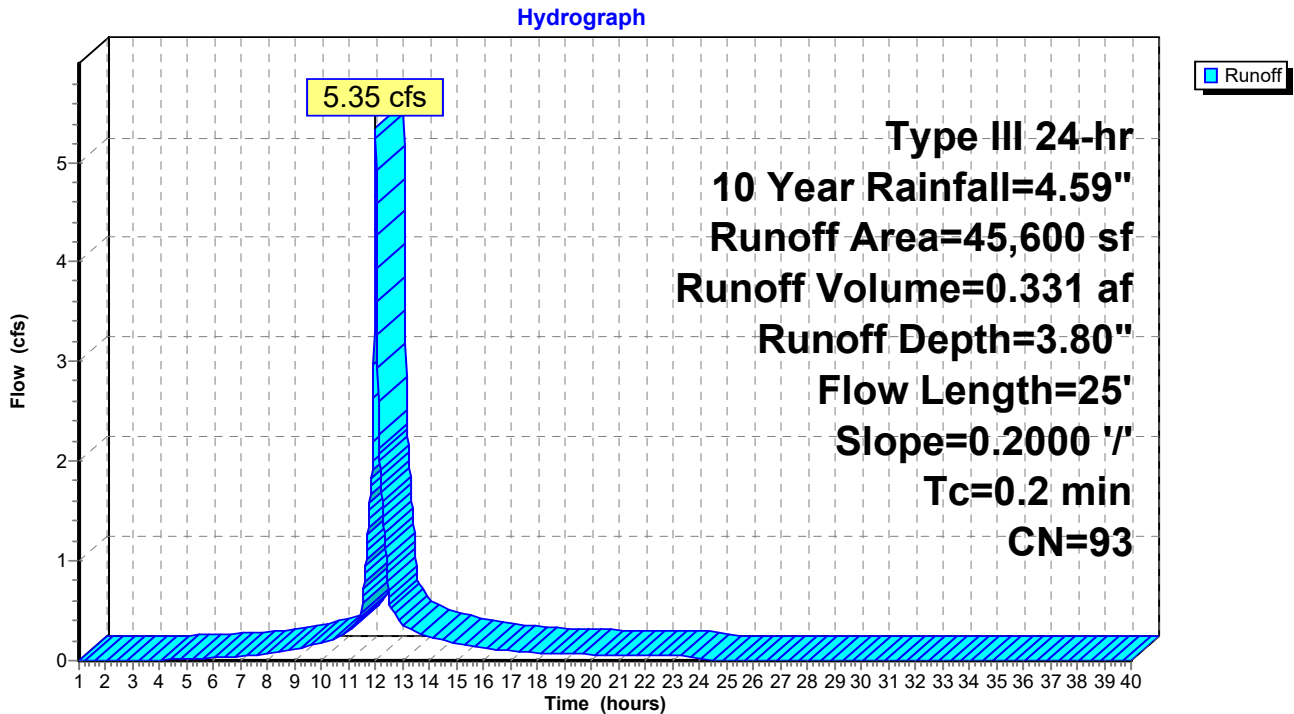
Runoff = 5.35 cfs @ 12.00 hrs, Volume= 0.331 af, Depth= 3.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 10 Year Rainfall=4.59"

	Area (sf)	CN	Description
*	36,000	98	Roof
	9,600	74	>75% Grass cover, Good, HSG C
	45,600	93	Weighted Average
	9,600		21.05% Pervious Area
	36,000		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	25	0.2000	2.49		Sheet Flow, Roof Smooth surfaces n= 0.011 P2= 2.90"

**Subcatchment III: Greenhouse 3**



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Type III 24-hr 10 Year Rainfall=4.59"

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## Summary for Reach 8R: Swale

Inflow Area = 2.826 ac, 3.66% Impervious, Inflow Depth = 2.81" for 10 Year event  
Inflow = 11.36 cfs @ 12.00 hrs, Volume= 0.661 af  
Outflow = 10.06 cfs @ 12.06 hrs, Volume= 0.661 af, Atten= 11%, Lag= 3.7 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Max. Velocity= 4.36 fps, Min. Travel Time= 2.3 min  
Avg. Velocity = 1.28 fps, Avg. Travel Time= 7.8 min

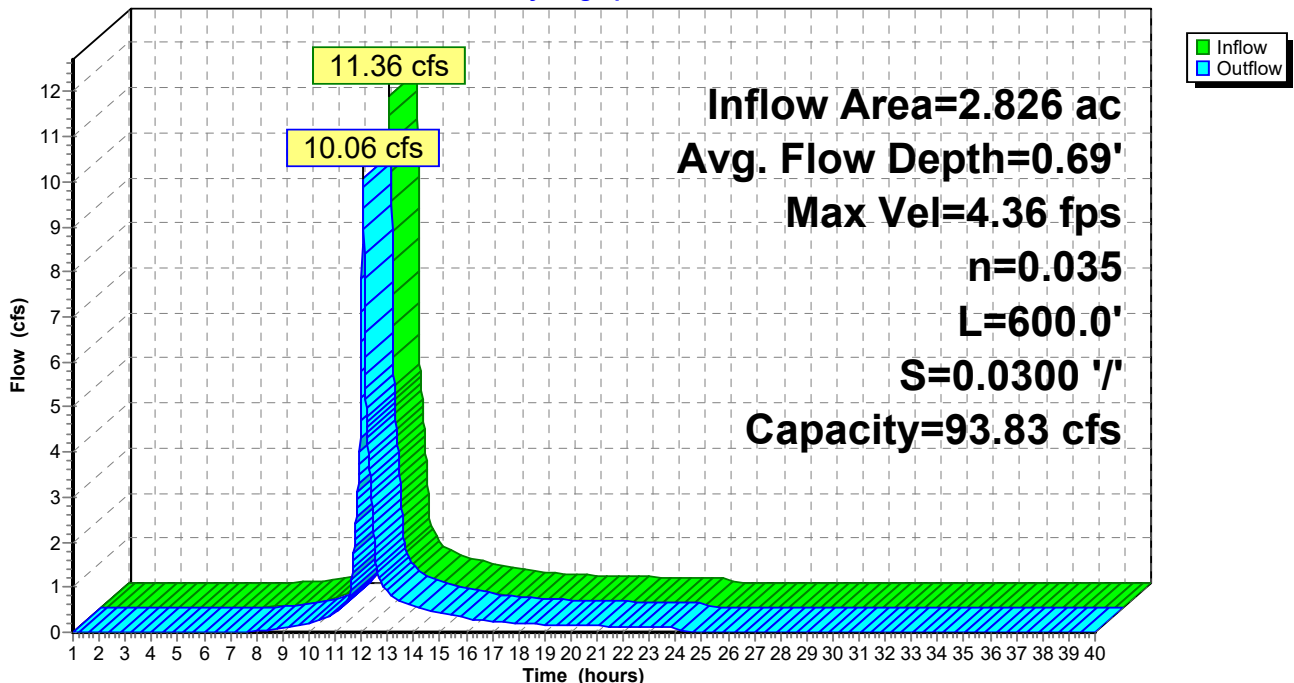
Peak Storage= 1,388 cf @ 12.02 hrs  
Average Depth at Peak Storage= 0.69'  
Bank-Full Depth= 2.00' Flow Area= 12.0 sf, Capacity= 93.83 cfs

2.00' x 2.00' deep channel, n= 0.035  
Side Slope Z-value= 2.0 '/' Top Width= 10.00'  
Length= 600.0' Slope= 0.0300 '/'  
Inlet Invert= 1,720.00', Outlet Invert= 1,702.00'



### Reach 8R: Swale

#### Hydrograph



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## Summary for Reach DPEast: Design Point

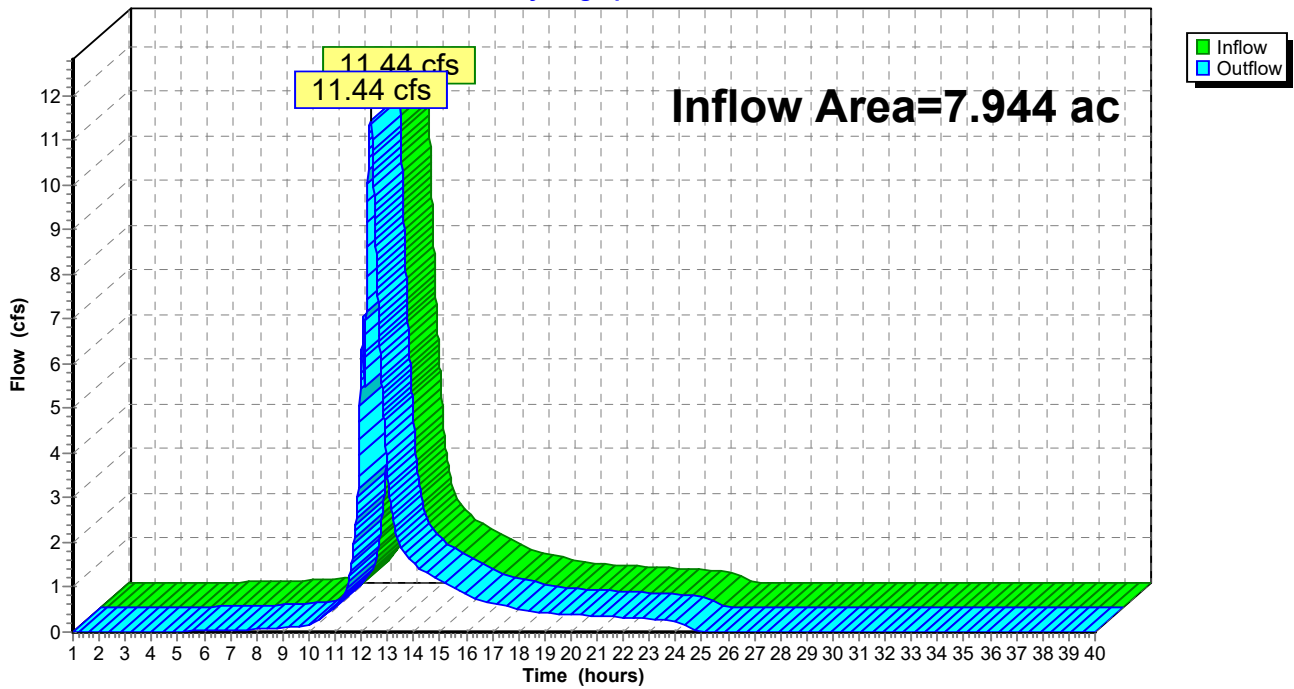
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 7.944 ac, 10.55% Impervious, Inflow Depth = 2.24" for 10 Year event  
Inflow = 11.44 cfs @ 12.35 hrs, Volume= 1.481 af  
Outflow = 11.44 cfs @ 12.35 hrs, Volume= 1.481 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs

### Reach DPEast: Design Point

Hydrograph



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## Summary for Reach DPWest: Design Point

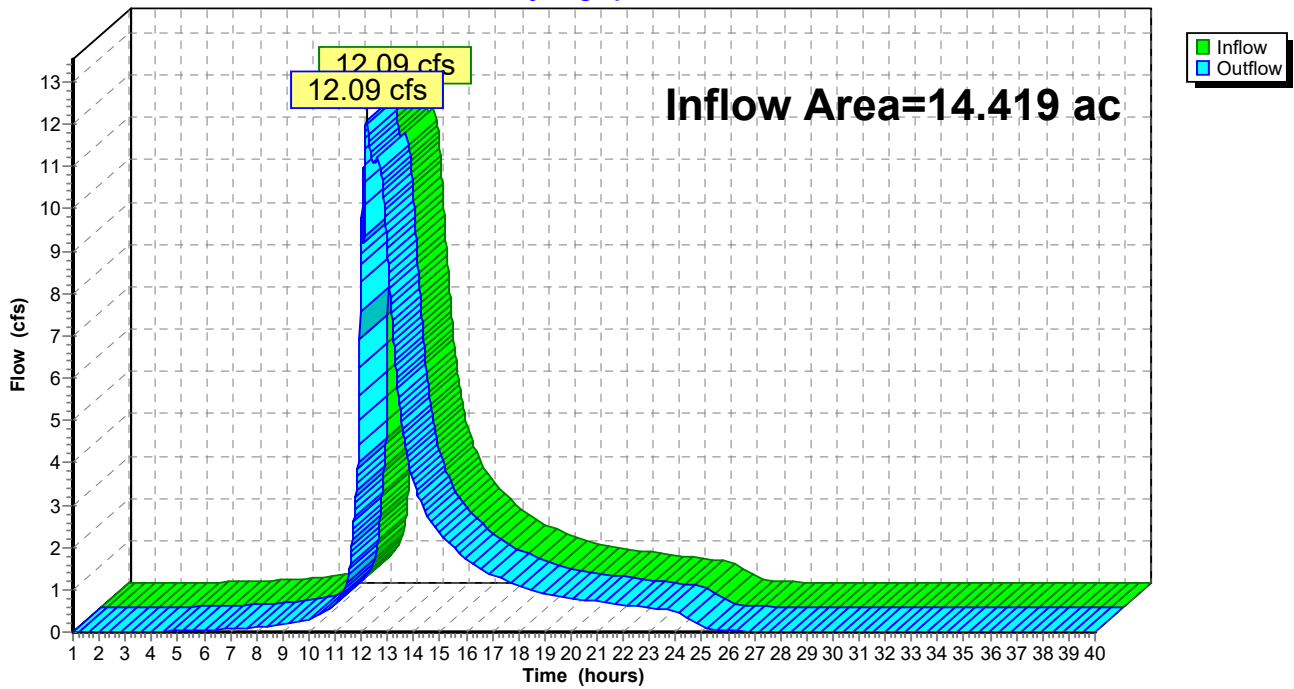
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 14.419 ac, 10.03% Impervious, Inflow Depth > 2.21" for 10 Year event  
Inflow = 12.09 cfs @ 12.20 hrs, Volume= 2.650 af  
Outflow = 12.09 cfs @ 12.20 hrs, Volume= 2.650 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs

### Reach DPWest: Design Point

Hydrograph



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**Summary for Pond 1: one**

Inflow Area = 0.654 ac, 78.95% Impervious, Inflow Depth = 3.80" for 10 Year event  
 Inflow = 3.35 cfs @ 12.00 hrs, Volume= 0.207 af  
 Outflow = 2.92 cfs @ 12.03 hrs, Volume= 0.207 af, Atten= 13%, Lag= 1.5 min  
 Discarded = 0.06 cfs @ 12.03 hrs, Volume= 0.004 af  
 Primary = 2.86 cfs @ 12.03 hrs, Volume= 0.203 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,713.56' @ 12.03 hrs Surf.Area= 0.056 ac Storage= 0.001 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.1 min ( 775.6 - 775.5 )

Volume	Invert	Avail.Storage	Storage Description
#1A	1,713.50'	0.032 af	<b>16.37'W x 150.00'L x 1.48'H Field A</b> 0.083 af Overall - 0.004 af Embedded = 0.079 af x 40.0% Voids
#2A	1,713.83'	0.004 af	<b>CPP single-wall 4" x 84</b> Inside #1 Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf 12 Rows of 7 Chambers
		0.036 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	1,712.00'	<b>6.0" Round Culvert X 4.00</b> L= 150.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,712.00' / 1,711.00' S= 0.0067 '/' Cc= 0.900 n= 0.011, Flow Area= 0.20 sf
#2	Discarded	1,713.50'	<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 2.00'

**Discarded OutFlow** Max=0.06 cfs @ 12.03 hrs HW=1,713.56' (Free Discharge)  
 ↳ **2=Exfiltration** ( Controls 0.06 cfs)

**Primary OutFlow** Max=2.86 cfs @ 12.03 hrs HW=1,713.56' (Free Discharge)  
 ↳ **1=Culvert** (Barrel Controls 2.86 cfs @ 3.64 fps)



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**Pond 1: one - Chamber Wizard Field A**

**Chamber Model = CPP single-wall 4" (Single-wall corrugated HDPE pipe)**

Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf

Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf

4.7" Wide + 12.0" Spacing = 16.7" C-C Row Spacing

7 Chambers/Row x 20.00' Long = 140.00' Row Length +60.0" End Stone x 2 = 150.00' Base Length

12 Rows x 4.7" Wide + 12.0" Spacing x 11 + 4.0" Side Stone x 2 = 16.37' Base Width

4.0" Base + 4.7" Chamber Height + 9.0" Cover = 1.48' Field Height

84 Chambers x 2.2 cf = 182.8 cf Chamber Storage

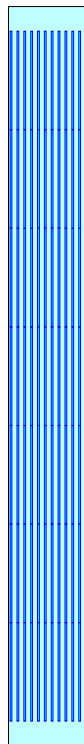
3,621.1 cf Field - 182.8 cf Chambers = 3,438.3 cf Stone x 40.0% Voids = 1,375.3 cf Stone Storage

Chamber Storage + Stone Storage = 1,558.1 cf = 0.036 af

Overall Storage Efficiency = 43.0%

Overall System Size = 150.00' x 16.37' x 1.48'

84 Chambers  
134.1 cy Field  
127.3 cy Stone



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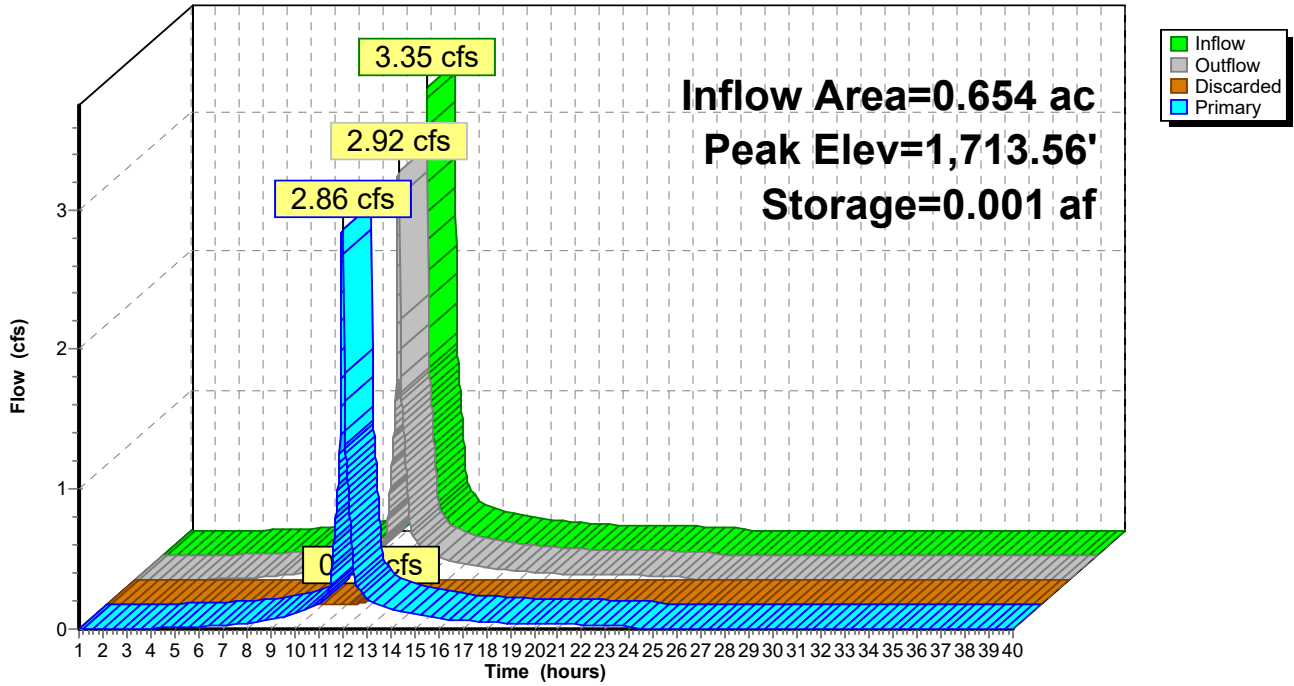
Type III 24-hr 10 Year Rainfall=4.59"

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## Pond 1: one

Hydrograph



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### Summary for Pond 2: Two

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area = 0.916 ac, 78.95% Impervious, Inflow Depth = 3.80" for 10 Year event  
Inflow = 4.68 cfs @ 12.00 hrs, Volume= 0.290 af  
Outflow = 3.76 cfs @ 12.04 hrs, Volume= 0.290 af, Atten= 20%, Lag= 2.2 min  
Discarded = 0.05 cfs @ 12.04 hrs, Volume= 0.004 af  
Primary = 3.71 cfs @ 12.04 hrs, Volume= 0.286 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Peak Elev= 1,713.68' @ 12.04 hrs Surf.Area= 0.051 ac Storage= 0.004 af

Plug-Flow detention time= 0.1 min calculated for 0.290 af (100% of inflow)  
Center-of-Mass det. time= 0.1 min ( 775.6 - 775.5 )

Volume	Invert	Avail.Storage	Storage Description
#1A	1,713.50'	0.029 af	<b>21.93'W x 102.00'L x 1.48'H Field A</b> 0.076 af Overall - 0.004 af Embedded = 0.072 af x 40.0% Voids
#2A	1,713.83'	0.004 af	<b>CPP single-wall 4" x 80</b> Inside #1 Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf 16 Rows of 5 Chambers
		0.033 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	1,712.00'	<b>12.0" Round Culvert</b> L= 150.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,712.00' / 1,711.00' S= 0.0067 '/' Cc= 0.900 n= 0.011, Flow Area= 0.79 sf
#2	Discarded	1,713.50'	<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 2.00'

**Discarded OutFlow** Max=0.05 cfs @ 12.04 hrs HW=1,713.68' (Free Discharge)  
↑**2=Exfiltration** ( Controls 0.05 cfs)

**Primary OutFlow** Max=3.70 cfs @ 12.04 hrs HW=1,713.68' (Free Discharge)  
↑**1=Culvert** (Barrel Controls 3.70 cfs @ 4.72 fps)

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**Pond 2: Two - Chamber Wizard Field A**

**Chamber Model = CPP single-wall 4" (Single-wall corrugated HDPE pipe)**

Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf

Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf

4.7" Wide + 12.0" Spacing = 16.7" C-C Row Spacing

5 Chambers/Row x 20.00' Long = 100.00' Row Length +12.0" End Stone x 2 = 102.00' Base Length

16 Rows x 4.7" Wide + 12.0" Spacing x 15 + 4.0" Side Stone x 2 = 21.93' Base Width

4.0" Base + 4.7" Chamber Height + 9.0" Cover = 1.48' Field Height

80 Chambers x 2.2 cf = 174.1 cf Chamber Storage

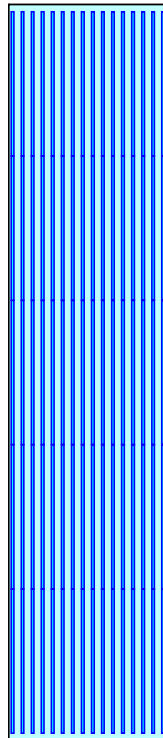
3,299.9 cf Field - 174.1 cf Chambers = 3,125.8 cf Stone x 40.0% Voids = 1,250.3 cf Stone Storage

Chamber Storage + Stone Storage = 1,424.4 cf = 0.033 af

Overall Storage Efficiency = 43.2%

Overall System Size = 102.00' x 21.93' x 1.48'

80 Chambers  
122.2 cy Field  
115.8 cy Stone



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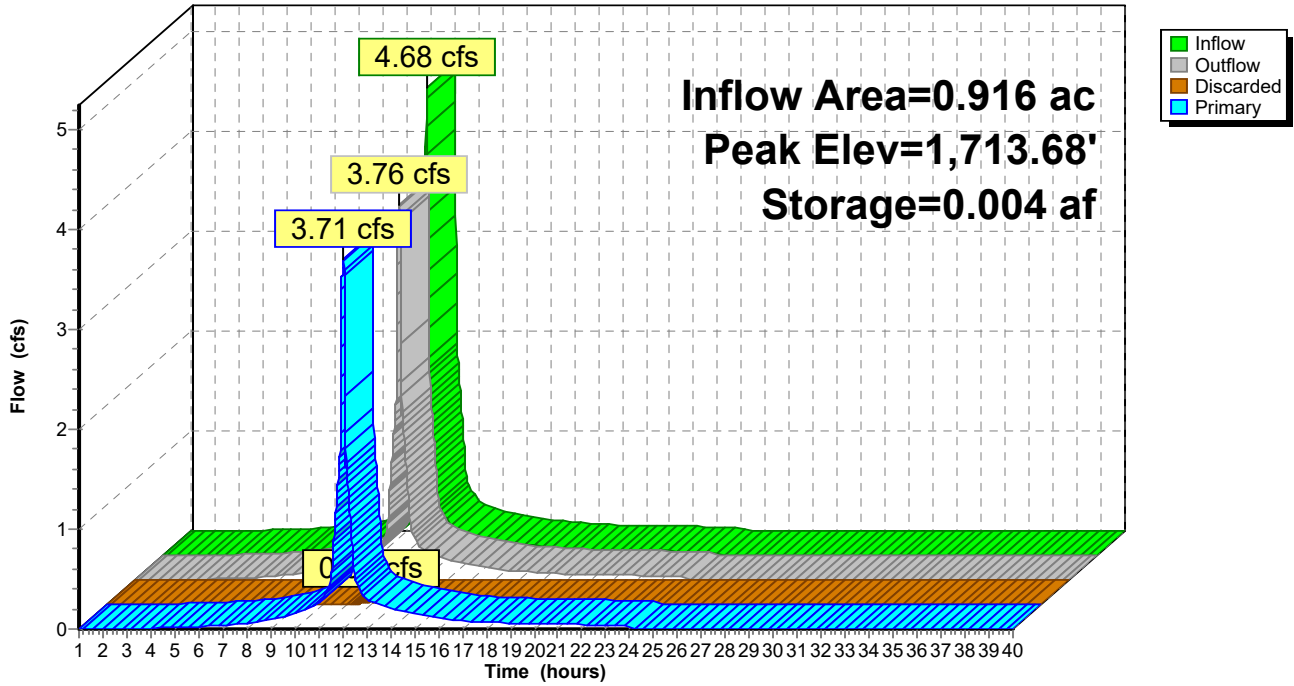
Type III 24-hr 10 Year Rainfall=4.59"

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## Pond 2: Two

Hydrograph



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**Summary for Pond 3: three**

Inflow Area = 1.047 ac, 78.95% Impervious, Inflow Depth = 3.80" for 10 Year event  
 Inflow = 5.35 cfs @ 12.00 hrs, Volume= 0.331 af  
 Outflow = 4.42 cfs @ 12.04 hrs, Volume= 0.331 af, Atten= 17%, Lag= 2.0 min  
 Discarded = 0.08 cfs @ 12.04 hrs, Volume= 0.006 af  
 Primary = 4.34 cfs @ 12.04 hrs, Volume= 0.325 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,713.61' @ 12.04 hrs Surf.Area= 0.076 ac Storage= 0.003 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.1 min ( 775.6 - 775.5 )

Volume	Invert	Avail.Storage	Storage Description
#1A	1,713.50'	0.042 af	<b>21.93'W x 150.00'L x 1.48'H Field A</b> 0.111 af Overall - 0.006 af Embedded = 0.106 af x 40.0% Voids
#2A	1,713.83'	0.006 af	<b>CPP single-wall 4" x 112 Inside #1</b> Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf 16 Rows of 7 Chambers
		0.048 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	1,712.00'	<b>6.0" Round Culvert X 6.00</b> L= 150.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,712.00' / 1,711.00' S= 0.0067 '/' Cc= 0.900 n= 0.011, Flow Area= 0.20 sf
#2	Discarded	1,713.50'	<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 2.00'

**Discarded OutFlow** Max=0.08 cfs @ 12.04 hrs HW=1,713.61' (Free Discharge)  
 ↳ **2=Exfiltration** ( Controls 0.08 cfs)

**Primary OutFlow** Max=4.34 cfs @ 12.04 hrs HW=1,713.61' (Free Discharge)  
 ↳ **1=Culvert** (Barrel Controls 4.34 cfs @ 3.69 fps)

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### Pond 3: three - Chamber Wizard Field A

#### Chamber Model = CPP single-wall 4" (Single-wall corrugated HDPE pipe)

Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf

Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf

4.7" Wide + 12.0" Spacing = 16.7" C-C Row Spacing

7 Chambers/Row x 20.00' Long = 140.00' Row Length +60.0" End Stone x 2 = 150.00' Base Length

16 Rows x 4.7" Wide + 12.0" Spacing x 15 + 4.0" Side Stone x 2 = 21.93' Base Width

4.0" Base + 4.7" Chamber Height + 9.0" Cover = 1.48' Field Height

112 Chambers x 2.2 cf = 243.8 cf Chamber Storage

4,852.7 cf Field - 243.8 cf Chambers = 4,609.0 cf Stone x 40.0% Voids = 1,843.6 cf Stone Storage

Chamber Storage + Stone Storage = 2,087.4 cf = 0.048 af

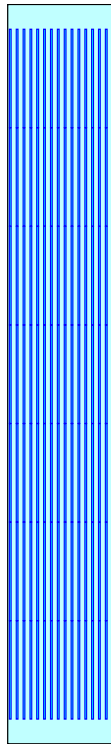
Overall Storage Efficiency = 43.0%

Overall System Size = 150.00' x 21.93' x 1.48'

112 Chambers

179.7 cy Field

170.7 cy Stone



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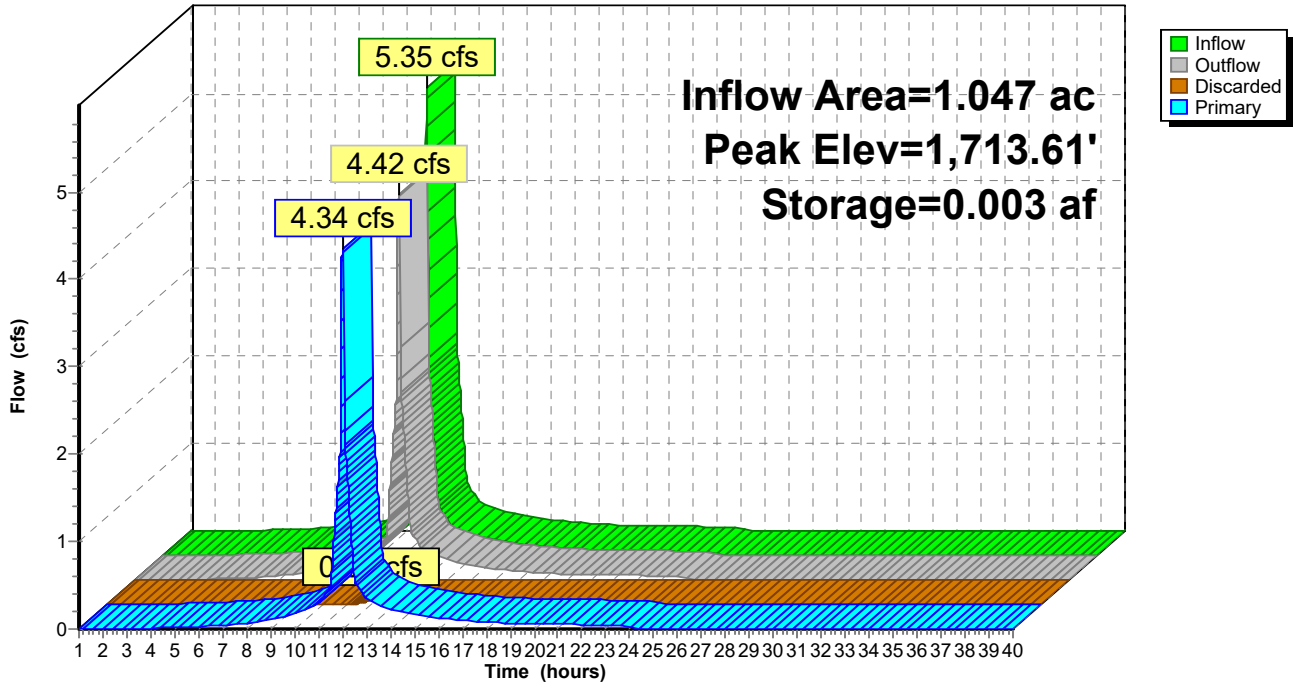
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## Pond 3: three

Hydrograph





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**Summary for Pond DP up: Dry Basin**

Inflow Area = 1.876 ac, 0.00% Impervious, Inflow Depth = 1.96" for 10 Year event  
 Inflow = 5.22 cfs @ 12.00 hrs, Volume= 0.307 af  
 Outflow = 0.72 cfs @ 12.49 hrs, Volume= 0.214 af, Atten= 86%, Lag= 29.2 min  
 Primary = 0.72 cfs @ 12.49 hrs, Volume= 0.214 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,731.82' @ 12.49 hrs Surf.Area= 0.087 ac Storage= 0.139 af

Plug-Flow detention time= 235.0 min calculated for 0.214 af (70% of inflow)  
 Center-of-Mass det. time= 134.1 min ( 972.9 - 838.8 )

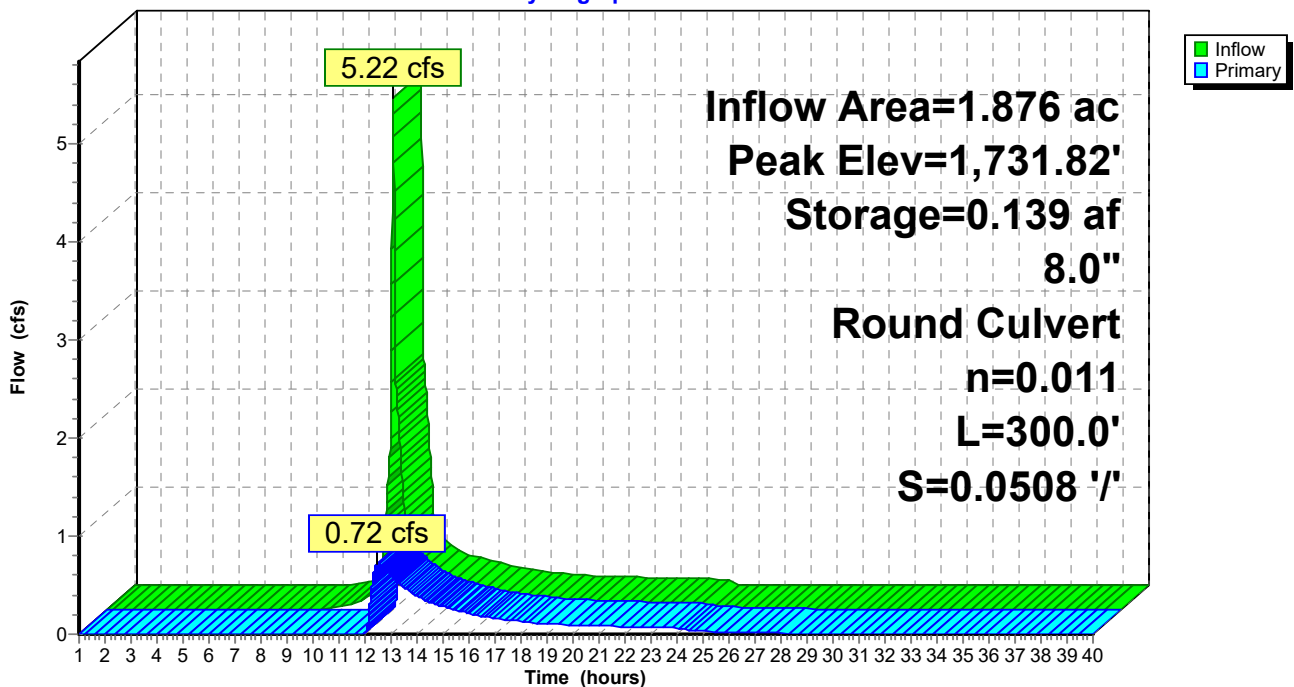
Volume	Invert	Avail.Storage	Storage Description
#1	1,730.00'	0.357 af	<b>45.00'W x 65.00'L x 4.00'H Hillside pond Z=2.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,731.25'	<b>8.0" Round Culvert</b> L= 300.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 1,731.25' / 1,716.00' S= 0.0508 '/ Cc= 0.900 n= 0.011, Flow Area= 0.35 sf

**Primary OutFlow** Max=0.72 cfs @ 12.49 hrs HW=1,731.82' (Free Discharge)  
 ←1=Culvert (Inlet Controls 0.72 cfs @ 2.26 fps)

**Pond DP up: Dry Basin**

Hydrograph



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**Summary for Pond DPe: Pond**

[79] Warning: Submerged Pond FBe Primary device # 1 INLET by 0.28'

Inflow Area = 7.028 ac, 1.63% Impervious, Inflow Depth = 2.04" for 10 Year event  
 Inflow = 10.47 cfs @ 12.35 hrs, Volume= 1.196 af  
 Outflow = 10.45 cfs @ 12.37 hrs, Volume= 1.196 af, Atten= 0%, Lag= 0.8 min  
 Primary = 10.45 cfs @ 12.37 hrs, Volume= 1.196 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,708.28' @ 12.37 hrs Surf.Area= 0.028 ac Storage= 0.040 af

Plug-Flow detention time= 5.2 min calculated for 1.196 af (100% of inflow)  
 Center-of-Mass det. time= 5.2 min ( 865.0 - 859.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,706.00'	0.105 af	<b>10.00'W x 38.00'L x 4.00'H Pond Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,706.00'	<b>15.0" Round Culvert</b> L= 35.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,706.00' / 1,706.00' S= 0.0000 1' Cc= 0.900 n= 0.011, Flow Area= 1.23 sf
#2	Primary	1,708.00'	<b>10.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Primary OutFlow** Max=10.44 cfs @ 12.37 hrs HW=1,708.28' (Free Discharge)

- 1=Culvert (Barrel Controls 6.92 cfs @ 5.64 fps)
- 2=Broad-Crested Rectangular Weir (Weir Controls 3.53 cfs @ 1.27 fps)

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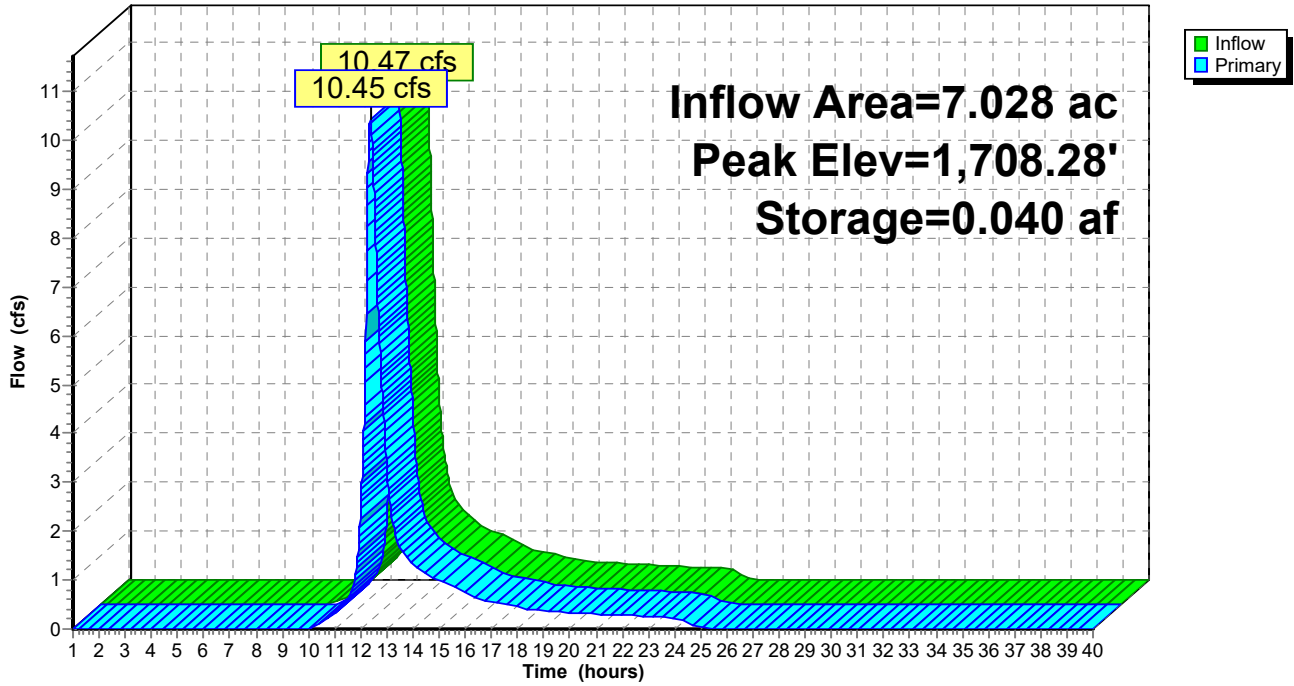
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## Pond DPe: Pond

Hydrograph



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**Summary for Pond DPw: Dry pond**

[81] Warning: Exceeded Pond FBw by 0.13' @ 22.67 hrs

Inflow Area = 2.826 ac, 3.66% Impervious, Inflow Depth = 2.81" for 10 Year event  
 Inflow = 7.43 cfs @ 12.13 hrs, Volume= 0.661 af  
 Outflow = 6.71 cfs @ 12.19 hrs, Volume= 0.595 af, Atten= 10%, Lag= 3.8 min  
 Primary = 6.71 cfs @ 12.19 hrs, Volume= 0.595 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,703.73' @ 12.19 hrs Surf.Area= 0.055 ac Storage= 0.123 af

Plug-Flow detention time= 86.0 min calculated for 0.595 af (90% of inflow)  
 Center-of-Mass det. time= 38.1 min ( 885.8 - 847.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,700.00'	0.153 af	<b>16.00'W x 40.00'L x 4.25'H Prismatic Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,702.50'	<b>15.0" Round Culvert</b> L= 40.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,702.50' / 1,699.00' S= 0.0875 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf
#2	Primary	1,700.00'	<b>4.0" Round Culvert X 0.00</b> L= 30.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,700.00' / 1,696.00' S= 0.1333 '/ Cc= 0.900 n= 0.011, Flow Area= 0.09 sf
#3	Primary	1,703.50'	<b>8.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Primary OutFlow** Max=6.71 cfs @ 12.19 hrs HW=1,703.73' (Free Discharge)

- 1=Culvert (Inlet Controls 4.62 cfs @ 3.78 fps)
- 2=Culvert ( Controls 0.00 cfs)
- 3=Broad-Crested Rectangular Weir (Weir Controls 2.09 cfs @ 1.13 fps)

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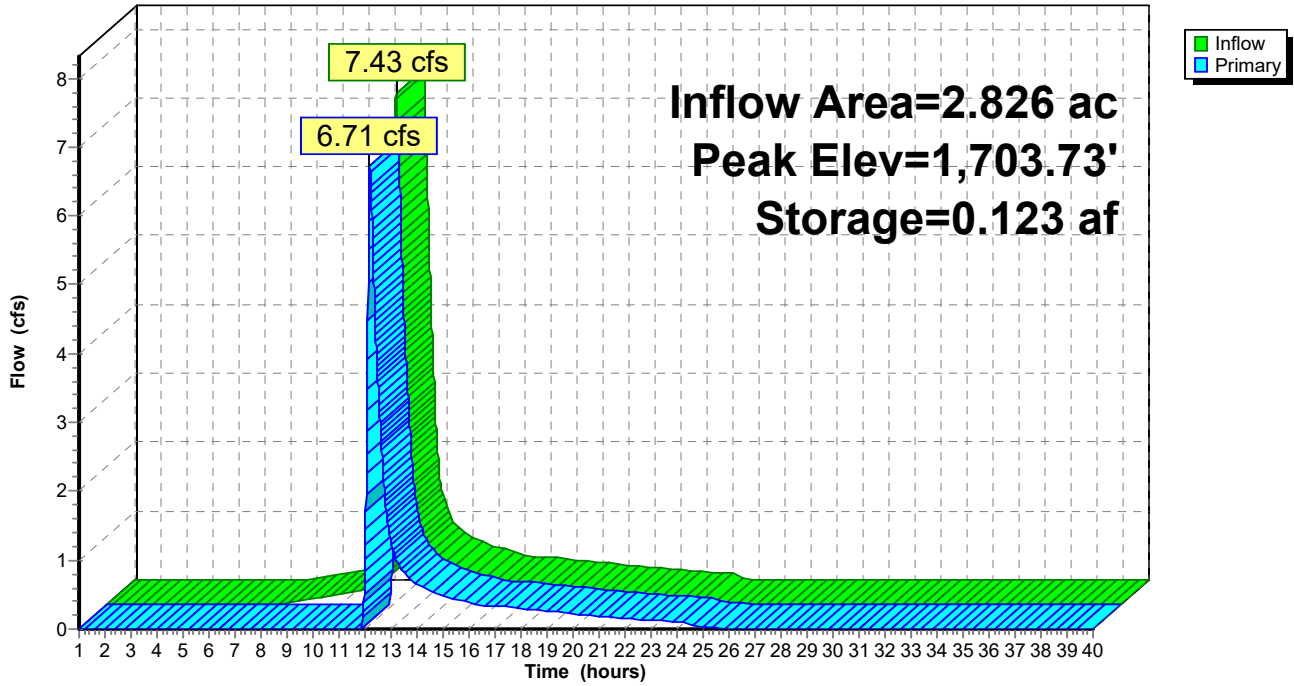
Type III 24-hr 10 Year Rainfall=4.59"

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## Pond DPw: Dry pond

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**Summary for Pond FBe: ForeBay**

Inflow Area = 7.028 ac, 1.63% Impervious, Inflow Depth = 2.04" for 10 Year event  
 Inflow = 10.49 cfs @ 12.34 hrs, Volume= 1.196 af  
 Outflow = 10.47 cfs @ 12.35 hrs, Volume= 1.196 af, Atten= 0%, Lag= 0.8 min  
 Primary = 10.47 cfs @ 12.35 hrs, Volume= 1.196 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,709.14' @ 12.35 hrs Surf.Area= 0.018 ac Storage= 0.015 af

Plug-Flow detention time= 2.1 min calculated for 1.196 af (100% of inflow)  
 Center-of-Mass det. time= 2.0 min ( 859.8 - 857.8 )

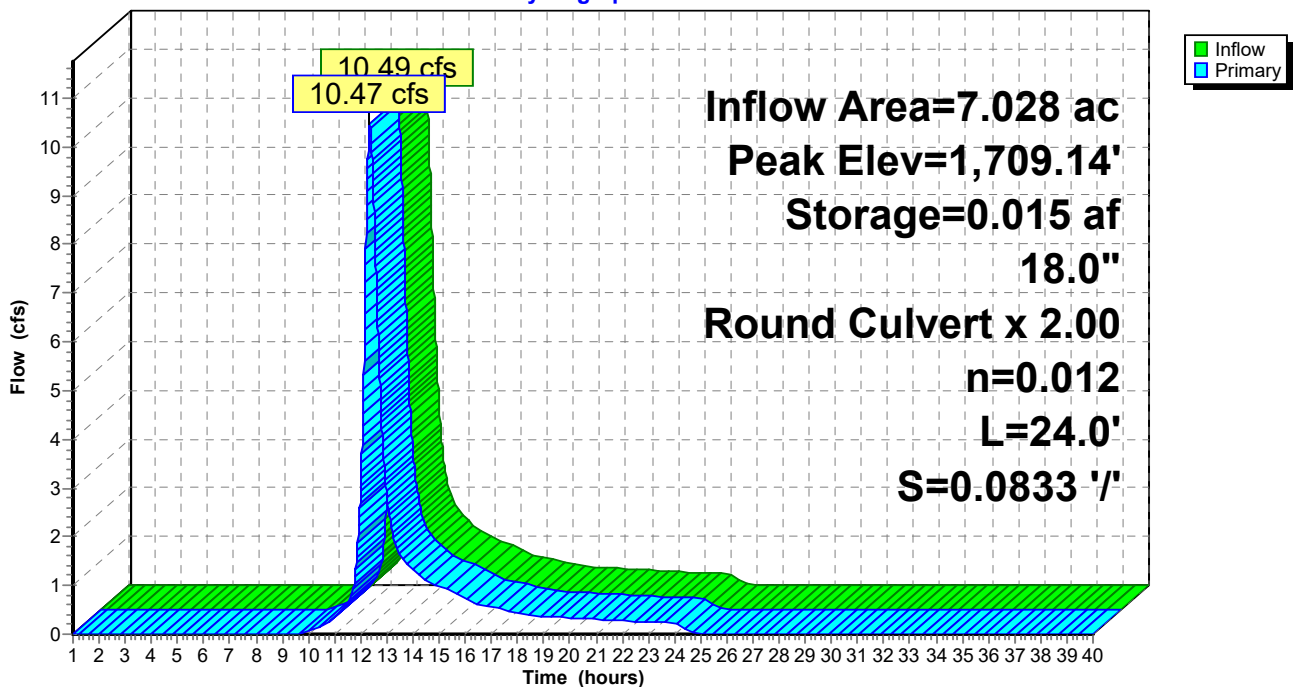
Volume	Invert	Avail.Storage	Storage Description
#1	1,708.00'	0.109 af	<b>10.00'W x 40.00'L x 4.00'H Prismaoid Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,708.00'	<b>18.0" Round Culvert X 2.00</b> L= 24.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,708.00' / 1,706.00' S= 0.0833 '/ Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

**Primary OutFlow** Max=10.47 cfs @ 12.35 hrs HW=1,709.14' (Free Discharge)  
 ←1=Culvert (Inlet Controls 10.47 cfs @ 3.63 fps)

**Pond FBe: ForeBay**

Hydrograph



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**Summary for Pond FBw: Forebay**

[62] Hint: Exceeded Reach 8R OUTLET depth by 2.41' @ 12.14 hrs

Inflow Area = 2.826 ac, 3.66% Impervious, Inflow Depth = 2.81" for 10 Year event  
 Inflow = 10.06 cfs @ 12.06 hrs, Volume= 0.661 af  
 Outflow = 7.43 cfs @ 12.13 hrs, Volume= 0.661 af, Atten= 26%, Lag= 3.9 min  
 Primary = 7.43 cfs @ 12.13 hrs, Volume= 0.661 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,704.91' @ 12.13 hrs Surf.Area= 0.059 ac Storage= 0.100 af

Plug-Flow detention time= 29.0 min calculated for 0.661 af (100% of inflow)  
 Center-of-Mass det. time= 29.0 min ( 847.6 - 818.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,702.50'	0.237 af	<b>16.00'W x 70.00'L x 4.25'H Prismatic Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,703.50'	<b>18.0" Round Culvert</b> L= 40.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,703.50' / 1,703.00' S= 0.0125 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf
#2	Primary	1,702.25'	<b>4.0" Round Culvert</b> L= 35.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 1,702.25' / 1,702.00' S= 0.0071 '/' Cc= 0.900 n= 0.011, Flow Area= 0.09 sf

**Primary OutFlow** Max=7.44 cfs @ 12.13 hrs HW=1,704.90' (Free Discharge)

- 1=Culvert (Inlet Controls 6.94 cfs @ 4.03 fps)
- 2=Culvert (Barrel Controls 0.50 cfs @ 5.70 fps)

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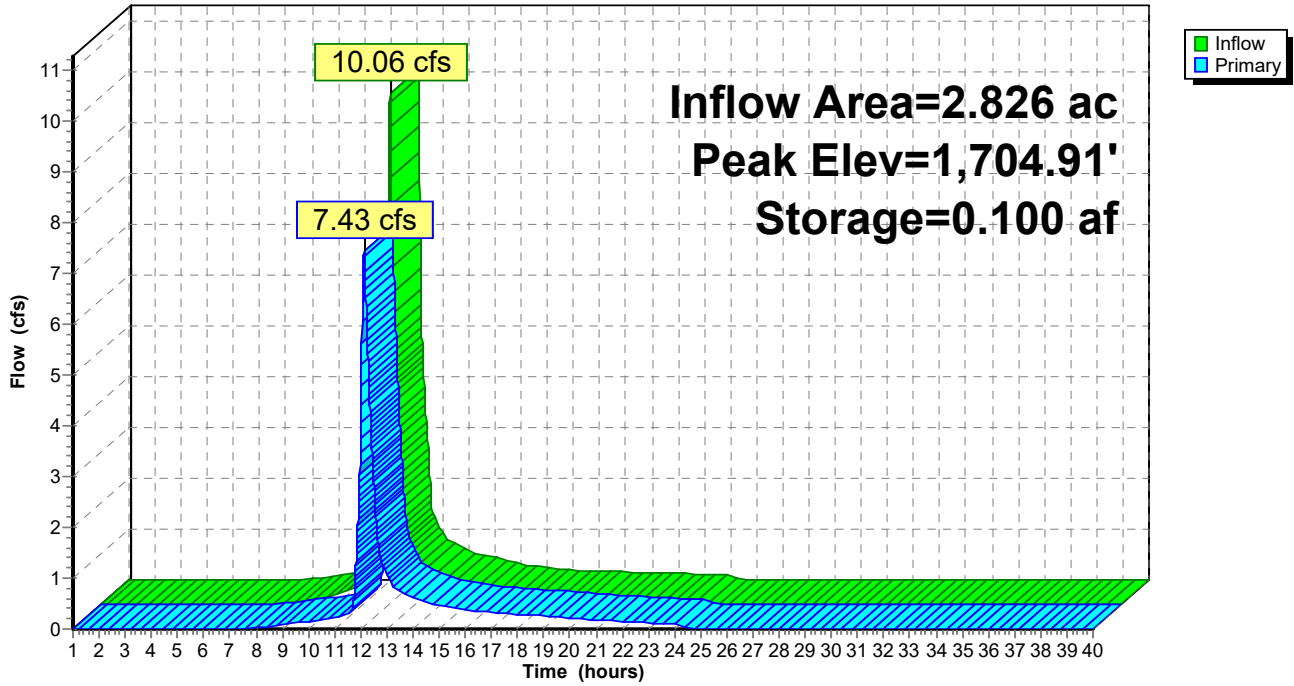
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**Pond FBw: Forebay**

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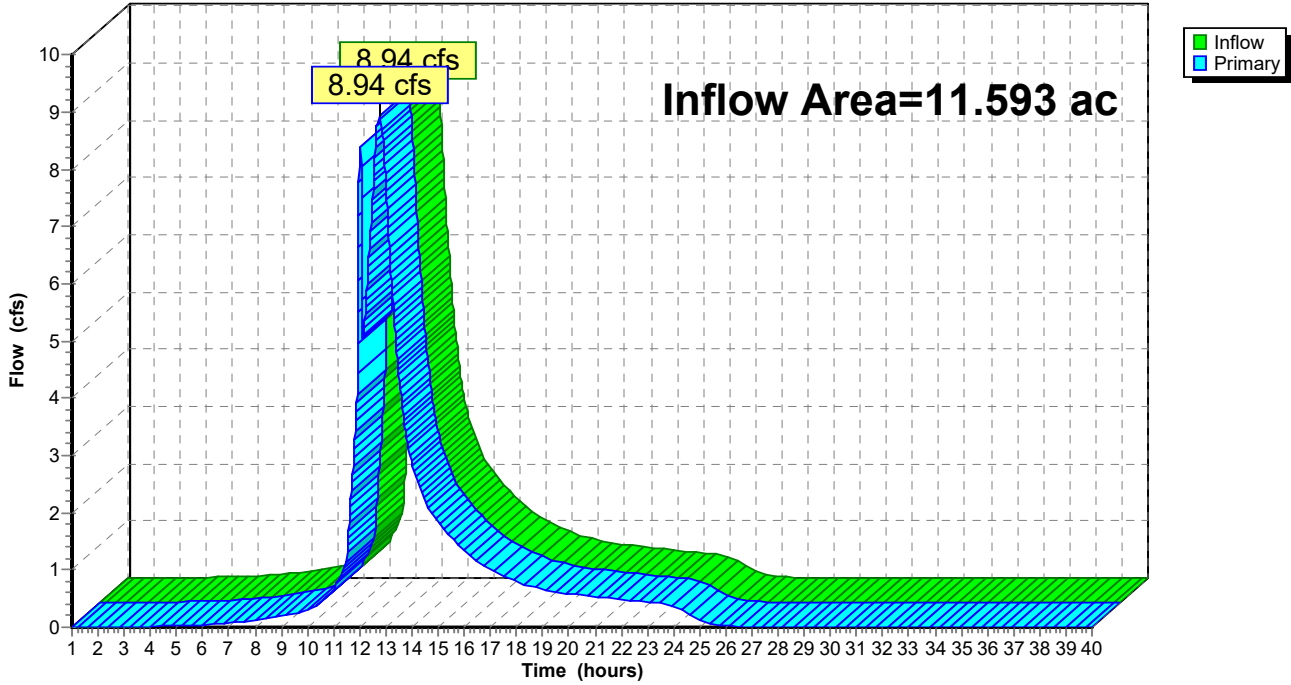
**Summary for Link 7L: wetland**

Inflow Area = 11.593 ac, 11.58% Impervious, Inflow Depth > 2.13" for 10 Year event  
Inflow = 8.94 cfs @ 12.77 hrs, Volume= 2.054 af  
Primary = 8.94 cfs @ 12.77 hrs, Volume= 2.054 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs

**Link 7L: wetland**

Hydrograph



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Time span=1.00-40.00 hrs, dt=0.01 hrs, 3901 points  
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN  
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

**Subcatchment 1S: Upgradient forest** Runoff Area=349,185 sf 0.00% Impervious Runoff Depth=4.98"  
Flow Length=1,729' Tc=54.5 min CN=73 Runoff=19.45 cfs 3.326 af

**Subcatchment 2S: Easterly Watershed** Runoff Area=306,150 sf 1.63% Impervious Runoff Depth=5.10"  
Flow Length=1,201' Tc=23.3 min CN=74 Runoff=26.49 cfs 2.985 af

**Subcatchment 4S: upgradient forest by-pass** Runoff Area=81,700 sf 0.00% Impervious Runoff Depth=4.98"  
Tc=0.0 min CN=73 Runoff=13.38 cfs 0.778 af

**Subcatchment 6S: development area** Runoff Area=123,100 sf 3.66% Impervious Runoff Depth=6.16"  
Tc=0.0 min CN=83 Runoff=24.24 cfs 1.450 af

**Subcatchment I: Greenhouse 1** Runoff Area=28,500 sf 78.95% Impervious Runoff Depth=7.35"  
Flow Length=25' Slope=0.2000 '/' Tc=0.2 min CN=93 Runoff=6.24 cfs 0.401 af

**Subcatchment II: Greenhouse 2** Runoff Area=39,900 sf 78.95% Impervious Runoff Depth=7.35"  
Flow Length=25' Slope=0.2000 '/' Tc=0.2 min CN=93 Runoff=8.73 cfs 0.561 af

**Subcatchment III: Greenhouse 3** Runoff Area=45,600 sf 78.95% Impervious Runoff Depth=7.35"  
Flow Length=25' Slope=0.2000 '/' Tc=0.2 min CN=93 Runoff=9.98 cfs 0.641 af

**Reach 8R: Swale** Avg. Flow Depth=1.02' Max Vel=5.38 fps Inflow=24.24 cfs 1.450 af  
n=0.035 L=600.0' S=0.0300 '/' Capacity=93.83 cfs Outflow=22.04 cfs 1.450 af

**Reach DPEast: Design Point** Inflow=28.72 cfs 3.538 af  
Outflow=28.72 cfs 3.538 af

**Reach DPWest: Design Point** Inflow=31.15 cfs 6.418 af  
Outflow=31.15 cfs 6.418 af

**Pond 1: one** Peak Elev=1,714.36' Storage=0.022 af Inflow=6.24 cfs 0.401 af  
Discarded=0.06 cfs 0.008 af Primary=3.37 cfs 0.393 af Outflow=3.43 cfs 0.400 af

**Pond 2: Two** Peak Elev=1,714.95' Storage=0.032 af Inflow=8.73 cfs 0.561 af  
Discarded=0.05 cfs 0.008 af Primary=4.91 cfs 0.553 af Outflow=4.96 cfs 0.561 af

**Pond 3: three** Peak Elev=1,714.64' Storage=0.038 af Inflow=9.98 cfs 0.641 af  
Discarded=0.08 cfs 0.011 af Primary=5.30 cfs 0.630 af Outflow=5.37 cfs 0.641 af

**Pond DP up: Dry Basin** Peak Elev=1,733.88' Storage=0.343 af Inflow=13.38 cfs 0.778 af  
8.0" Round Culvert n=0.011 L=300.0' S=0.0508 '/' Outflow=2.25 cfs 0.685 af

**Pond DPe: Pond** Peak Elev=1,708.74' Storage=0.054 af Inflow=25.50 cfs 2.985 af  
Outflow=25.47 cfs 2.985 af

**Pond DPw: Dry pond** Peak Elev=1,703.99' Storage=0.138 af Inflow=12.83 cfs 1.450 af  
Outflow=12.70 cfs 1.385 af

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**Pond FBe: ForeBay**

Peak Elev=1,711.00' Storage=0.066 af Inflow=26.49 cfs 2.985 af  
18.0" Round Culvert x 2.00 n=0.012 L=24.0' S=0.0833 '/' Outflow=25.50 cfs 2.985 af

**Pond FBw: Forebay**

Peak Elev=1,706.31' Storage=0.199 af Inflow=22.04 cfs 1.450 af  
Outflow=12.83 cfs 1.450 af

**Link 7L: wetland**

Inflow=22.97 cfs 5.033 af  
Primary=22.97 cfs 5.033 af

**Total Runoff Area = 22.363 ac Runoff Volume = 10.142 af Average Runoff Depth = 5.44"**  
**89.79% Pervious = 20.079 ac 10.21% Impervious = 2.284 ac**

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**Summary for Subcatchment 1S: Upgradient forest**

Runoff = 19.45 cfs @ 12.77 hrs, Volume= 3.326 af, Depth= 4.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 100 Year Rainfall=8.19"

Area (sf)	CN	Description
* 349,185	73	Woods, Fair, HSG C (fair condition C/D soil)
349,185		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
29.8	50	0.0100	0.03		<b>Sheet Flow, Sheet Flow</b> Woods: Dense underbrush n= 0.800 P2= 2.90"
1.9	138	0.0580	1.20		<b>Shallow Concentrated Flow, hillside segment 1</b> Woodland Kv= 5.0 fps
0.8	127	0.2519	2.51		<b>Shallow Concentrated Flow, Hillside segment 2</b> Woodland Kv= 5.0 fps
5.8	461	0.0694	1.32		<b>Shallow Concentrated Flow, Hillside Segment 3</b> Woodland Kv= 5.0 fps
4.9	258	0.0310	0.88		<b>Shallow Concentrated Flow, Hillside Segment 4</b> Woodland Kv= 5.0 fps
11.3	695	0.0417	1.02		<b>Shallow Concentrated Flow, Hillside Segment 5</b> Woodland Kv= 5.0 fps
54.5	1,729	Total			

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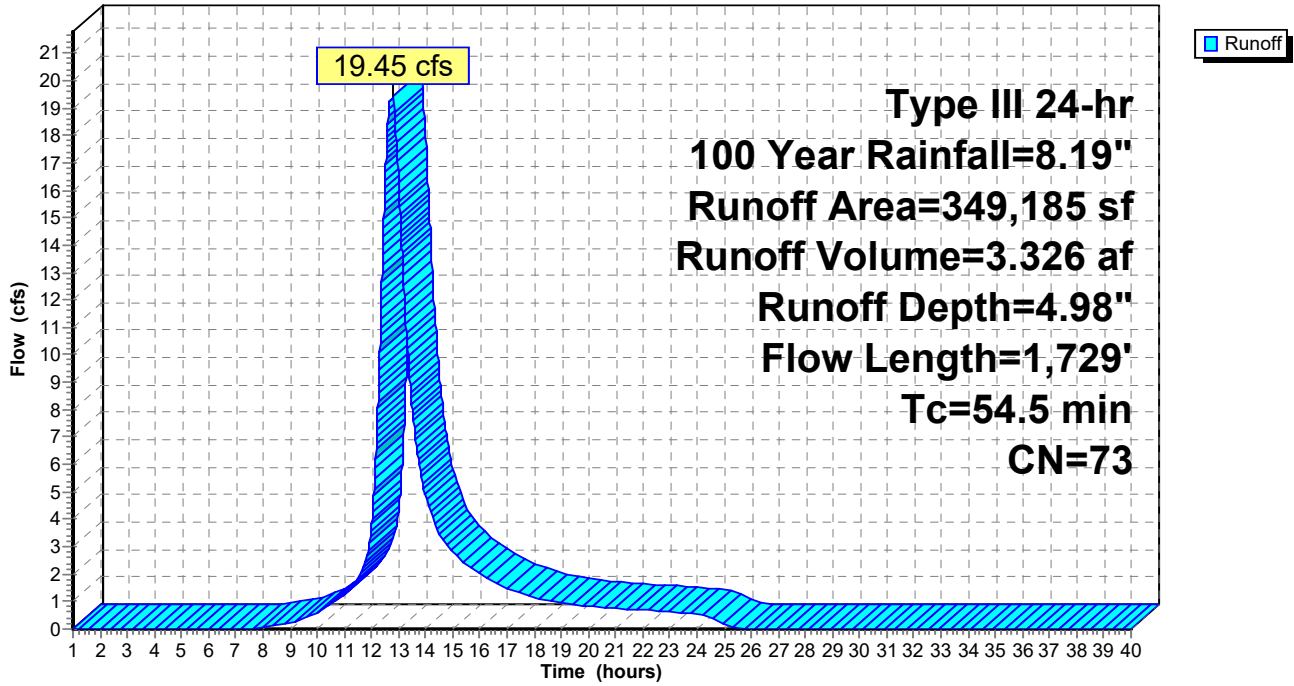
Type III 24-hr 100 Year Rainfall=8.19"

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**Subcatchment 1S: Upgradient forest**

Hydrograph



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**Summary for Subcatchment 2S: Easterly Watershed**

Runoff = 26.49 cfs @ 12.32 hrs, Volume= 2.985 af, Depth= 5.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 100 Year Rainfall=8.19"

Area (sf)	CN	Description
4,500	73	Woods, Fair, HSG C
5,000	98	Roofs, HSG C
36,500	96	Gravel surface, HSG C
10,000	74	>75% Grass cover, Good, HSG C
12,900	71	Meadow, non-grazed, HSG C
237,250	70	Woods, Good, HSG C
306,150	74	Weighted Average
301,150		98.37% Pervious Area
5,000		1.63% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.0	50	0.0200	0.06		<b>Sheet Flow, Sheet Flow</b> Woods: Light underbrush n= 0.400 P2= 2.90"
2.7	277	0.1155	1.70		<b>Shallow Concentrated Flow, Hillside Segment 1</b> Woodland Kv= 5.0 fps
6.9	524	0.0648	1.27		<b>Shallow Concentrated Flow, Hillside Segment #2</b> Woodland Kv= 5.0 fps
0.7	350	0.0500	7.85	23.56	<b>Channel Flow,</b> Area= 3.0 sf Perim= 8.0' r= 0.38' n= 0.022 Earth, clean & straight
23.3	1,201	Total			

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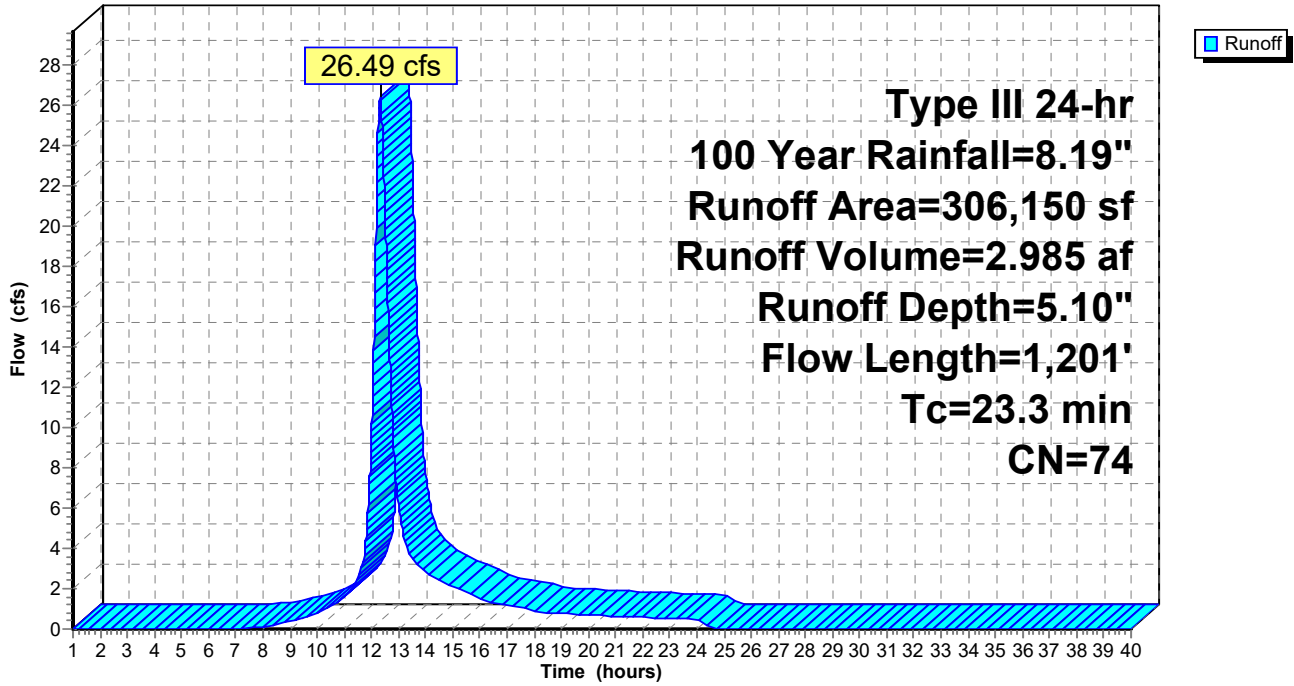
Type III 24-hr 100 Year Rainfall=8.19"

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**Subcatchment 2S: Easterly Watershed**

Hydrograph



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## Summary for Subcatchment 4S: upgradient forest by-pass

[46] Hint: Tc=0 (Instant runoff peak depends on dt)

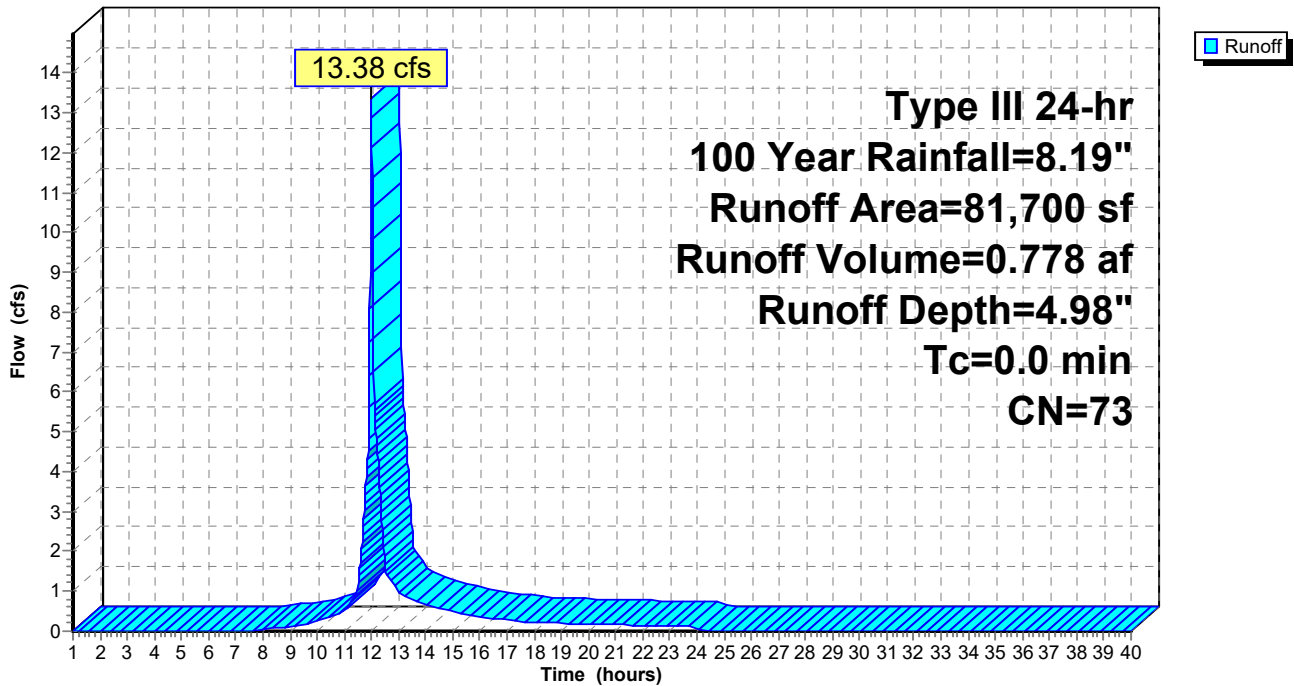
Runoff = 13.38 cfs @ 12.00 hrs, Volume= 0.778 af, Depth= 4.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100 Year Rainfall=8.19"

Area (sf)	CN	Description
76,000	73	Woods, Fair, HSG C
5,700	71	Meadow, non-grazed, HSG C
81,700	73	Weighted Average
81,700		100.00% Pervious Area

## Subcatchment 4S: upgradient forest by-pass

Hydrograph





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## Summary for Subcatchment 6S: development area

[46] Hint: Tc=0 (Instant runoff peak depends on dt)

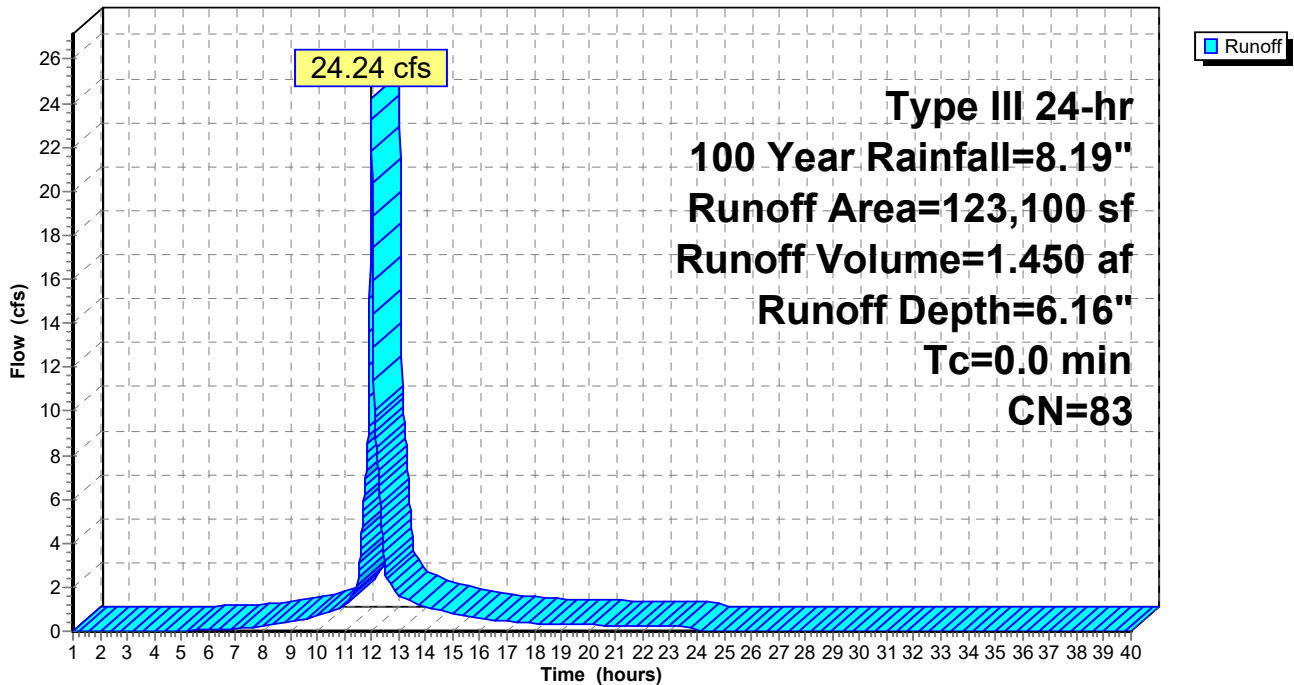
Runoff = 24.24 cfs @ 12.00 hrs, Volume= 1.450 af, Depth= 6.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100 Year Rainfall=8.19"

Area (sf)	CN	Description
54,800	96	Gravel surface, HSG C
* 4,500	98	rooftop
63,800	71	Meadow, non-grazed, HSG C
123,100	83	Weighted Average
118,600		96.34% Pervious Area
4,500		3.66% Impervious Area

## Subcatchment 6S: development area

Hydrograph



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### Summary for Subcatchment I: Greenhouse 1

[49] Hint:  $T_c < 2dt$  may require smaller dt

Runoff = 6.24 cfs @ 12.00 hrs, Volume= 0.401 af, Depth= 7.35"

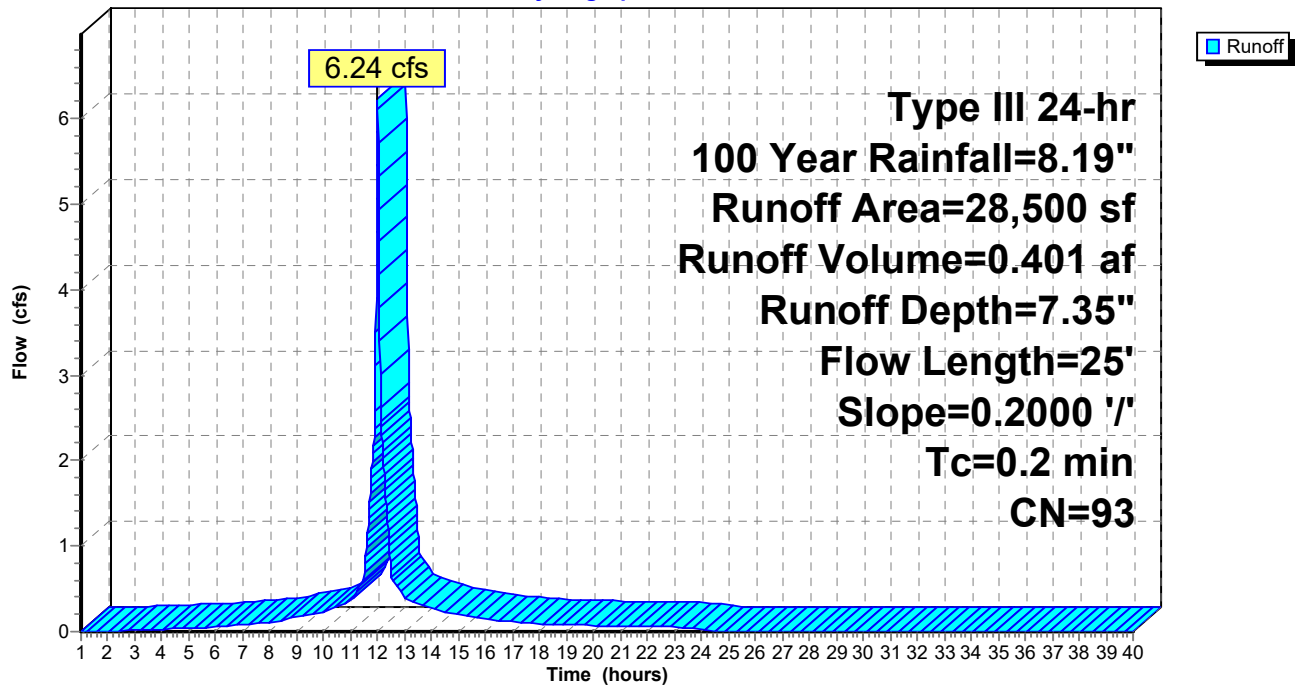
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100 Year Rainfall=8.19"

	Area (sf)	CN	Description
*	22,500	98	Roof
	6,000	74	>75% Grass cover, Good, HSG C
	28,500	93	Weighted Average
	6,000		21.05% Pervious Area
	22,500		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	25	0.2000	2.49		Sheet Flow, Roof Smooth surfaces n= 0.011 P2= 2.90"

### Subcatchment I: Greenhouse 1

Hydrograph



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## Summary for Subcatchment II: Greenhouse 2

[49] Hint:  $T_c < 2dt$  may require smaller dt

Runoff = 8.73 cfs @ 12.00 hrs, Volume= 0.561 af, Depth= 7.35"

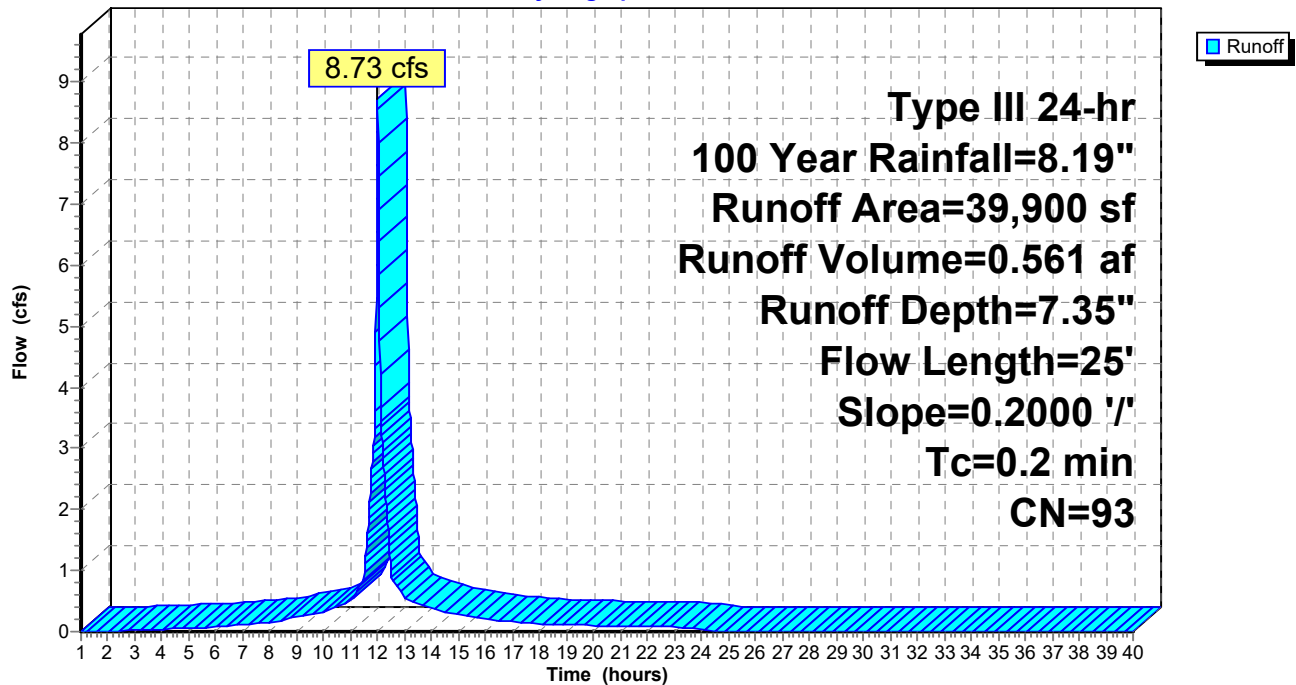
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100 Year Rainfall=8.19"

	Area (sf)	CN	Description
*	31,500	98	Roof
	8,400	74	>75% Grass cover, Good, HSG C
	39,900	93	Weighted Average
	8,400		21.05% Pervious Area
	31,500		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	25	0.2000	2.49		Sheet Flow, Roof Smooth surfaces n= 0.011 P2= 2.90"

## Subcatchment II: Greenhouse 2

Hydrograph



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**Summary for Subcatchment III: Greenhouse 3**

[49] Hint: Tc<2dt may require smaller dt

Runoff = 9.98 cfs @ 12.00 hrs, Volume= 0.641 af, Depth= 7.35"

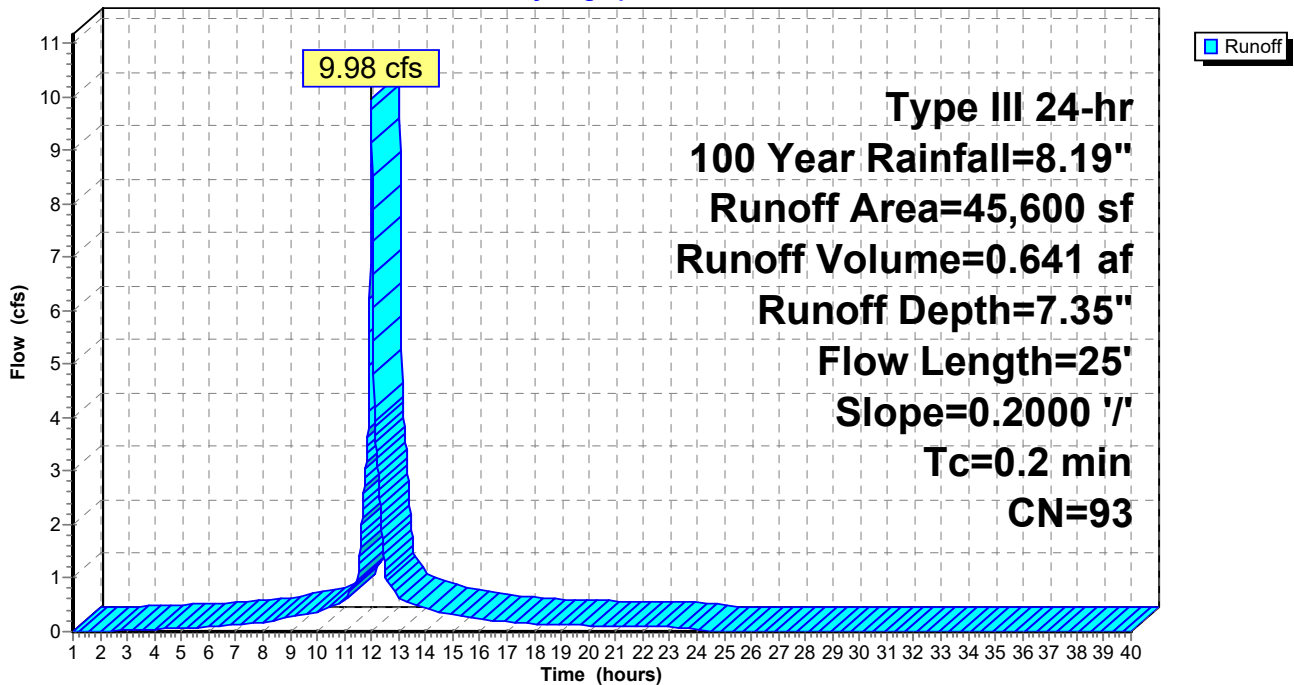
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 100 Year Rainfall=8.19"

	Area (sf)	CN	Description
*	36,000	98	Roof
	9,600	74	>75% Grass cover, Good, HSG C
	45,600	93	Weighted Average
	9,600		21.05% Pervious Area
	36,000		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.2	25	0.2000	2.49		Sheet Flow, Roof Smooth surfaces n= 0.011 P2= 2.90"

**Subcatchment III: Greenhouse 3**

Hydrograph



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### Summary for Reach 8R: Swale

Inflow Area = 2.826 ac, 3.66% Impervious, Inflow Depth = 6.16" for 100 Year event  
Inflow = 24.24 cfs @ 12.00 hrs, Volume= 1.450 af  
Outflow = 22.04 cfs @ 12.05 hrs, Volume= 1.450 af, Atten= 9%, Lag= 3.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
Max. Velocity= 5.38 fps, Min. Travel Time= 1.9 min  
Avg. Velocity = 1.58 fps, Avg. Travel Time= 6.3 min

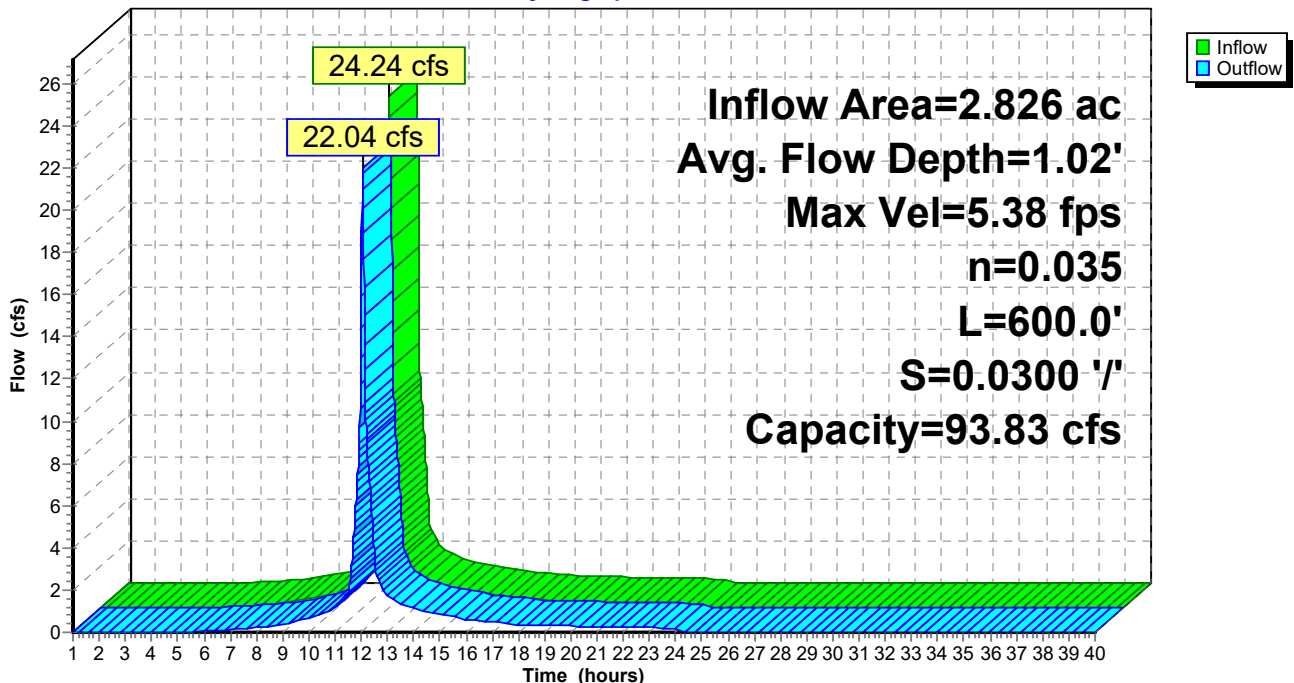
Peak Storage= 2,460 cf @ 12.02 hrs  
Average Depth at Peak Storage= 1.02'  
Bank-Full Depth= 2.00' Flow Area= 12.0 sf, Capacity= 93.83 cfs

2.00' x 2.00' deep channel, n= 0.035  
Side Slope Z-value= 2.0 '/' Top Width= 10.00'  
Length= 600.0' Slope= 0.0300 '/'  
Inlet Invert= 1,720.00', Outlet Invert= 1,702.00'



### Reach 8R: Swale

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## Summary for Reach DPEast: Design Point

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 7.944 ac, 10.55% Impervious, Inflow Depth = 5.34" for 100 Year event

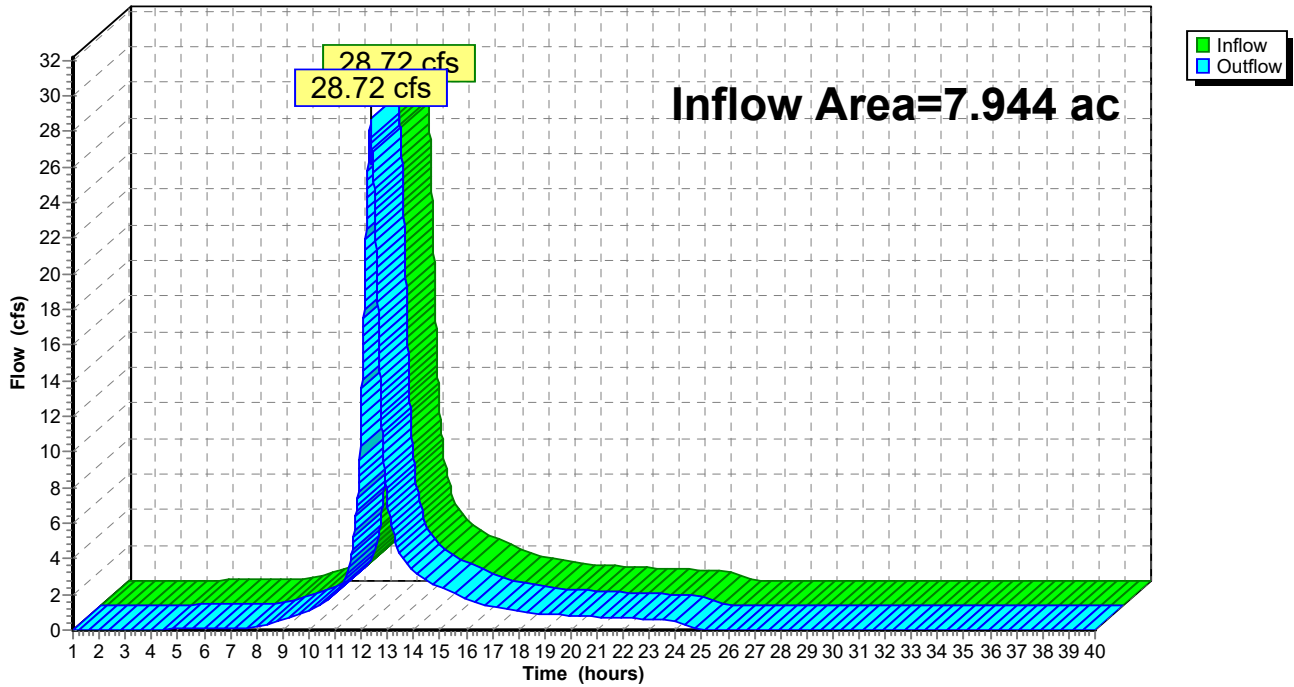
Inflow = 28.72 cfs @ 12.34 hrs, Volume= 3.538 af

Outflow = 28.72 cfs @ 12.34 hrs, Volume= 3.538 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs

### Reach DPEast: Design Point

Hydrograph



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## Summary for Reach DPWest: Design Point

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 14.419 ac, 10.03% Impervious, Inflow Depth = 5.34" for 100 Year event

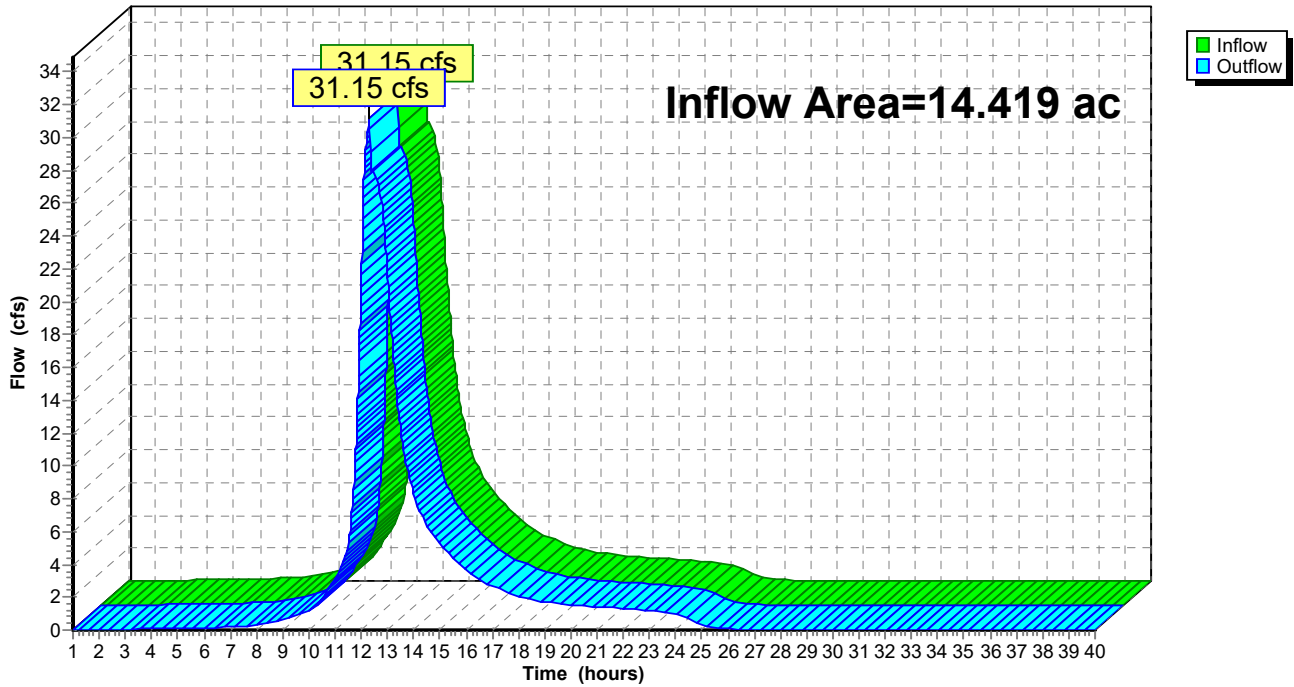
Inflow = 31.15 cfs @ 12.31 hrs, Volume= 6.418 af

Outflow = 31.15 cfs @ 12.31 hrs, Volume= 6.418 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs

### Reach DPWest: Design Point

Hydrograph



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**Summary for Pond 1: one**

Inflow Area = 0.654 ac, 78.95% Impervious, Inflow Depth = 7.35" for 100 Year event  
 Inflow = 6.24 cfs @ 12.00 hrs, Volume= 0.401 af  
 Outflow = 3.43 cfs @ 12.08 hrs, Volume= 0.400 af, Atten= 45%, Lag= 4.6 min  
 Discarded = 0.06 cfs @ 12.08 hrs, Volume= 0.008 af  
 Primary = 3.37 cfs @ 12.08 hrs, Volume= 0.393 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,714.36' @ 12.08 hrs Surf.Area= 0.056 ac Storage= 0.022 af

Plug-Flow detention time= 1.4 min calculated for 0.400 af (100% of inflow)  
 Center-of-Mass det. time= 0.8 min ( 760.2 - 759.4 )

Volume	Invert	Avail.Storage	Storage Description
#1A	1,713.50'	0.032 af	<b>16.37'W x 150.00'L x 1.48'H Field A</b> 0.083 af Overall - 0.004 af Embedded = 0.079 af x 40.0% Voids
#2A	1,713.83'	0.004 af	<b>CPP single-wall 4" x 84</b> Inside #1 Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf 12 Rows of 7 Chambers
		0.036 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	1,712.00'	<b>6.0" Round Culvert X 4.00</b> L= 150.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,712.00' / 1,711.00' S= 0.0067 '/' Cc= 0.900 n= 0.011, Flow Area= 0.20 sf
#2	Discarded	1,713.50'	<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 2.00'

**Discarded OutFlow** Max=0.06 cfs @ 12.08 hrs HW=1,714.36' (Free Discharge)  
 ↑**2=Exfiltration** ( Controls 0.06 cfs)

**Primary OutFlow** Max=3.37 cfs @ 12.08 hrs HW=1,714.36' (Free Discharge)  
 ↑**1=Culvert** (Barrel Controls 3.37 cfs @ 4.29 fps)



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**Pond 1: one - Chamber Wizard Field A**

**Chamber Model = CPP single-wall 4" (Single-wall corrugated HDPE pipe)**

Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf

Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf

4.7" Wide + 12.0" Spacing = 16.7" C-C Row Spacing

7 Chambers/Row x 20.00' Long = 140.00' Row Length +60.0" End Stone x 2 = 150.00' Base Length

12 Rows x 4.7" Wide + 12.0" Spacing x 11 + 4.0" Side Stone x 2 = 16.37' Base Width

4.0" Base + 4.7" Chamber Height + 9.0" Cover = 1.48' Field Height

84 Chambers x 2.2 cf = 182.8 cf Chamber Storage

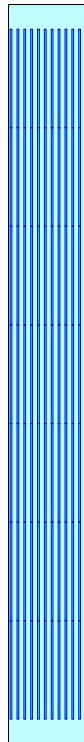
3,621.1 cf Field - 182.8 cf Chambers = 3,438.3 cf Stone x 40.0% Voids = 1,375.3 cf Stone Storage

Chamber Storage + Stone Storage = 1,558.1 cf = 0.036 af

Overall Storage Efficiency = 43.0%

Overall System Size = 150.00' x 16.37' x 1.48'

84 Chambers  
134.1 cy Field  
127.3 cy Stone



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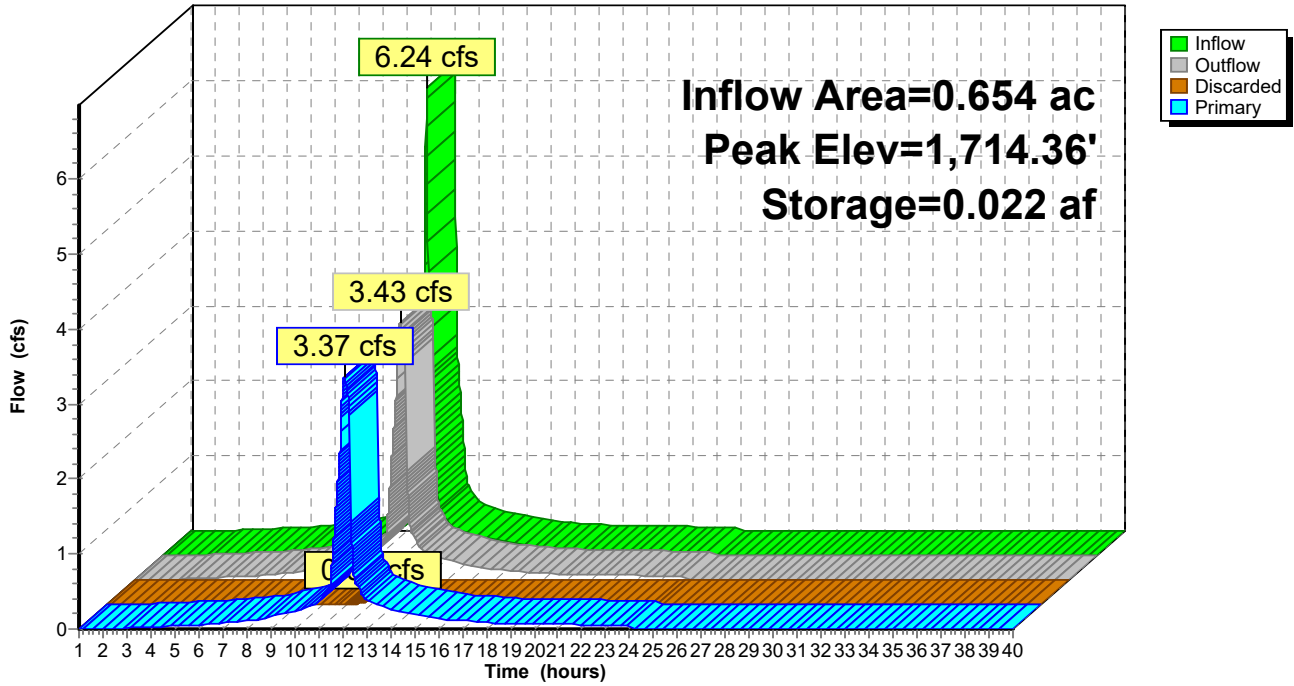
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## Pond 1: one

Hydrograph



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**Summary for Pond 2: Two**

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=2)

Inflow Area = 0.916 ac, 78.95% Impervious, Inflow Depth = 7.35" for 100 Year event  
 Inflow = 8.73 cfs @ 12.00 hrs, Volume= 0.561 af  
 Outflow = 4.96 cfs @ 12.08 hrs, Volume= 0.561 af, Atten= 43%, Lag= 4.4 min  
 Discarded = 0.05 cfs @ 12.08 hrs, Volume= 0.008 af  
 Primary = 4.91 cfs @ 12.08 hrs, Volume= 0.553 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,714.95' @ 12.08 hrs Surf.Area= 0.051 ac Storage= 0.032 af

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.8 min ( 760.2 - 759.4 )

Volume	Invert	Avail.Storage	Storage Description
#1A	1,713.50'	0.029 af	<b>21.93'W x 102.00'L x 1.48'H Field A</b> 0.076 af Overall - 0.004 af Embedded = 0.072 af x 40.0% Voids
#2A	1,713.83'	0.004 af	<b>CPP single-wall 4" x 80</b> Inside #1 Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf 16 Rows of 5 Chambers
		0.033 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	1,712.00'	<b>12.0" Round Culvert</b> L= 150.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,712.00' / 1,711.00' S= 0.0067 '/' Cc= 0.900 n= 0.011, Flow Area= 0.79 sf
#2	Discarded	1,713.50'	<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 2.00'

**Discarded OutFlow** Max=0.05 cfs @ 12.08 hrs HW=1,714.95' (Free Discharge)  
 ↳2=Exfiltration ( Controls 0.05 cfs)

**Primary OutFlow** Max=4.90 cfs @ 12.08 hrs HW=1,714.95' (Free Discharge)  
 ↳1=Culvert (Barrel Controls 4.90 cfs @ 6.24 fps)

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### Pond 2: Two - Chamber Wizard Field A

#### Chamber Model = CPP single-wall 4" (Single-wall corrugated HDPE pipe)

Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf

Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf

4.7" Wide + 12.0" Spacing = 16.7" C-C Row Spacing

5 Chambers/Row x 20.00' Long = 100.00' Row Length +12.0" End Stone x 2 = 102.00' Base Length

16 Rows x 4.7" Wide + 12.0" Spacing x 15 + 4.0" Side Stone x 2 = 21.93' Base Width

4.0" Base + 4.7" Chamber Height + 9.0" Cover = 1.48' Field Height

80 Chambers x 2.2 cf = 174.1 cf Chamber Storage

3,299.9 cf Field - 174.1 cf Chambers = 3,125.8 cf Stone x 40.0% Voids = 1,250.3 cf Stone Storage

Chamber Storage + Stone Storage = 1,424.4 cf = 0.033 af

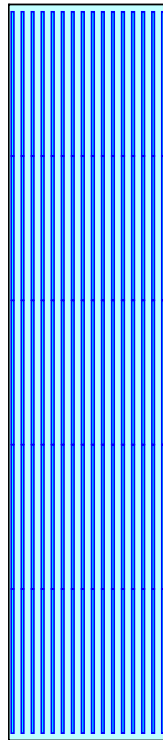
Overall Storage Efficiency = 43.2%

Overall System Size = 102.00' x 21.93' x 1.48'

80 Chambers

122.2 cy Field

115.8 cy Stone



# Proposed Conditions

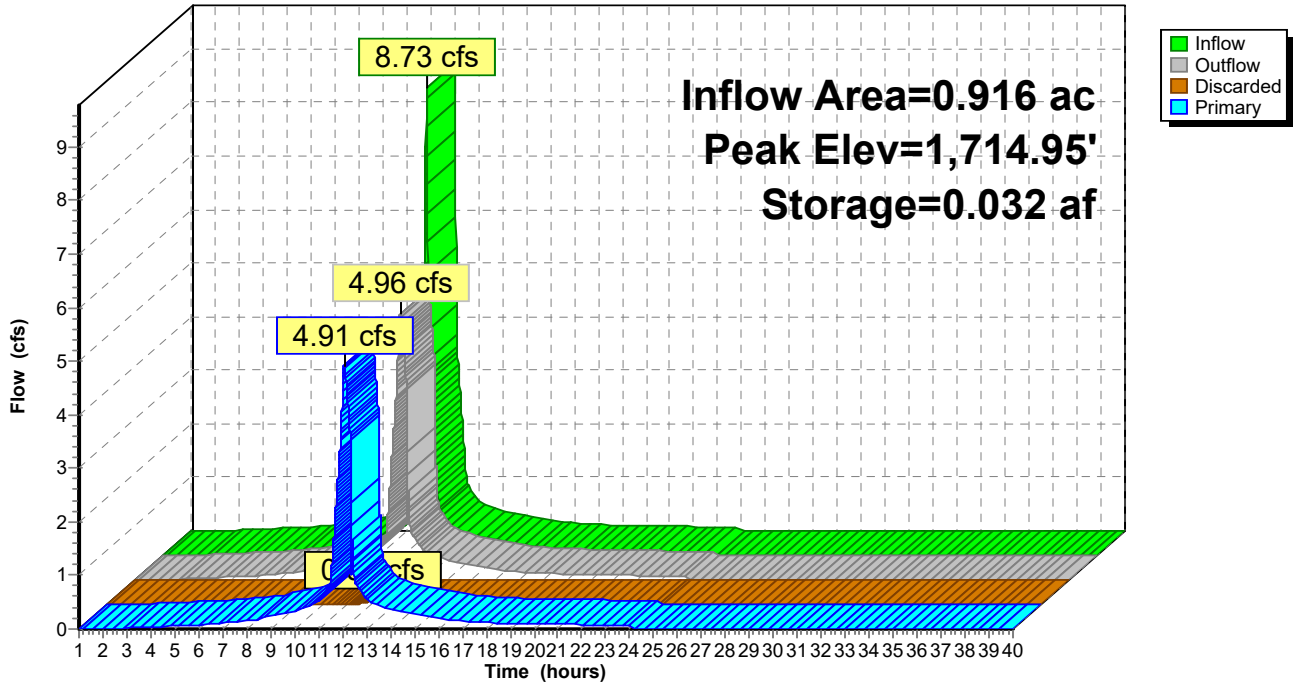
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## Pond 2: Two

Hydrograph



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**Summary for Pond 3: three**

Inflow Area = 1.047 ac, 78.95% Impervious, Inflow Depth = 7.35" for 100 Year event  
 Inflow = 9.98 cfs @ 12.00 hrs, Volume= 0.641 af  
 Outflow = 5.37 cfs @ 12.08 hrs, Volume= 0.641 af, Atten= 46%, Lag= 4.7 min  
 Discarded = 0.08 cfs @ 12.08 hrs, Volume= 0.011 af  
 Primary = 5.30 cfs @ 12.08 hrs, Volume= 0.630 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,714.64' @ 12.08 hrs Surf.Area= 0.076 ac Storage= 0.038 af

Plug-Flow detention time= 1.7 min calculated for 0.641 af (100% of inflow)  
 Center-of-Mass det. time= 0.9 min ( 760.3 - 759.4 )

Volume	Invert	Avail.Storage	Storage Description
#1A	1,713.50'	0.042 af	<b>21.93'W x 150.00'L x 1.48'H Field A</b> 0.111 af Overall - 0.006 af Embedded = 0.106 af x 40.0% Voids
#2A	1,713.83'	0.006 af	<b>CPP single-wall 4" x 112 Inside #1</b> Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf 16 Rows of 7 Chambers
		0.048 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	1,712.00'	<b>6.0" Round Culvert X 6.00</b> L= 150.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,712.00' / 1,711.00' S= 0.0067 '/' Cc= 0.900 n= 0.011, Flow Area= 0.20 sf
#2	Discarded	1,713.50'	<b>1.000 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 2.00'

**Discarded OutFlow** Max=0.08 cfs @ 12.08 hrs HW=1,714.64' (Free Discharge)  
 ↑**2=Exfiltration** ( Controls 0.08 cfs)

**Primary OutFlow** Max=5.30 cfs @ 12.08 hrs HW=1,714.64' (Free Discharge)  
 ↑**1=Culvert** (Barrel Controls 5.30 cfs @ 4.50 fps)

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### Pond 3: three - Chamber Wizard Field A

#### Chamber Model = CPP single-wall 4" (Single-wall corrugated HDPE pipe)

Inside= 4.0"W x 4.0"H => 0.11 sf x 20.00'L = 2.2 cf

Outside= 4.7"W x 4.7"H => 0.11 sf x 20.00'L = 2.2 cf

4.7" Wide + 12.0" Spacing = 16.7" C-C Row Spacing

7 Chambers/Row x 20.00' Long = 140.00' Row Length +60.0" End Stone x 2 = 150.00' Base Length

16 Rows x 4.7" Wide + 12.0" Spacing x 15 + 4.0" Side Stone x 2 = 21.93' Base Width

4.0" Base + 4.7" Chamber Height + 9.0" Cover = 1.48' Field Height

112 Chambers x 2.2 cf = 243.8 cf Chamber Storage

4,852.7 cf Field - 243.8 cf Chambers = 4,609.0 cf Stone x 40.0% Voids = 1,843.6 cf Stone Storage

Chamber Storage + Stone Storage = 2,087.4 cf = 0.048 af

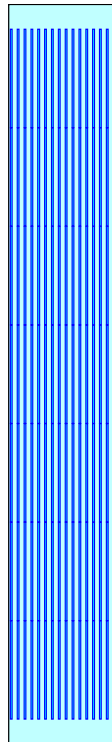
Overall Storage Efficiency = 43.0%

Overall System Size = 150.00' x 21.93' x 1.48'

112 Chambers

179.7 cy Field

170.7 cy Stone



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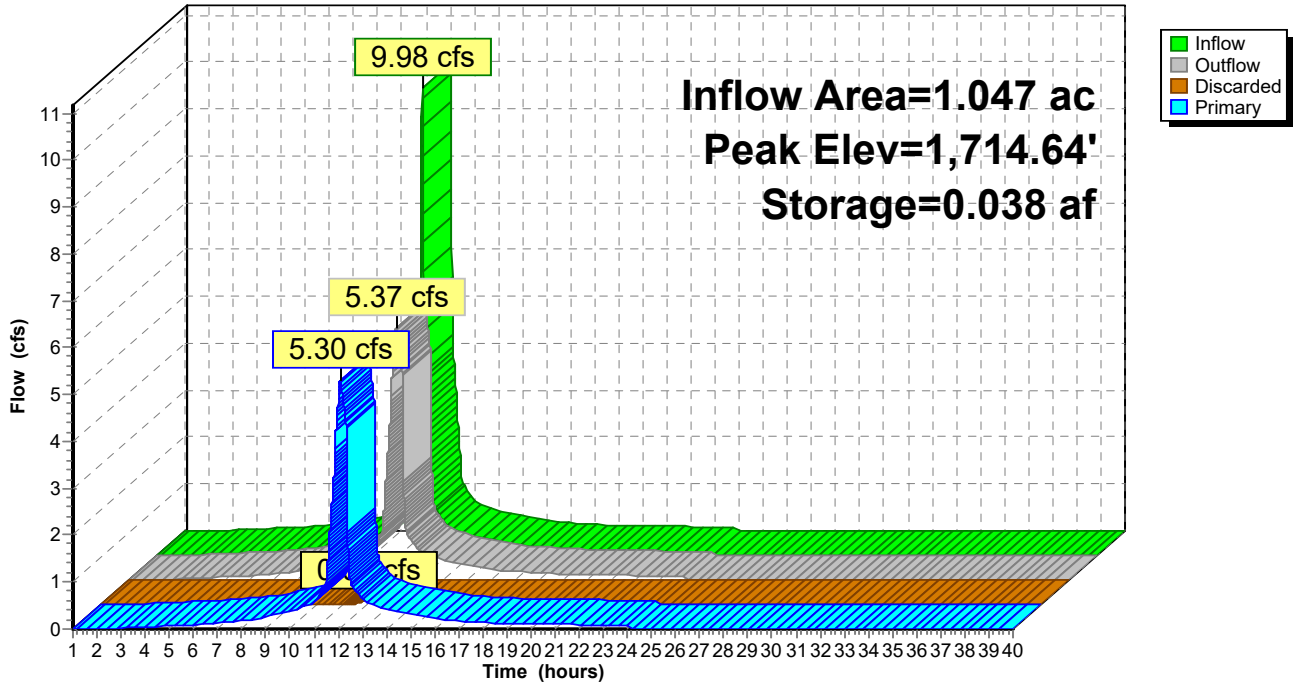
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## Pond 3: three

Hydrograph





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## Summary for Pond DP up: Dry Basin

Inflow Area = 1.876 ac, 0.00% Impervious, Inflow Depth = 4.98" for 100 Year event  
 Inflow = 13.38 cfs @ 12.00 hrs, Volume= 0.778 af  
 Outflow = 2.25 cfs @ 12.43 hrs, Volume= 0.685 af, Atten= 83%, Lag= 25.9 min  
 Primary = 2.25 cfs @ 12.43 hrs, Volume= 0.685 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,733.88' @ 12.43 hrs Surf.Area= 0.112 ac Storage= 0.343 af

Plug-Flow detention time= 147.5 min calculated for 0.685 af (88% of inflow)  
 Center-of-Mass det. time= 92.1 min ( 904.0 - 811.9 )

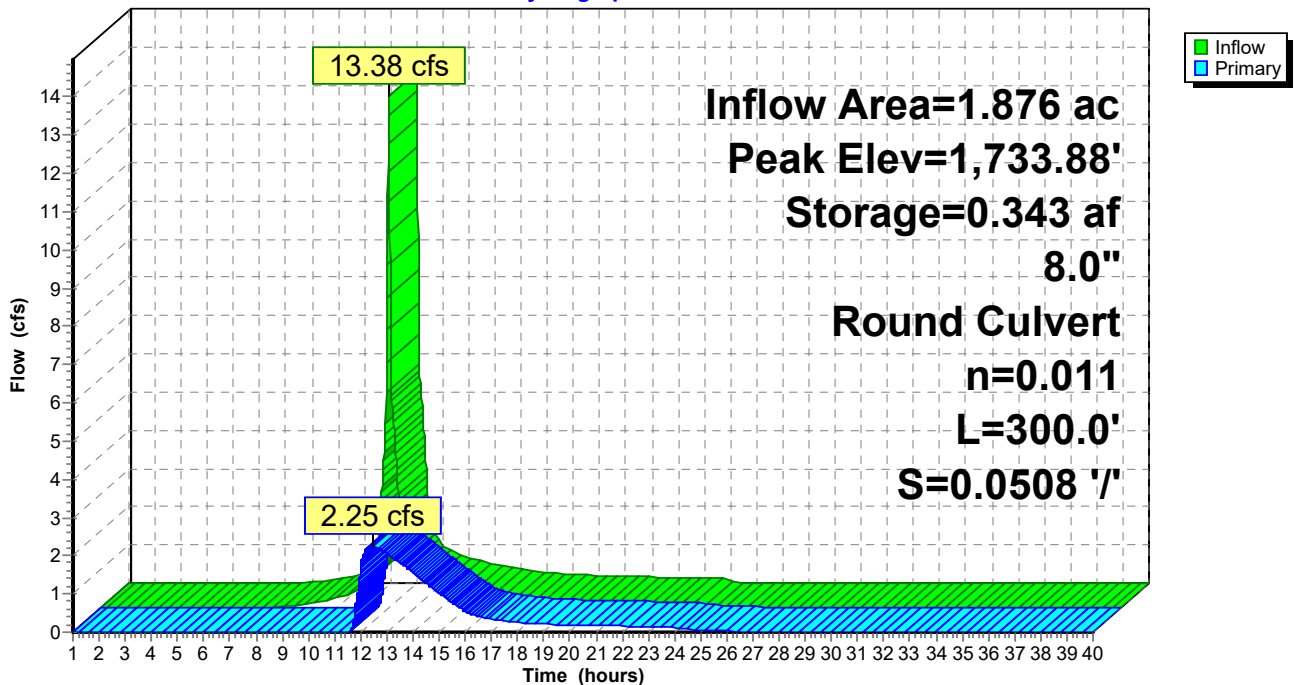
Volume	Invert	Avail.Storage	Storage Description
#1	1,730.00'	0.357 af	<b>45.00'W x 65.00'L x 4.00'H Hillside pond Z=2.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,731.25'	<b>8.0" Round Culvert</b> L= 300.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 1,731.25' / 1,716.00' S= 0.0508 '/ Cc= 0.900 n= 0.011, Flow Area= 0.35 sf

**Primary OutFlow** Max=2.25 cfs @ 12.43 hrs HW=1,733.88' (Free Discharge)  
 ←1=Culvert (Inlet Controls 2.25 cfs @ 6.43 fps)

## Pond DP up: Dry Basin

Hydrograph



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**Summary for Pond DPe: Pond**

[79] Warning: Submerged Pond FBe Primary device # 1 INLET by 0.74'

Inflow Area = 7.028 ac, 1.63% Impervious, Inflow Depth = 5.10" for 100 Year event  
 Inflow = 25.50 cfs @ 12.38 hrs, Volume= 2.985 af  
 Outflow = 25.47 cfs @ 12.39 hrs, Volume= 2.985 af, Atten= 0%, Lag= 0.7 min  
 Primary = 25.47 cfs @ 12.39 hrs, Volume= 2.985 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,708.74' @ 12.39 hrs Surf.Area= 0.033 ac Storage= 0.054 af

Plug-Flow detention time= 3.6 min calculated for 2.984 af (100% of inflow)  
 Center-of-Mass det. time= 3.6 min ( 836.7 - 833.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,706.00'	0.105 af	<b>10.00'W x 38.00'L x 4.00'H Pond Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,706.00'	<b>15.0" Round Culvert</b> L= 35.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,706.00' / 1,706.00' S= 0.0000 '/ Cc= 0.900 n= 0.011, Flow Area= 1.23 sf
#2	Primary	1,708.00'	<b>10.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Primary OutFlow** Max=25.47 cfs @ 12.39 hrs HW=1,708.74' (Free Discharge)

1=Culvert (Barrel Controls 8.33 cfs @ 6.79 fps)

2=Broad-Crested Rectangular Weir (Weir Controls 17.14 cfs @ 2.31 fps)

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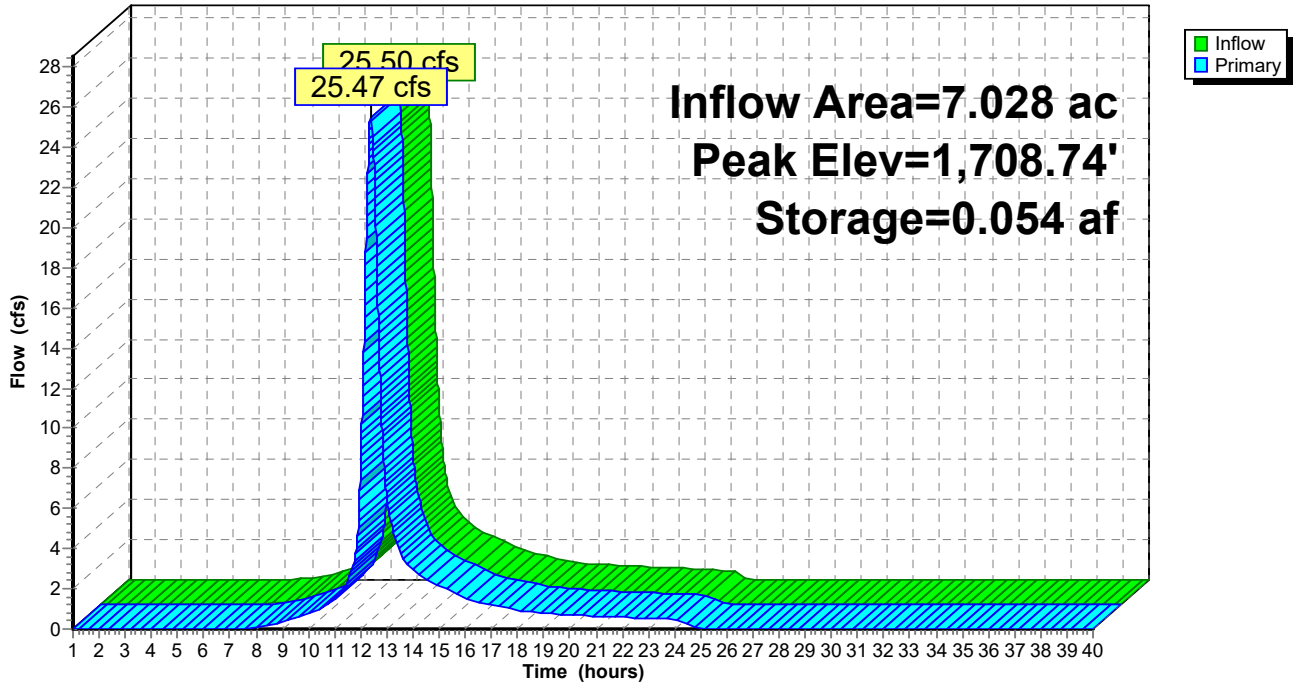
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## Pond DPe: Pond

Hydrograph



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**Summary for Pond DPw: Dry pond**

[81] Warning: Exceeded Pond FBw by 0.16' @ 24.98 hrs

Inflow Area = 2.826 ac, 3.66% Impervious, Inflow Depth = 6.16" for 100 Year event  
 Inflow = 12.83 cfs @ 12.14 hrs, Volume= 1.450 af  
 Outflow = 12.70 cfs @ 12.17 hrs, Volume= 1.385 af, Atten= 1%, Lag= 2.0 min  
 Primary = 12.70 cfs @ 12.17 hrs, Volume= 1.385 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs / 3  
 Peak Elev= 1,703.99' @ 12.17 hrs Surf.Area= 0.059 ac Storage= 0.138 af

Plug-Flow detention time= 52.9 min calculated for 1.385 af (95% of inflow)  
 Center-of-Mass det. time= 25.9 min ( 846.8 - 820.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,700.00'	0.153 af	<b>16.00'W x 40.00'L x 4.25'H Prismatic Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,702.50'	<b>15.0" Round Culvert</b> L= 40.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,702.50' / 1,699.00' S= 0.0875 '/ Cc= 0.900 n= 0.012, Flow Area= 1.23 sf
#2	Primary	1,700.00'	<b>4.0" Round Culvert X 0.00</b> L= 30.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,700.00' / 1,696.00' S= 0.1333 '/ Cc= 0.900 n= 0.011, Flow Area= 0.09 sf
#3	Primary	1,703.50'	<b>8.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Primary OutFlow** Max=12.69 cfs @ 12.17 hrs HW=1,703.99' (Free Discharge)

- 1=Culvert (Inlet Controls 5.51 cfs @ 4.49 fps)
- 2=Culvert ( Controls 0.00 cfs)
- 3=Broad-Crested Rectangular Weir (Weir Controls 7.19 cfs @ 1.82 fps)

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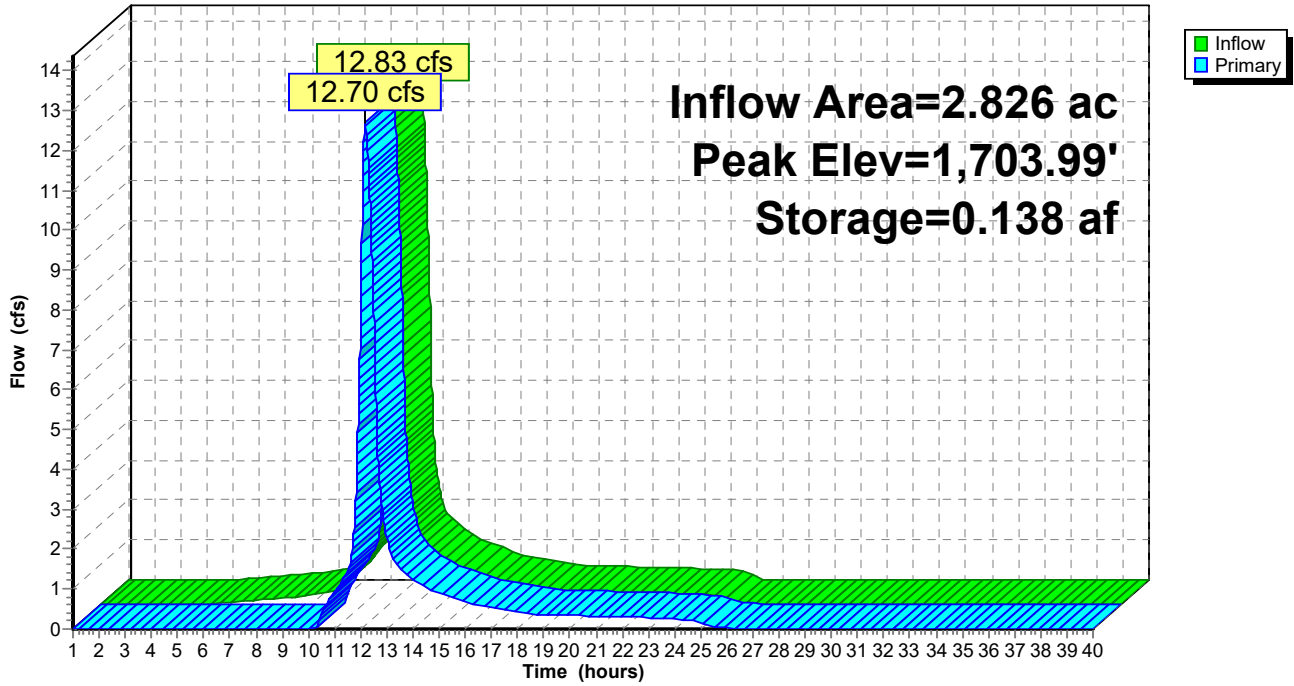
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**Pond DPw: Dry pond**

Hydrograph



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**Summary for Pond FBe: ForeBay**

Inflow Area = 7.028 ac, 1.63% Impervious, Inflow Depth = 5.10" for 100 Year event  
 Inflow = 26.49 cfs @ 12.32 hrs, Volume= 2.985 af  
 Outflow = 25.50 cfs @ 12.38 hrs, Volume= 2.985 af, Atten= 4%, Lag= 3.8 min  
 Primary = 25.50 cfs @ 12.38 hrs, Volume= 2.985 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,711.00' @ 12.38 hrs Surf.Area= 0.037 ac Storage= 0.066 af

Plug-Flow detention time= 1.7 min calculated for 2.984 af (100% of inflow)  
 Center-of-Mass det. time= 1.7 min ( 833.1 - 831.4 )

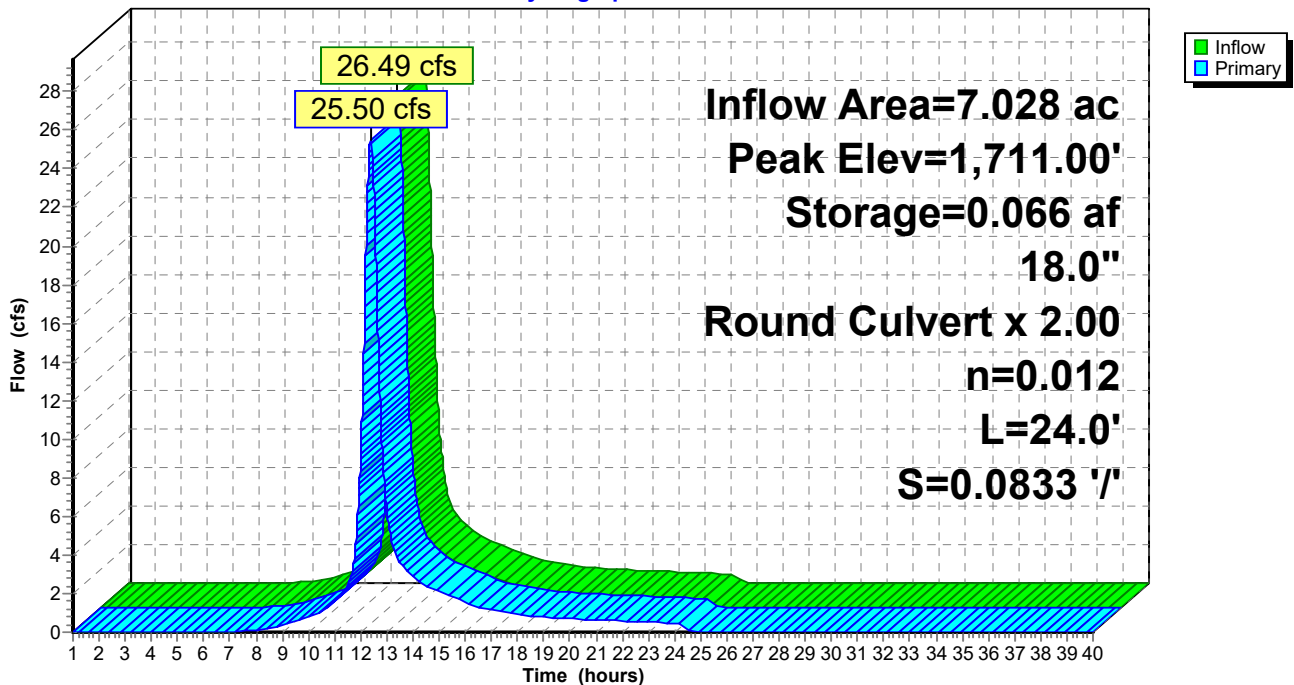
Volume	Invert	Avail.Storage	Storage Description
#1	1,708.00'	0.109 af	<b>10.00'W x 40.00'L x 4.00'H Prismaoid Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,708.00'	<b>18.0" Round Culvert X 2.00</b> L= 24.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,708.00' / 1,706.00' S= 0.0833 '/ Cc= 0.900 n= 0.012, Flow Area= 1.77 sf

**Primary OutFlow** Max=25.50 cfs @ 12.38 hrs HW=1,710.99' (Free Discharge)  
 ←1=Culvert (Inlet Controls 25.50 cfs @ 7.21 fps)

**Pond FBe: ForeBay**

Hydrograph



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**Summary for Pond FBw: Forebay**

[62] Hint: Exceeded Reach 8R OUTLET depth by 3.62' @ 12.15 hrs

Inflow Area = 2.826 ac, 3.66% Impervious, Inflow Depth = 6.16" for 100 Year event  
 Inflow = 22.04 cfs @ 12.05 hrs, Volume= 1.450 af  
 Outflow = 12.83 cfs @ 12.14 hrs, Volume= 1.450 af, Atten= 42%, Lag= 5.2 min  
 Primary = 12.83 cfs @ 12.14 hrs, Volume= 1.450 af

Routing by Stor-Ind method, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs  
 Peak Elev= 1,706.31' @ 12.14 hrs Surf.Area= 0.083 ac Storage= 0.199 af

Plug-Flow detention time= 25.8 min calculated for 1.450 af (100% of inflow)  
 Center-of-Mass det. time= 25.8 min ( 820.8 - 795.1 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,702.50'	0.237 af	<b>16.00'W x 70.00'L x 4.25'H Prismatic Z=3.0</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	1,703.50'	<b>18.0" Round Culvert</b> L= 40.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 1,703.50' / 1,703.00' S= 0.0125 '/' Cc= 0.900 n= 0.012, Flow Area= 1.77 sf
#2	Primary	1,702.25'	<b>4.0" Round Culvert</b> L= 35.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 1,702.25' / 1,702.00' S= 0.0071 '/' Cc= 0.900 n= 0.011, Flow Area= 0.09 sf

**Primary OutFlow** Max=12.83 cfs @ 12.14 hrs HW=1,706.31' (Free Discharge)

- 1=Culvert (Inlet Controls 12.21 cfs @ 6.91 fps)
- 2=Culvert (Barrel Controls 0.62 cfs @ 7.08 fps)

# Proposed Conditions

Prepared by {enter your company name here}

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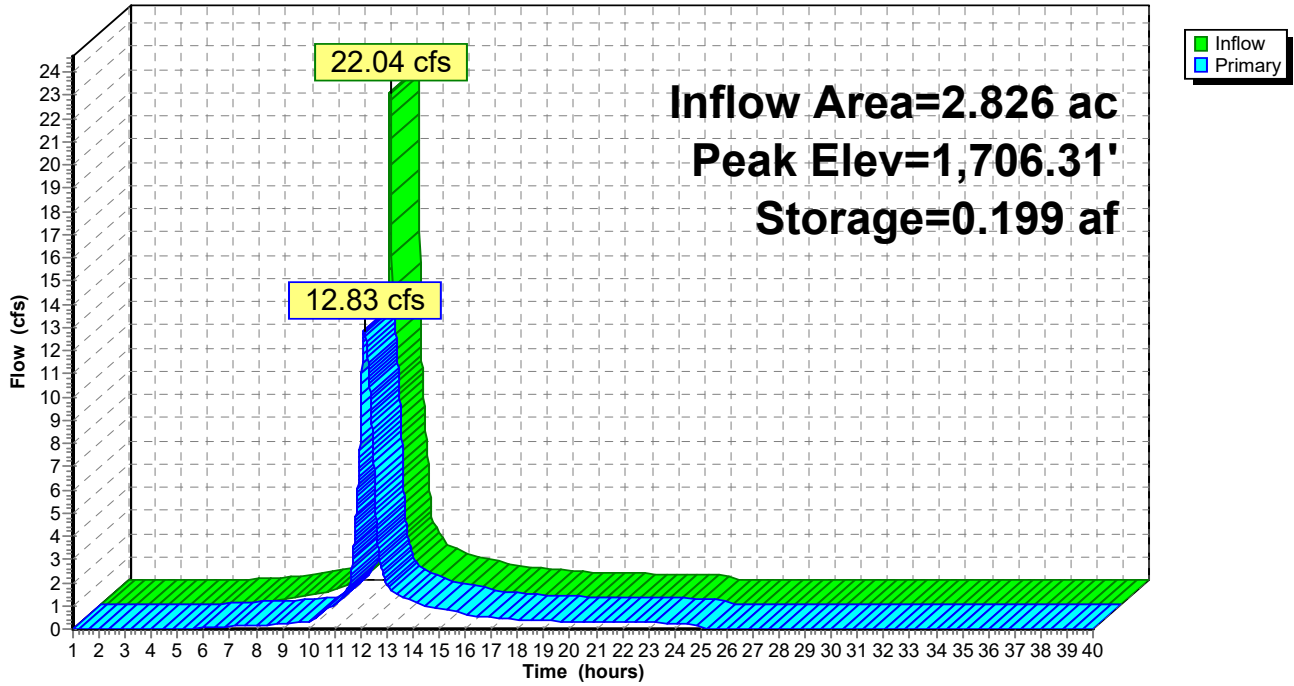
SAMA Productions-Proposed Conditions  
Type III 24-hr 100 Year Rainfall=8.19"

Printed 2/24/2021

Page 100

## Pond FBw: Forebay

Hydrograph





# Proposed Conditions

Prepared by {enter your company name here}

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SAMA Productions-Proposed Conditions  
Type III 24-hr 100 Year Rainfall=8.19"

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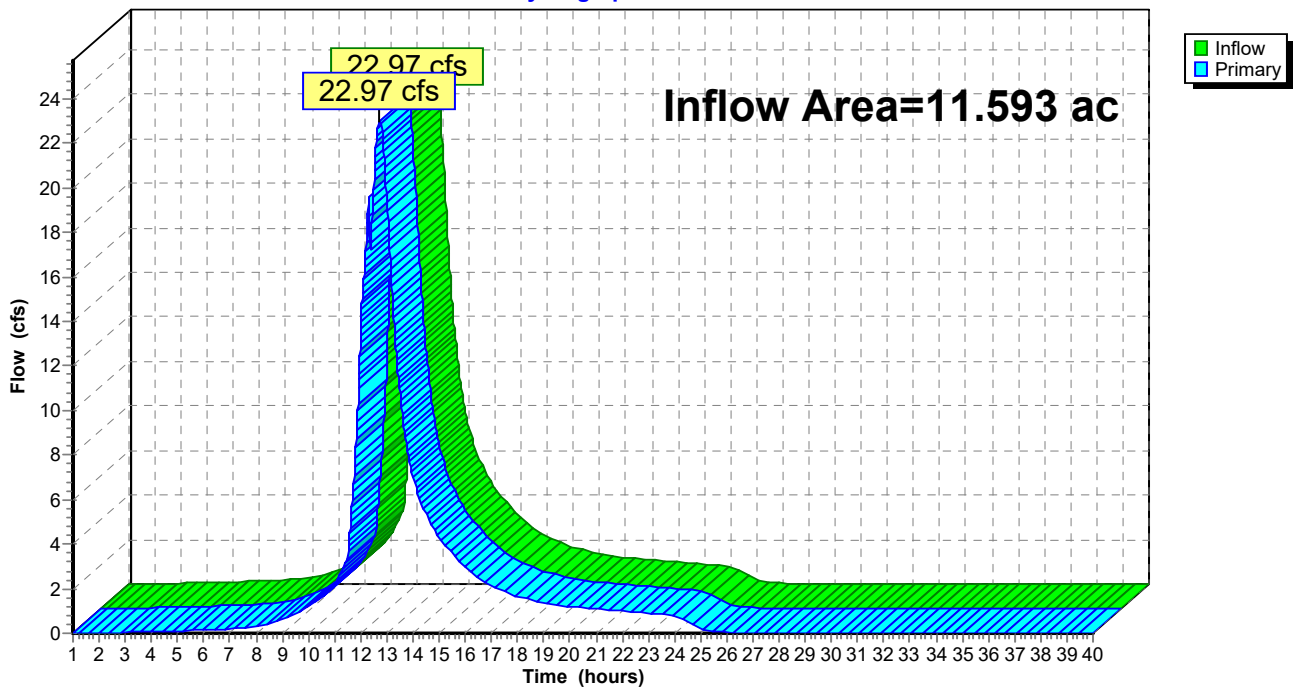
## Summary for Link 7L: wetland

Inflow Area = 11.593 ac, 11.58% Impervious, Inflow Depth = 5.21" for 100 Year event  
Inflow = 22.97 cfs @ 12.72 hrs, Volume= 5.033 af  
Primary = 22.97 cfs @ 12.72 hrs, Volume= 5.033 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 1.00-40.00 hrs, dt= 0.01 hrs

### Link 7L: wetland

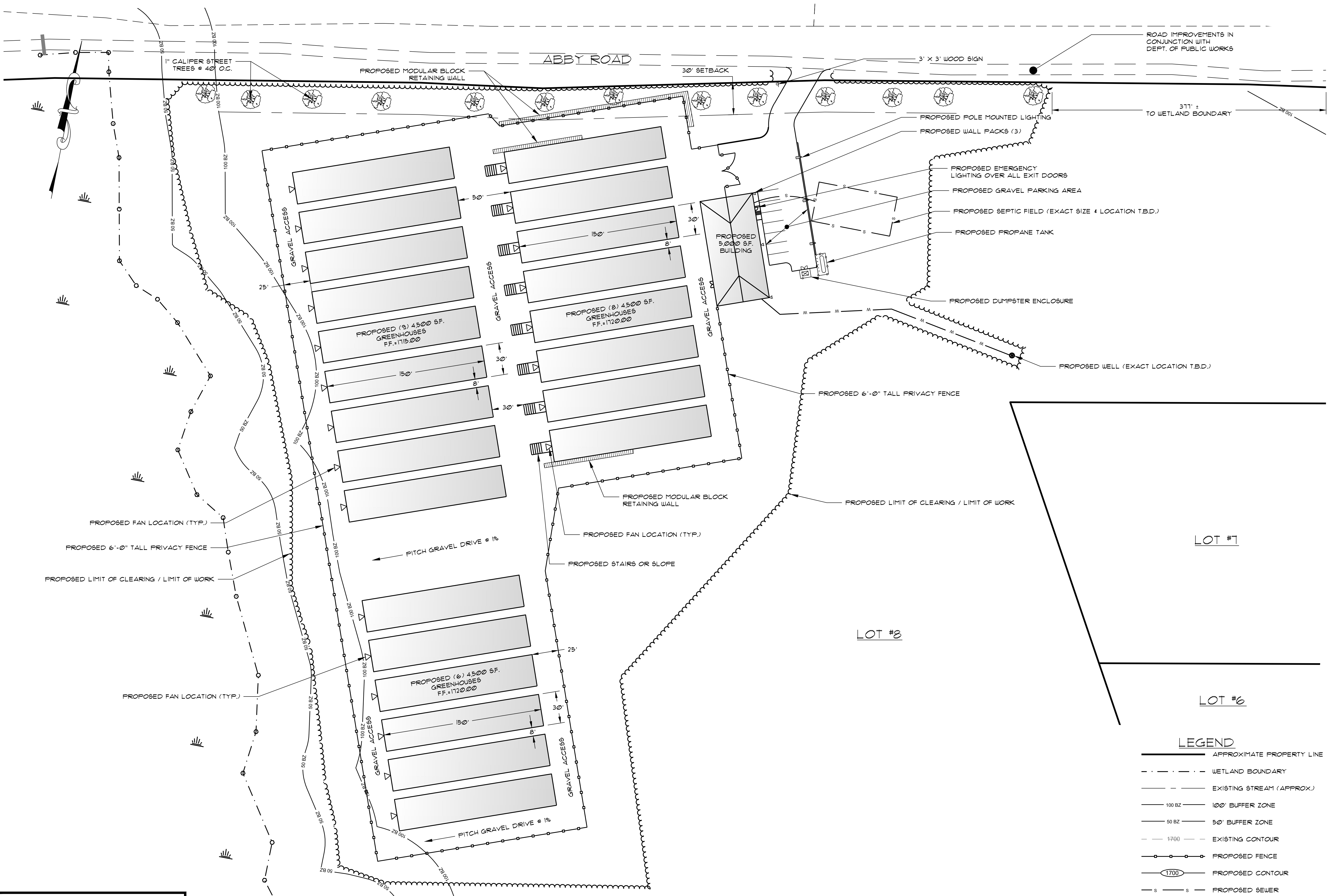
Hydrograph



# **Attachment E**

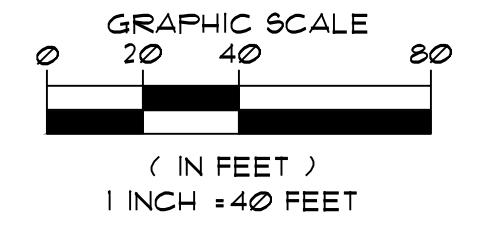
Plans to Accompany Notice of Intent Application  
Prepared for SAMA Productions LLC





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**ENLARGED SITE PLAN**  
 SCALE: 1" = 40'



**LEGEND**

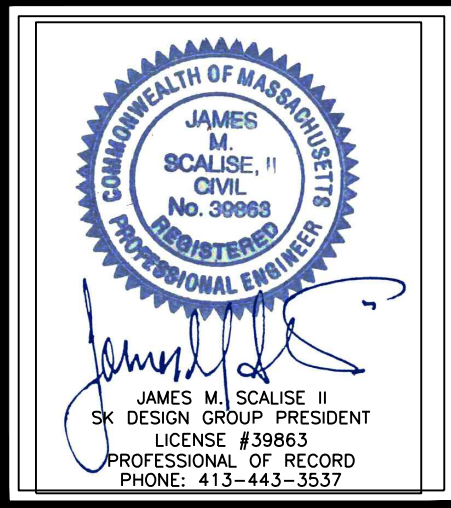
---	APPROXIMATE PROPERTY LINE
- - - - -	WETLAND BOUNDARY
---	EXISTING STREAM (APPROX.)
100 BZ	100' BUFFER ZONE
50 BZ	50' BUFFER ZONE
1700	EXISTING CONTOUR
○	PROPOSED FENCE
○ 1700	PROPOSED CONTOUR
- s - s -	PROPOSED SEWER
- - - - -	PROPOSED DRAIN
~ ~ ~ ~ ~	PROPOSED CLEARING LIMITS
- EC -	PROPOSED EROSION CONTROL

PLANS TO ACCOMPANY NOTICE OF INTENT  
 PREPARED FOR:  
**SAMA PRODUCTIONS, LLC.**  
 LOCATED AT:  
 TOWN HILL ROAD  
 SANDSFIELD MA

**Design Group, Inc.**  
 Civil Engineers \* Surveyors \* Consultants  
 2 FERBER DRIVE \* PITTSFIELD, MASSACHUSETTS 01201 \* (413) 443-3377

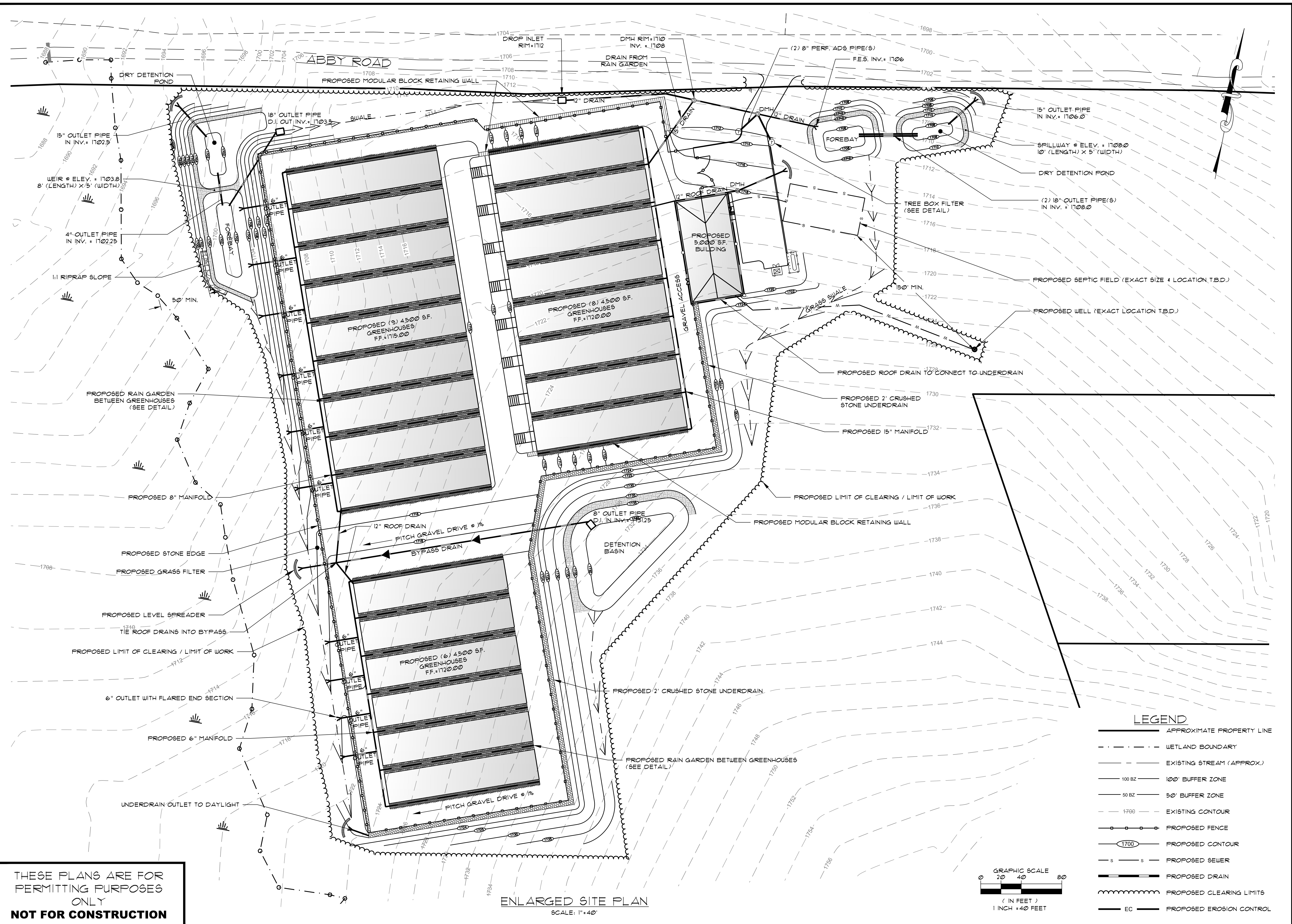
PLAN DESCRIPTION:  
**ENLARGED SITE PLAN**

SK DESIGN GROUP PROJECT #:  
**20012**



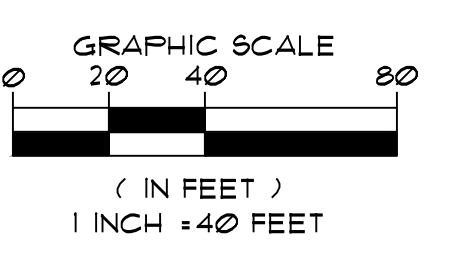
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ORIG. DATE: MARCH 2, 2021	SHEET NO. <b>2</b>
ISSUED FOR: Permit	OF <b>6</b>
SCALE: As Noted	

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 Date: March 04, 2021 - 11:18am



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**ENLARGED SITE PLAN**  
 SCALE: 1" = 40'



**LEGEND**

	APPROXIMATE PROPERTY LINE
	WETLAND BOUNDARY
	EXISTING STREAM (APPROX.)
	100' BUFFER ZONE
	50' BUFFER ZONE
	EXISTING CONTOUR
	PROPOSED FENCE
	PROPOSED CONTOUR
	PROPOSED SEWER
	PROPOSED DRAIN
	PROPOSED CLEARING LIMITS
	PROPOSED EROSION CONTROL

PLANS TO ACCOMPANY NOTICE OF INTENT  
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**SAMA PRODUCTIONS, LLC.**  
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 Civil Engineers \* Surveyors \* Consultants  
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PLAN DESCRIPTION:  
**GRADING PLAN**

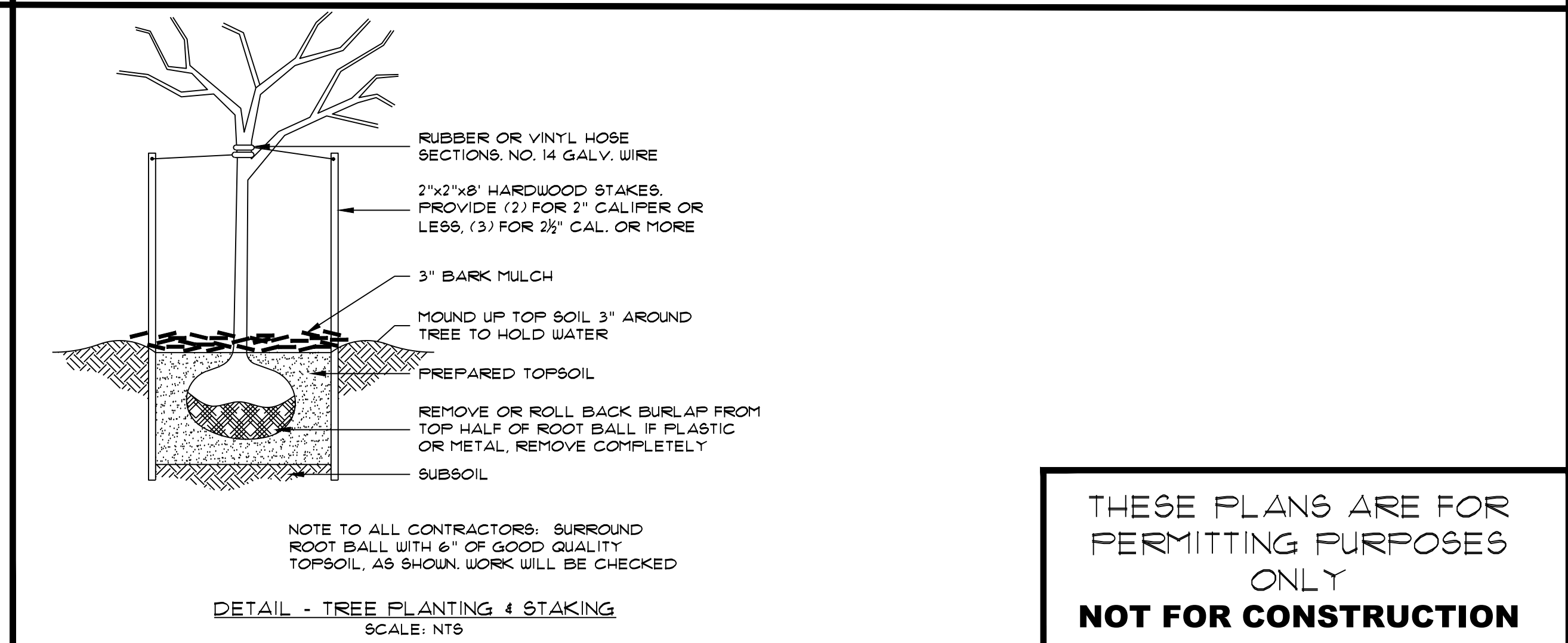
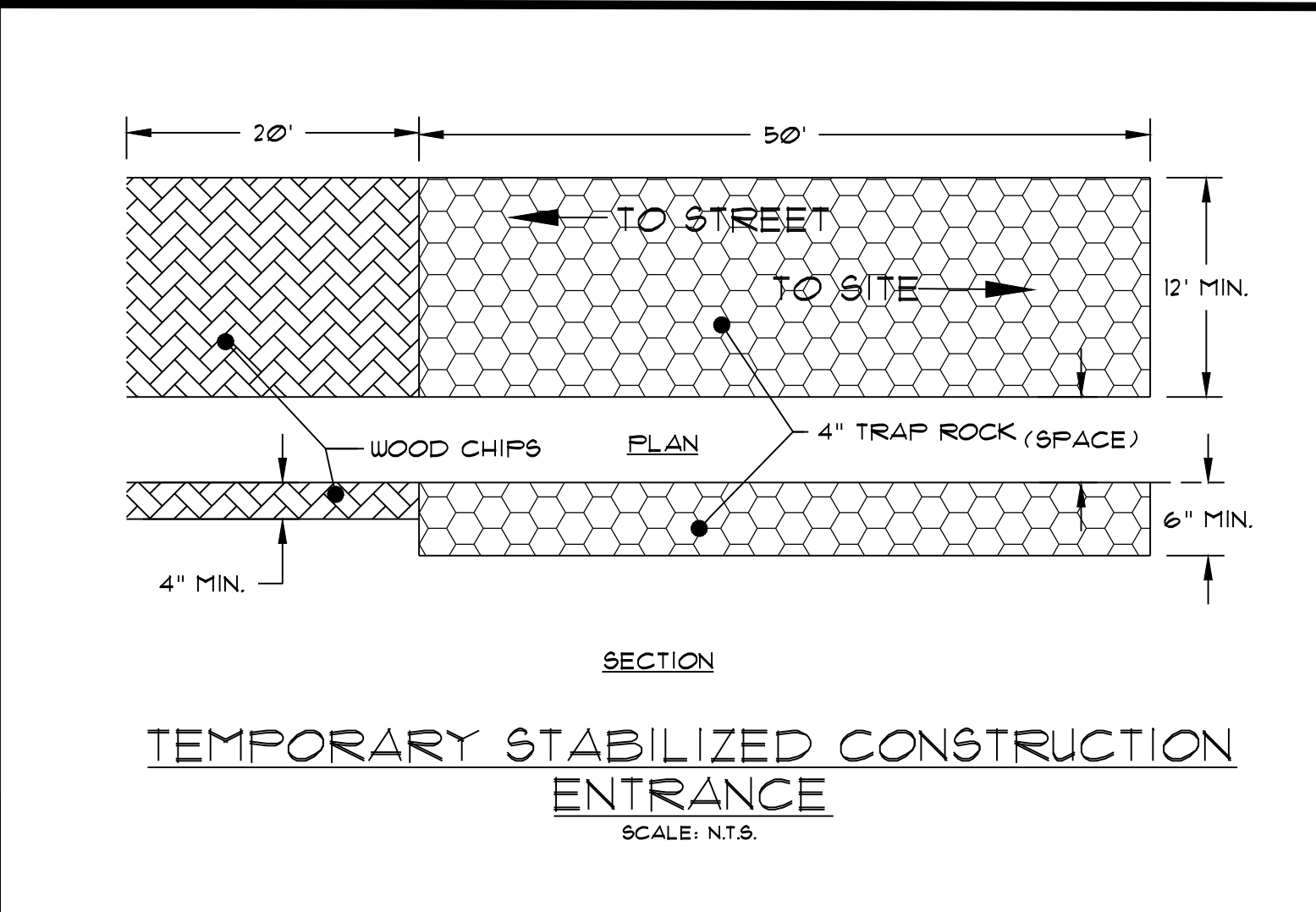
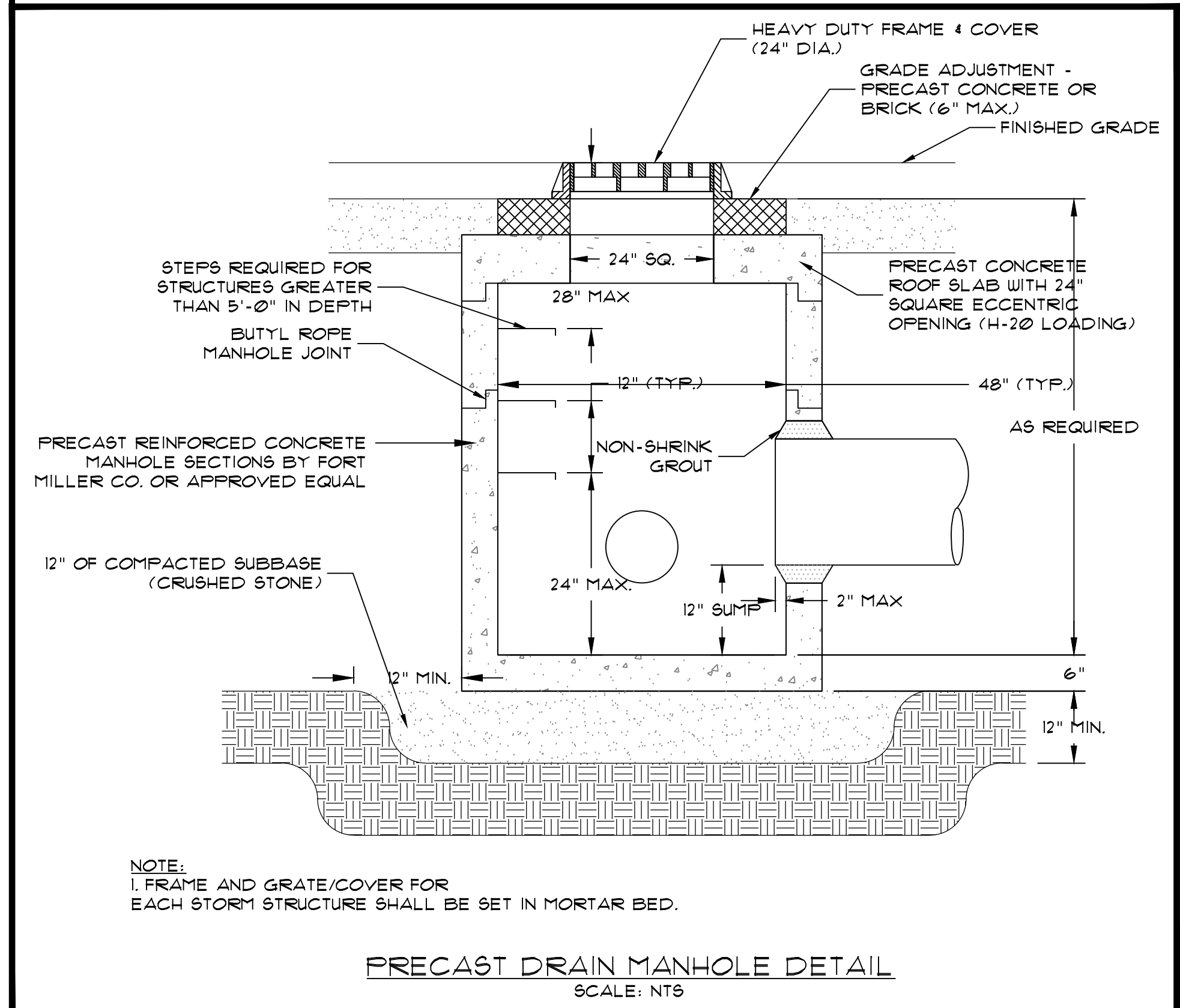
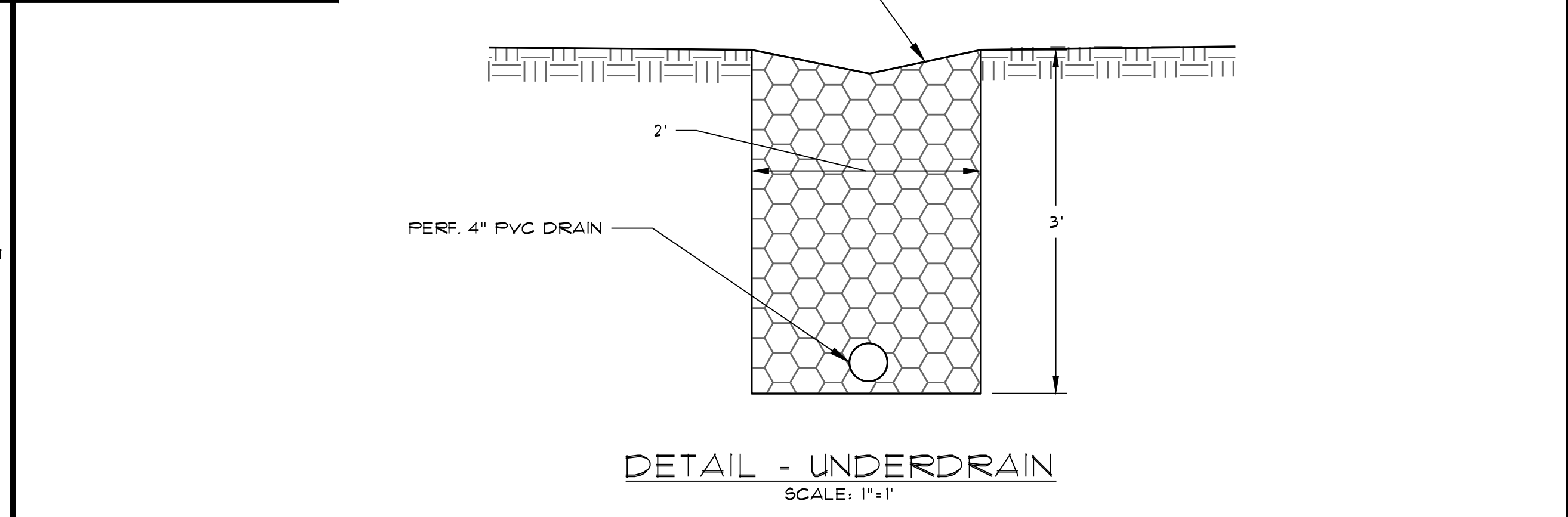
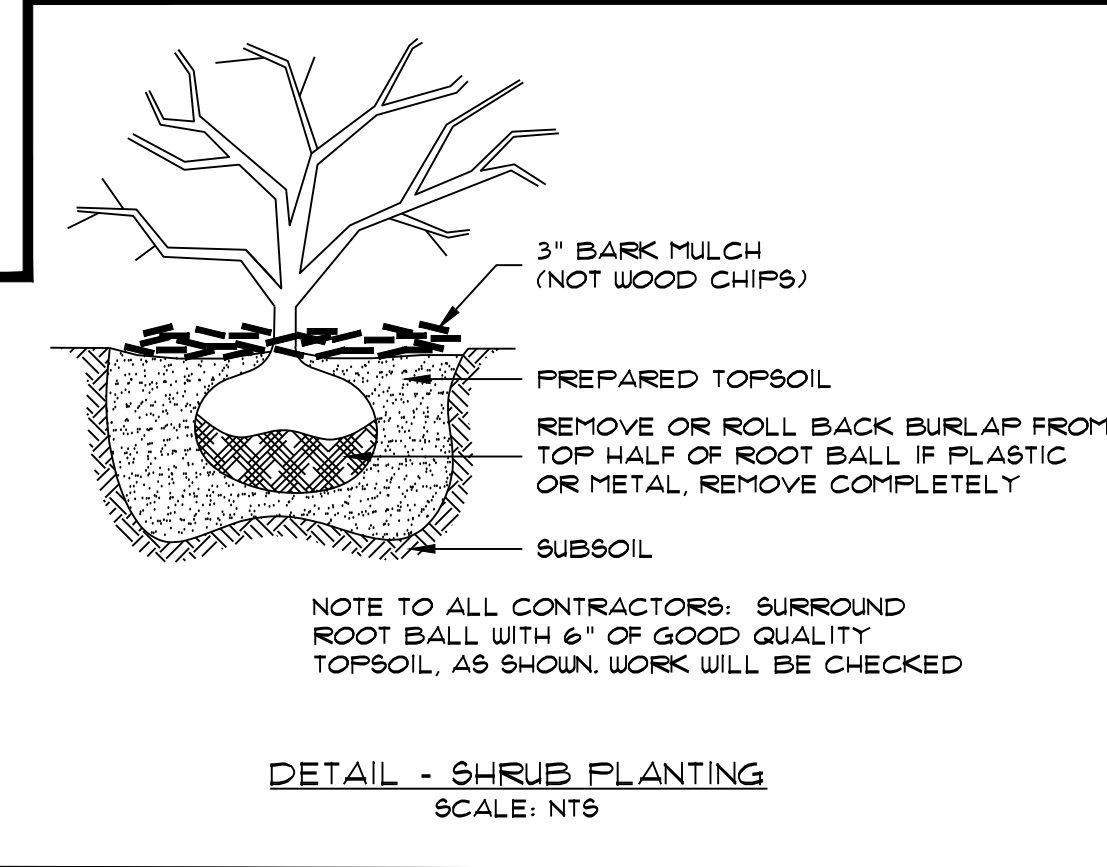
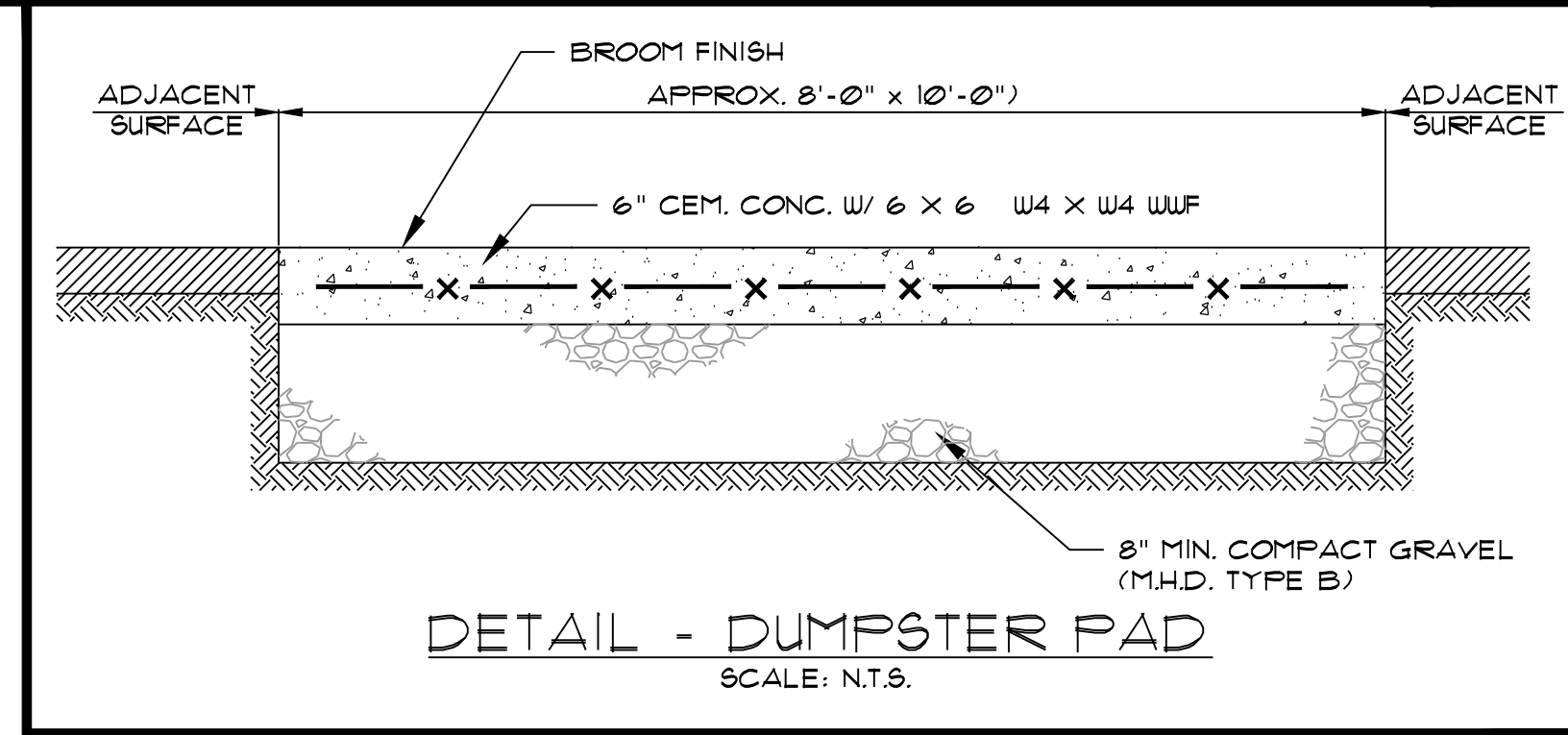
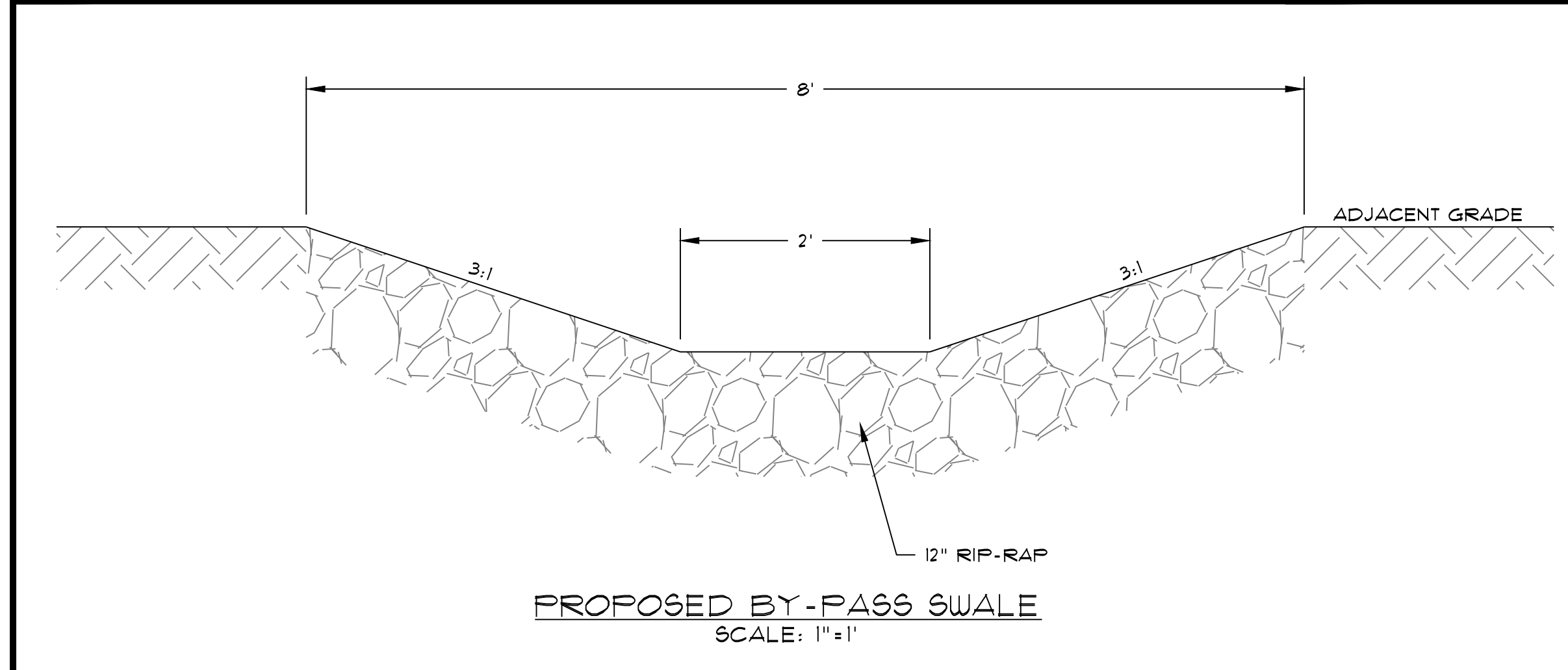
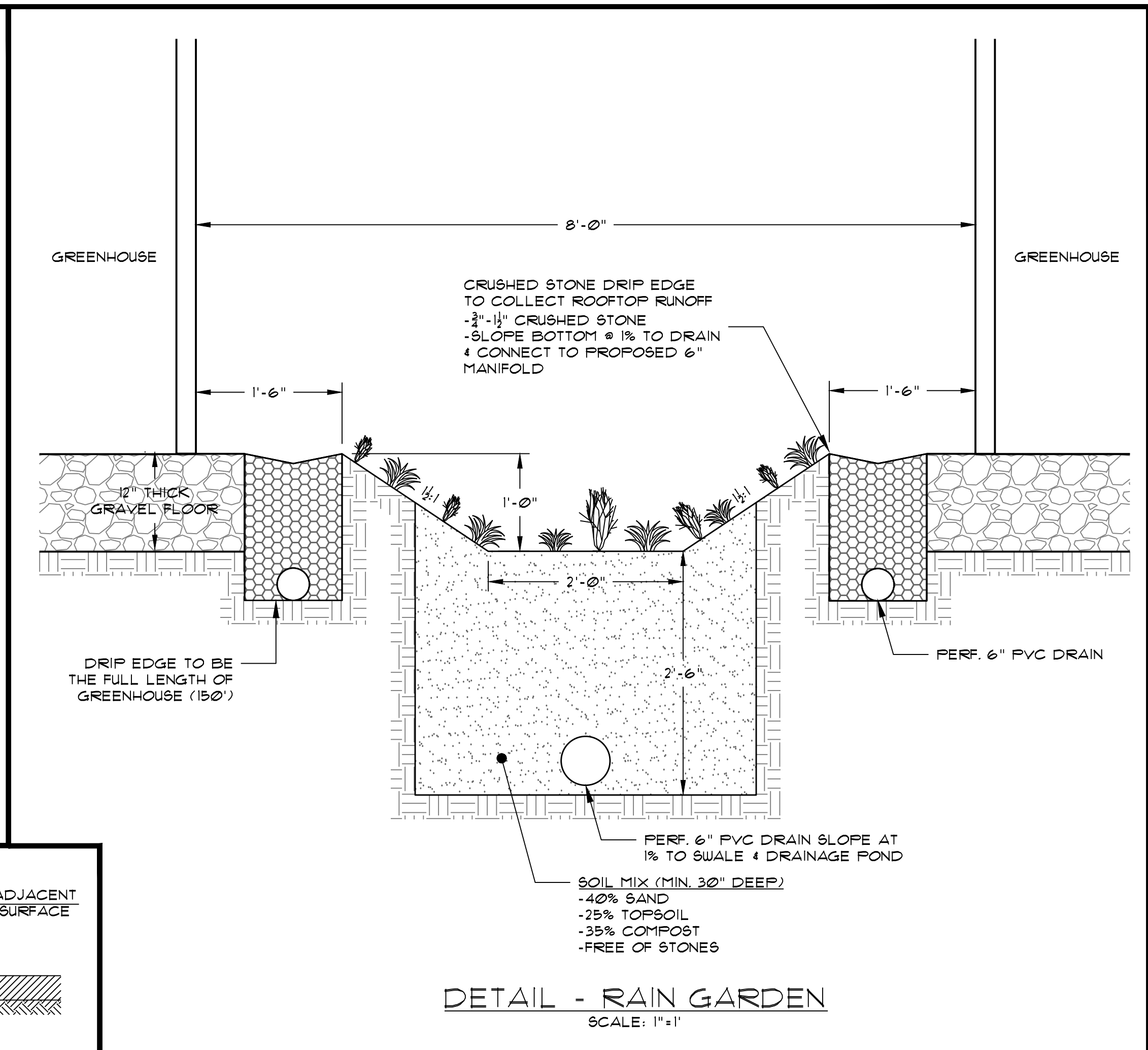
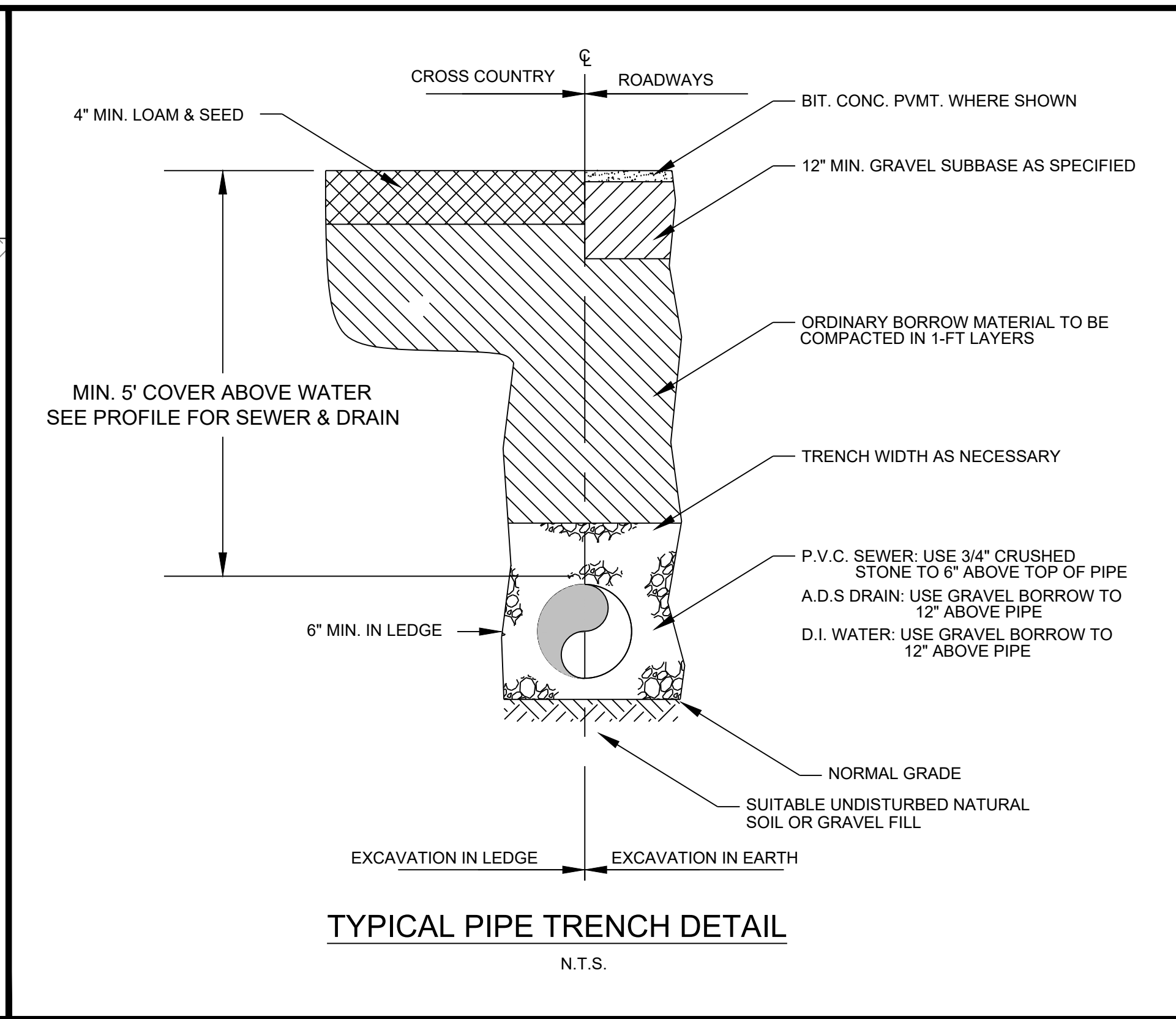
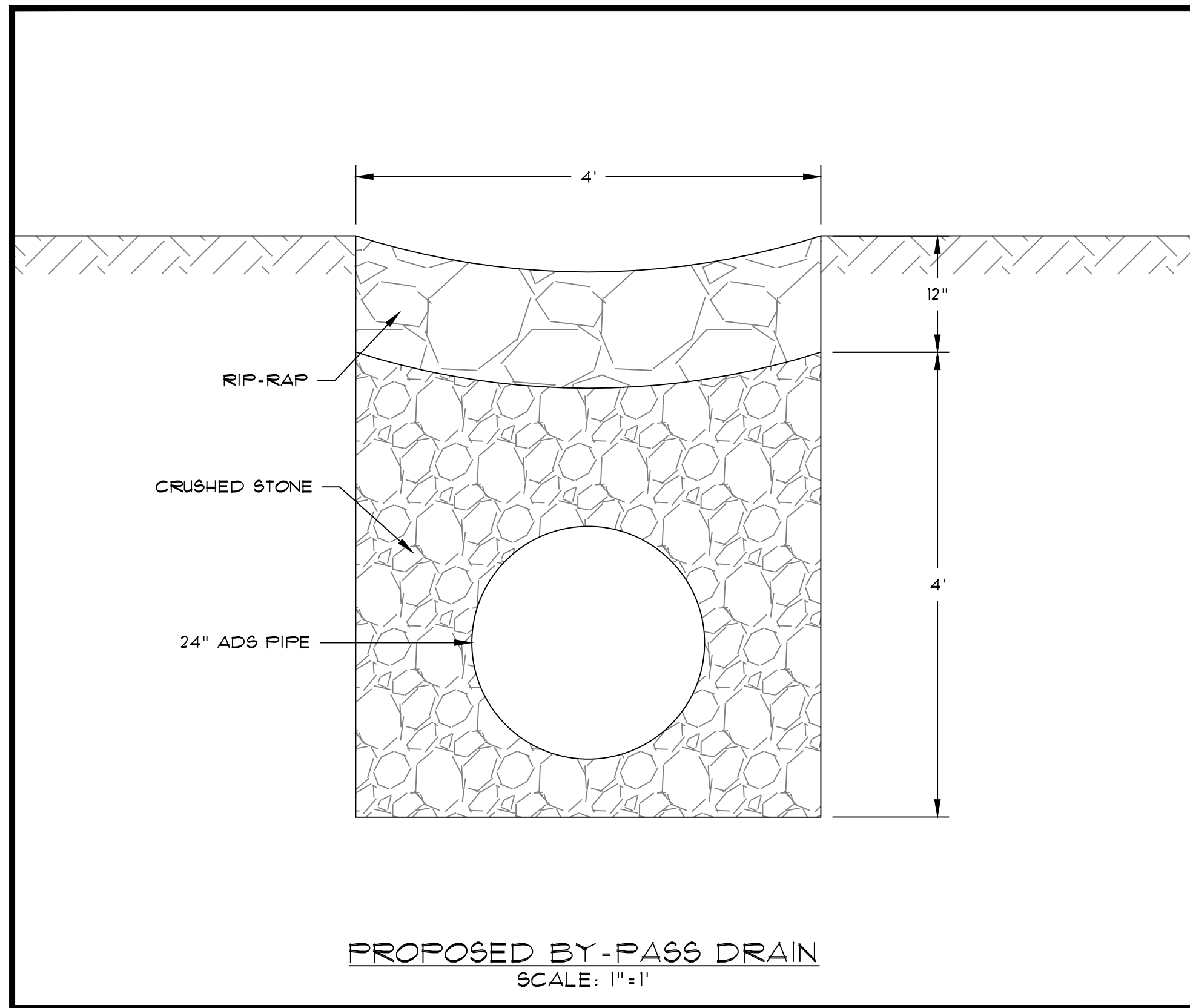
SK DESIGN GROUP PROJECT #:  
**200181**

JAMES M. SCALISE II  
 CIVIL ENGINEER  
 LICENSE #39863  
 PROFESSIONAL OF RECORD  
 PHONE: 413-443-3337

REVISION:	
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ORIG. DATE: MARCH 2, 2021	SHEET NO. <b>3</b>
ISSUED FOR: Permit	OF <b>6</b>
SCALE: As Noted	

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 Date: March 04, 2021 - 11:15am





PLANS TO ACCOMPANY NOTICE OF INTENT  
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LOCATED AT:  
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SANDSFIELD MA

**Design Group, Inc.**  
Civil Engineers & Surveyors & Consultants  
2 FERRIS DRIVE • PITTSFIELD, MASSACHUSETTS 01201 • (413) 445-3377

SK DESIGN GROUP PROJECT #:  
**200181**

PLAN DESCRIPTION:  
**DETAILS**

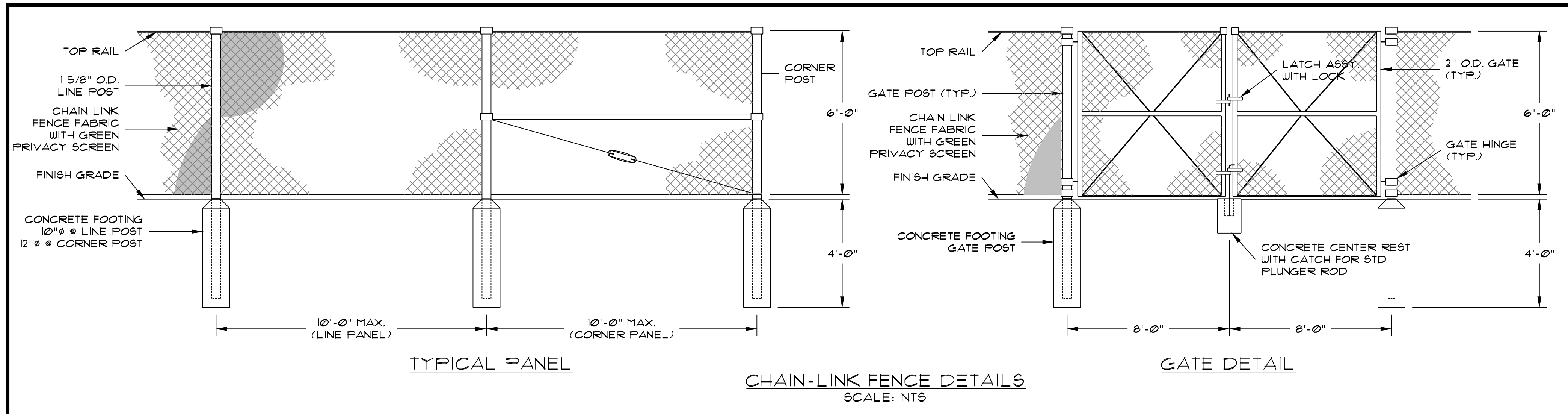
COMMONWEALTH OF MASSACHUSETTS  
JAMES M. SCAUSE II  
CIVIL ENGINEER  
LICENSE #39863  
PROFESSIONAL OF RECORD  
PHONE: 413-443-3377

REVISION:

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SCALE: As Noted

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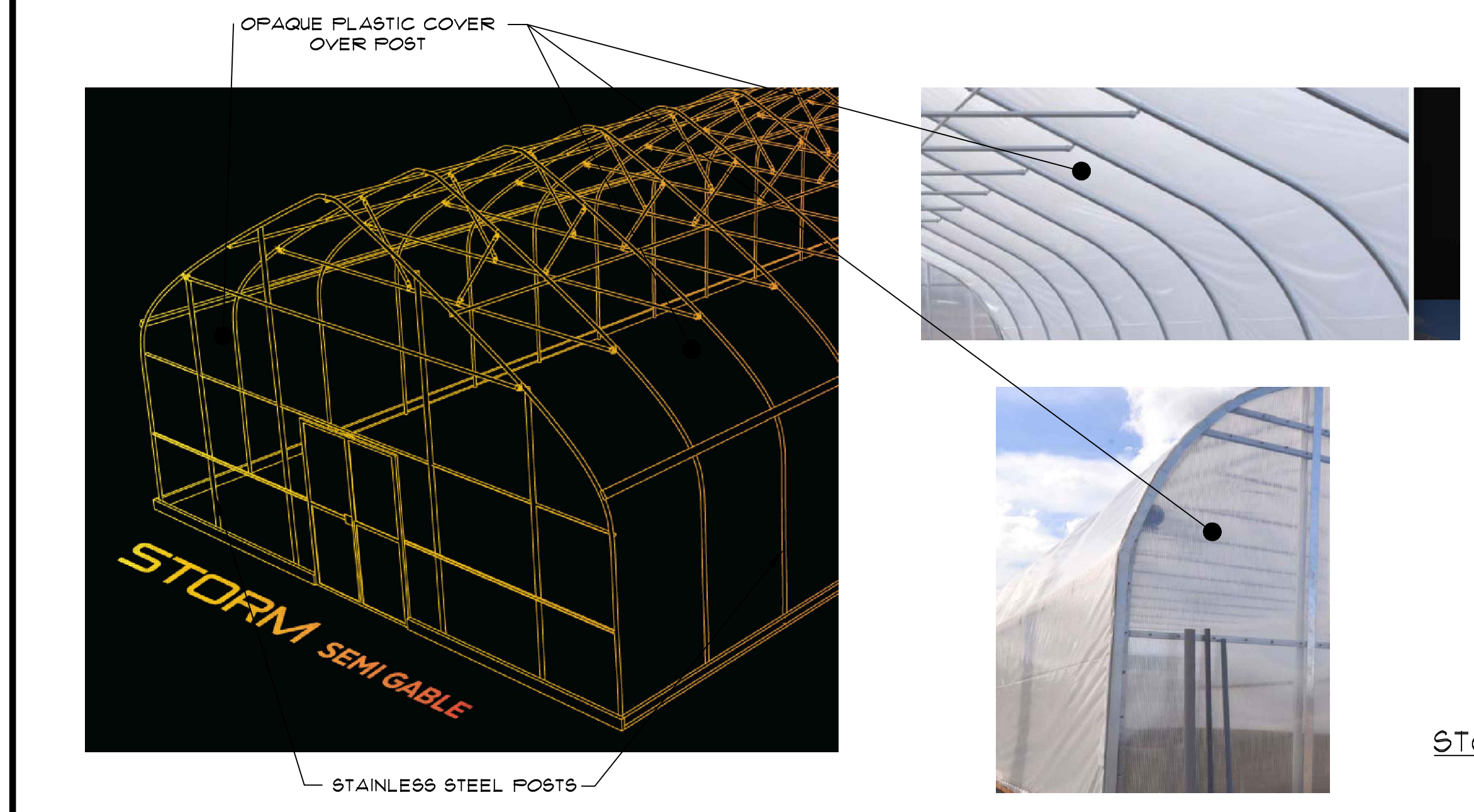


CHAIN-LINK FENCE DETAILS  
SCALE: NTS



SCHEMATIC SECURITY  
SCALE: NOT TO SCALE

NOTE: DUE TO SAFETY & SECURITY CONCERNS, NO FINAL DESIGN OR LAYOUT WILL BE PROVIDED FOR PUBLIC CONSUMPTION. ALL SECURITY WILL BE INSTALLED IN ACCORDANCE WITH ALL APPLICABLE STATE AND LOCAL REGULATIONS.



STORM SEMI GABLE GREENHOUSE  
BY HORTITECH GREENHOUSE  
SCALE: NOT TO SCALE

GREENHOUSE HEIGHT = 16FT

**OPERATION & MAINTENANCE PLAN (STORMWATER SYSTEM)**

1. MAINTENANCE OF THE ON-SITE STORM WATER SYSTEM IS THE RESPONSIBILITY OF THE PROPERTY OWNER. THIS INCLUDES ALL CATCH BASINS, YARD DRAINS, TRENCH DRAIN, SYSTEM PIPING, MANHOLES, ROOF LEADERS, WATER QUALITY BASINS, OUTLETS AND ASSOCIATED PIPING.
2. IN GENERAL, GOOD HOUSEKEEPING PRACTICES SHALL BE INCORPORATED INTO THE ROUTINE SITE AND FACILITY MAINTENANCE PLAN TO MINIMIZE DEPOSITION OF SEDIMENT, LITTER, AND CONTAMINANTS INTO THE STORM DRAINAGE SYSTEM, PAVED PARKING AREAS AND DRIVES, TRUCK LOADING AREA, AND FACILITY GROUNDS.
3. MAINTENANCE RECORDS DOCUMENTING SYSTEM INSPECTION AND CLEANING OPERATIONS SHALL BE MAINTAINED BY THE PROPERTY OWNER AND SHALL BE MADE AVAILABLE FOR INSPECTION BY THE TOWN AS REQUESTED.

**THE FOLLOWING SCHEDULE OF MAINTENANCE SHALL BE FOLLOWED:**

- ANNUALLY: (IN LATE SPRING):**
- A. VISUALLY INSPECT ALL DRAINAGE STRUCTURES. STRUCTURES CONSIST OF CATCH BASINS, YARD DRAINS, MANHOLES, TRENCH DRAIN, AND ALL OTHER SITE DRAINAGE FACILITIES, INCLUDING THE WATER QUALITY BASINS, OUTLETS AND DISCHARGE PIPING AND RIP RAP EROSION CONTROLS AT THE OUTFALLS. NOTE ANY DEFICIENCIES AND MAKE REPAIRS.
  - B. CLEAN THE CATCH BASINS, YARD DRAINS, TRENCH DRAIN, AND ALL OTHER SITE DRAINAGE SITE FACILITIES OF ANY ACCUMULATION OF SEDIMENT AND/OR DEBRIS.
    - B.1. ALL CLEANING AND REMOVAL OF SEDIMENT AND DEBRIS TO BE PERFORMED BY A LICENSED CONTRACTOR.
    - B.2. CLEANING TO BE DONE WITH A VACUUM TRUCK SO THAT DIRECT ACCESS INTO THE DRAINAGE STRUCTURES IS NOT REQUIRED.
    - B.3. ALL MATERIAL REMOVED SHALL BE DISPOSED ACCORDING TO THE REQUIREMENTS OF THE STATE OF CONNECTICUT AND LOCAL REGULATIONS. IF ANY REPAIR WORK IS REQUIRED FOR THE STORMWATER MANAGEMENT SYSTEM, THE WORK INVOLVED SHALL BE CONDUCTED ACCORDING TO FEDERAL, STATE AND LOCAL REGULATIONS.

**SEMI-ANNUALLY: (LATE SPRING, AFTER WINTER SANDING OPERATIONS AND MID FALL, AFTER LEAF LITTER):**

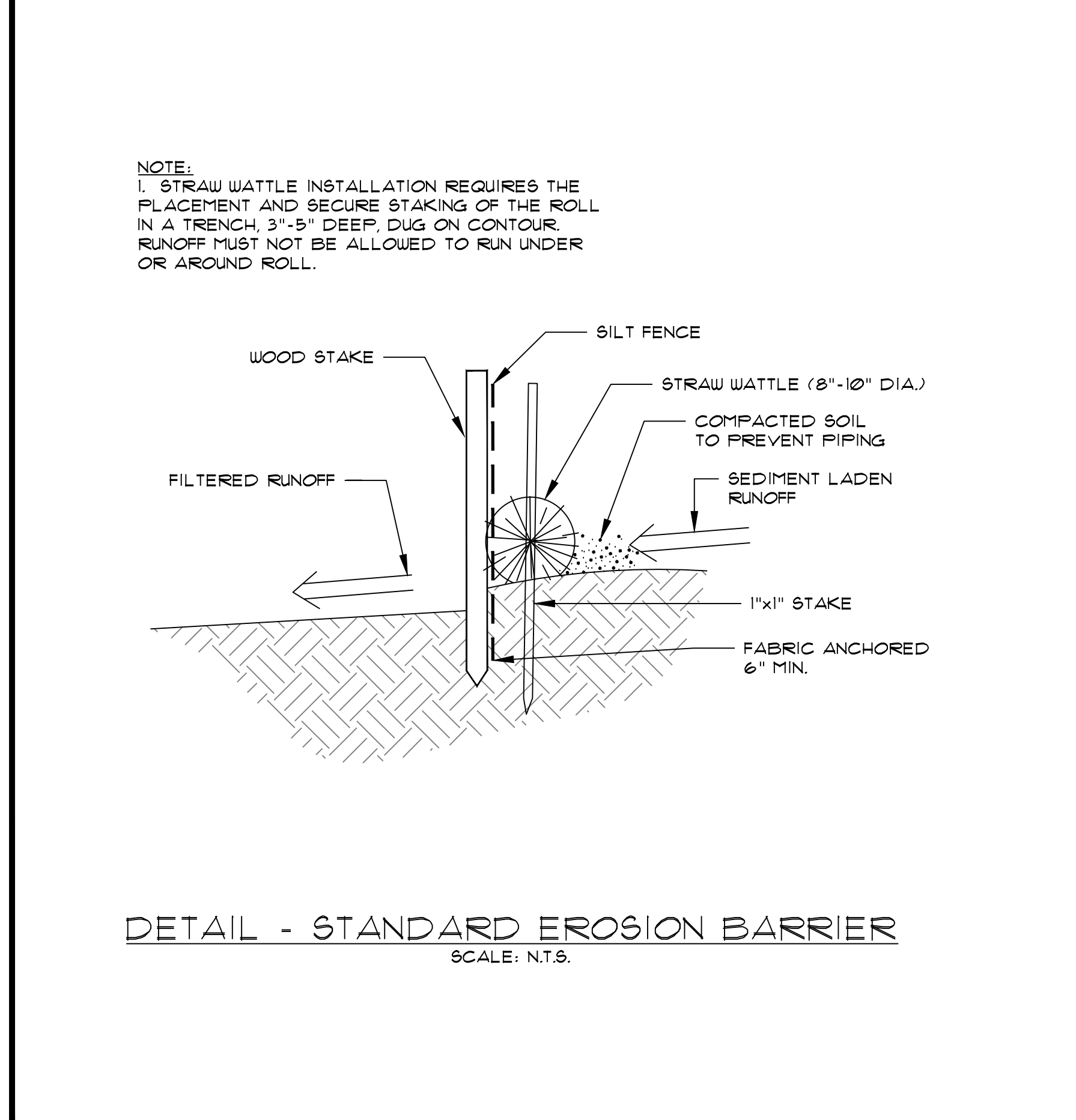
- A. SWEEP OR VACUUM ALL PAVED DRIVES AND PARKING AREAS TO REMOVE ACCUMULATED SEDIMENT. DISPOSE OF MATERIALS AT LICENSED FACILITY.
- B. VERIFY THAT THE OUTLET STRUCTURES FOR THE WATER QUALITY BASINS ARE FREE OF DEBRIS AND LITTER. CLEAN, AS NECESSARY.

**MONTHLY:**

- A. REMOVE LITTER AND OTHER DEBRIS FROM THE SITE, INCLUDING TRUCK LOADING AREA.
  - B. VERIFY THAT THE OUTLET STRUCTURES FOR THE WATER QUALITY BASINS ARE FREE OF DEBRIS AND LITTER. CLEAN, AS NECESSARY.
- ANNUALLY:**
- A. INSPECT WATER QUALITY BASINS.
    - A.1. CUT TRIM AND REMOVE WOODY VEGETATION GROWTH IN SIDESLOPES, BERMS AND BASIN BOTTOM.
    - A.2. INSPECT SEDIMENT FOREBAYS FOR ACCUMULATED SEDIMENT, REMOVE AS NECESSARY.
    - A.3. INSPECT STONE CHECK DAMS AND REPLACE/REPAIR AS NECESSARY.
    - A.4. VERIFY THAT BASINS DRAIN BELOW THE LEVEL OF THE OUTLET ORIFICES WITHIN 24 HOURS FOLLOWING A STORM EVENT. IF REQUIRED, REMOVE DEBRIS FROM OUTLET ORIFICES TO RESTORE FUNCTIONALITY.

**AS NEEDED:**

- A. MAINTAIN LAWN AREAS BY CUTTING WITH MULCHING BLADES OR COLLECTING TRIMMINGS AND DISPOSING OFF SITE.
- B. DO NOT DISPOSE OF LAWN CUTTINGS OR LANDSCAPE TRIMMING ON SITE. DO NOT DEPOSIT OVER FILL SLOPES! DISPOSE OFF SITE.
- C. STABILIZE OR REPAIR ANY LANDSCAPED AREAS ON THE SITE.
- D. CLEAN UP ANY SPILLS OR MATERIAL DEPOSITS IMMEDIATELY AS REQUIRED ACCORDING TO THE REQUIREMENTS OF THE STATE OF CONNECTICUT AND LOCAL REGULATIONS.
- E. PROHIBIT VEHICULAR REPAIR AND VEHICLE WASHING ON THE PREMISES.



DETAIL - STANDARD EROSION BARRIER  
SCALE: N.T.S.

**EROSION CONTROL NOTES**

1. TEMPORARY SEDIMENT CONTROL FEATURES ARE TO BE CONSTRUCTED & FULLY OPERATIONAL PRIOR TO ANY OTHER CONSTRUCTION OR GRADING, AND ARE TO BE MAINTAINED THROUGHOUT CONSTRUCTION, UNTIL PERMANENT GROUND COVER IS ESTABLISHED & APPROVED.
2. ALL SLOPES SHALL BE SODDED, HYDRO-SEEDED OR APPLIED WITH GROUND COVER SPECIFIED AS SOON AS CONSTRUCTION PHASES PERMIT.
3. CONTRACTOR IS RESPONSIBLE FOR DAILY INSPECTIONS AND MAINTENANCE OF THE DETENTION FACILITIES THROUGH CONSTRUCTION AND UNTIL ESTABLISHMENT OF PERMANENT GROUND COVER. SEDIMENT SHALL BE REMOVED AS NECESSARY TO INSURE EFFICIENT OPERATION OF THE FACILITIES DURING AND AFTER CONSTRUCTION.
4. CONTRACTOR IS RESPONSIBLE FOR MONITORING DOWNSIDE CONDITIONS THROUGHOUT THE CONSTRUCTION PERIOD AND CLEARING ANY DEBRIS AND SEDIMENT CAUSED BY THE CONSTRUCTION.
5. ADDITIONAL EROSION CONTROL MEASURES MAY BE REQUIRED BY THE ENGINEER OR REGULATING AGENCIES.
6. THE SOIL EROSION SEDIMENT CONTROL PROCEDURES AND DETAILS AS SHOWN SHALL BE FOLLOWED AND INSTALLED IN A MANNER SO AS TO MINIMIZE EROSION OF THE DISTURBED AREAS AND PREVENT SEDIMENT FROM LEAVING THE SITE, OR ENTERING THE ADJACENT WETLANDS.
7. ANY DISTURBED EARTH AREAS THAT SHALL BE IDLE FOR 20 DAYS OR LONGER, SHALL HAVE TEMPORARY GRASSING APPLIED.
8. PRIOR TO ANY OTHER CONSTRUCTION, A STABILIZED CONSTRUCTION ENTRANCE SHALL BE CONSTRUCTED AT THE ENTRY TO THE SITE
9. AFTER CONSTRUCTION, THE DETENTION POND AND POND OUTLET STRUCTURES SHALL BE CLEANED OF ALL DEBRIS AND EXCESS SEDIMENT. BOTTOMS OF PONDS SHALL BE BROUGHT TO ELEVATION AND SHAPED AS SHOWN ON SITE GRADING PLAN AND DETAILS.
10. ADDITIONAL EROSION CONTROL DEVICES SHALL BE INSTALLED IMMEDIATELY AFTER GROUND DISTURBANCE OCCURS. THE LOCATION OF SOME OF THE EROSION CONTROL DEVICES MAY HAVE TO BE ALTERED FROM THAT SHOWN ON THE APPROVED PLANS IF DRAINAGE PATTERNS DURING CONSTRUCTION ARE DIFFERENT FROM THE FINAL PROPOSED DRAINAGE PATTERNS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ACCOMPLISH EROSION CONTROL FOR ALL DRAINAGE PATTERNS CREATED AT VARIOUS STAGES DURING CONSTRUCTION. ANY DIFFICULTY IN CONTROLLING EROSION DURING ANY PHASE OF CONSTRUCTION SHALL BE REPORTED TO THE ENGINEER IMMEDIATELY. IN NO CASE SHALL LESS EROSION CONTROLS THAN APPROVED BE PERMITTED.
11. ALL OPEN SWALES MUST BE STABILIZED W/ MULCH & SEED OR RIP-RAP IMMEDIATELY AFTER CONSTRUCTION.
12. CONDUIT OUTLET PROTECTION (RIP-RAP & EROSION CONTROLS) MUST BE INSTALLED AT ALL REQUIRED OUT FALLS PRIOR TO THE DRAINAGE SYSTEM BECOMING OPERATIONAL.
14. AT THE TIME WHEN SITE PREPARATION FOR PERMANENT VEGETATIVE STABILIZATION IS TO BE ESTABLISHED, ANY SOIL THAT WILL NOT PROVIDE A SUITABLE ENVIRONMENT TO SUPPORT ADEQUATE VEGETATIVE GROUND COVER SHALL BE REMOVED OR TREATED IN SUCH A WAY THAT WILL PERMANENTLY ADJUST THE SOIL CONDITIONS AND RENDER IT SUITABLE FOR VEGETATIVE GROUND COVER.
15. IF THE REMOVAL OR TREATMENT OF THE SOIL WILL NOT PROVIDE SUITABLE CONDITIONS, ANOTHER MEANS OF PERMANENT GROUND STABILIZATION WILL HAVE TO BE EMPLOYED.
16. HAY BALES AND/OR STRAW BALES SHALL BE REPLACED EVERY 3 MONTHS TO MAINTAIN THEIR EFFECTIVENESS.

PLANS TO ACCOMPANY NOTICE OF INTENT  
PREPARED FOR:  
**SAMA PRODUCTIONS, LLC.**  
LOCATED AT:  
TOWN HILL ROAD  
SANDSFIELD MA

**Design Group, Inc.**  
Civil Engineers \* Surveyors \* Consultants  
2 FEDERAL DRIVE \* PITTSFIELD, MASSACHUSETTS 01201 \* (413) 443-3377

PLAN DESCRIPTION:  
**DETAILS**

SK DESIGN GROUP PROJECT #:  
**200181**

COMMONWEALTH OF MASSACHUSETTS  
JAMES M. SCALISE, II  
CIVIL  
No. 30983  
PROFESSIONAL ENGINEER

JAMES M. SCALISE, II  
SK DESIGN GROUP PRESIDENT  
LICENSE #30983  
PROFESSIONAL OF RECORD  
PHONE: 413-443-3377

REVISION:

DRAWN BY: AMS	CHECKED BY: JMS II
ORIG. DATE: MARCH 2, 2021	SHEET NO. 6
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SCALE: As Noted	6

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